GENERAL SITE CONSTRUCTION NOTES

- HUSKA CONSULTING, LLC IS NOT RESPONSIBLE FOR CONSTRUCTION SAFETY. ACCIDENTS, OR SUPERVISION; HUSKA CONSULTING, LLC IS NOT RESPONSIBLE FOR ANY CONSTRUCTION DAMAGE OR INJURY TO ANY PERSON, VEHICLE, EQUIPMENT, OR PROPERTY ON OR NEAR THE CONSTRUCTION SITE.
- 2. HUSKA CONSULTING, LLC IS NOT RESPONSIBLE FOR CONSTRUCTION SITE SECURITY. THE CONTRACTOR SHALL COORDINATE ALL TEMPORARY SITE SECURITY WITH THE OWNER AS REQUIRED AND APPROPRIATE.
- 3. THE PROJECT PROPERTY SHALL BE VERIFIED BY A LICENSED LAND SURVEYOR PRIOR TO CONSTRUCTION. IF ANY DISCREPANCIES ARE FOUND REGARDING THE PROJECT BOUNDARY NOTIFY HUSKA CONSULTING, LLC.
- 4. BEFORE COMMENCING CONSTRUCTION, CALL 'MISS UTILITY' TO FIELD MARK UNDERGROUND UTILITIES. FOLLOW MISS UTILITY REQUIREMENTS.
- 5. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS WHICH ARE NOT PROVIDED WITH THE CONSTRUCTION DOCUMENTS. THIS INCLUDES ANCILLARY DESIGN, PERMIT PROCESSING, INSPECTIONS, AND CLOSEOUTS. ALL PERMITS MUST BE ONSITE. INCLUDING PUBLIC SPACE
- 6. THE EXTENT OF EXISTING STRUCTURES INCLUDING UNDERGROUND FEATURES MAY NOT BE DEPICTED ON THE PLANS.

EXCAVATION, OCCUPANCY, AND TRAFFIC CONTROL PLANS IF/AS REQUIRED.

- THE CONTRACTOR MUST NOTIFY THE FAIRFAX COUNTY INSPECTOR BEFORE MAKING ANY FIELD ADJUSTMENTS TO ACCOMMODATE EXISTING CONDITIONS.
- 8. ALL GENERAL NOTES ARE FOR TYPICAL CONSTRUCTION ACTIVITIES; THEY MAY INCLUDE INFORMATION THAT IS NOT APPLICABLE TO THE SCOPE OF THIS PROJECT.
- 9. THE VARIOUS CODES AND STANDARDS WHICH ARE SHOWN ON THE PLANS ARE FOR GENERAL INFORMATION ONLY: THEY DO NOT NECESSARILY REPRESENT THE MOST CURRENT OR COMPLETE STANDARDS REQUIRED FOR THE CONSTRUCTION. THE CONTRACTOR MUST REFER TO THE CORRECT, APPLICABLE CODES AND STANDARDS.
- 10. ACCESS TO THE PROJECT PROPERTY AND ALL SURROUNDING AREAS MUST BE MAINTAINED FOR ALL EMERGENCY SERVICES, PEDESTRIANS, AND DELIVERIES IF REQUIRED AND AS APPROPRIATE. ACCESS TO FIRE HYDRANTS MUST NOT BE IMPAIRED
- 11. THE CONTRACTOR SHALL RESTORE OR REPLACE ANY ITEMS TO REMAIN THAT ARE DAMAGED DURING CONSTRUCTION.
- 12. THE CONTRACTOR MUST MAINTAIN A SET OF CONSTRUCTION PLANS WHICH HAVE BEEN MARKED UP TO ACCURATELY CONVEY CONSTRUCTION WHICH HAS DEVIATED FROM THE APPROVED CONSTRUCTION PLANS. THESE PLANS MUST BE PROVIDED TO THE CLIENT, THE CLIENT'S REPRESENTATIVE, OR HUSKA CONSULTING, LLC PRIOR TO THE PROJECT CLOSEOUT.

GENERAL PAVEMENT NOTES

- EXISTING PAVEMENT TO BE REPLACED SHALL AT MINIMUM MATCH THE EXISTING CROSS SECTION.
- 2. EXISTING CURB AND/OR GUTTER TO BE REPLACED SHALL MATCH EXISTING TYPE, MATERIAL, AND DIMENSIONS.
- 3. SAWCUT EXISTING ASPHALT PAVEMENT 1' FROM EDGE OF NEW CURB AND/OR GUTER FOR REPLACEMENT.
- 4. MILL AND OVERLAY EXISTING ASPHALT PAVEMENT 1' FROM EDGE OF NEW PAVEMENT TO PROVIDE SMOOTH TRANSITION.

GENERAL UTILITY NOTES

- THE CONSTRUCTION WORK SHALL BE COMPLETED IN SUCH A WAY AS TO MINIMIZE UTILITY OUTAGES. ALL UTILITY OUTAGES MUST BE COORDINATED WITH THE UTILITY OWNER AND AFFECTED PARTIES.
- 2. SOME EXISTING UTILITIES MAY NOT BE SHOWN ON THE PLANS. BEFORE BEGINNING CONSTRUCTION. VERIFY THERE ARE NO CONFLICTS WITH EXISTING UTILITIES. TEST PIT AS REQUIRED TO DETERMINE LOCATIONS AND DEPTHS OF EXISTING UTILITIES WITHIN THE CONSTRUCTION WORK AREA. IF ANY EXISTING UTILITIES ARE FOUND WHICH ARE NOT DEPICTED IN THE EXISTING CONDITIONS PLAN OR CONFLICT WITH THE PROPOSED WORK NOTIFY HUSKA CONSULTING,
- 3. NOTIFY HUSKA CONSULTING, LLC IF COVER FOR ANY UTILITY IS REDUCED BELOW THE MINIMUM REQUIRED.
- 4. THE SITE CIVIL PLAN IS MEANT TO CONVEY 'WET' (SANITARY SEWER, STORM SEWER, AND WATER) UTILITY WORK. ALL 'DRY' (ELECTRIC, NATURAL GAS, TELECOMMUNICATIONS) UTILITY WORK SHOWN IS FOR INFORMATION AND REFERENCE ONLY. REFER TO THE DRY UTILITY AND/OR MEP PLANS FOR DRY UTILITY WORK.
- 5. REFER TO FAIRFAX WATER STANDARDS FOR ABANDONMENT OF EXISTING WATER LATERALS AND MAINS. NOTE THIS INVOLVES DISCONNECTING ALL LATERALS AT THE MAINS, PLUGGING AND SEALING THE MAINS, AND REMOVING ALL ABANDONED METERS, VALVES, AND APPURTENANCES. COORDINATE WITH THE FAIRFAX WATER INSPECTOR.
- REFER TO THE FAIRFAX CITY PUBLIC FACILITIES MANUAL FOR ABANDONMENT OF EXISTING SANITARY SEWER MAINS, STORM SEWER MAINS, AND LATERALS. NOTE THIS INVOLVES DISCONNECTING ALL LATERALS AT THE MAINS, PLUGGING AND SEALING THE MAINS, AND REMOVING ALL ABANDONED METERS, VALVES, AND APPURTENANCES. COORDINATE WITH THE FAIRFAX CITY INSPECTOR.
- 7. ALL WYE CONNECTIONS TO EXISTING SEWER LINES SHALL MATCH THE EXISTING SIZE AND MATERIAL
- 8. REMOVE ABANDONED UTILITIES AS REQUIRED.
- 9. ADJUST EXISTING STRUCTURE TOPS AND MANHOLES TO REMAIN WITHIN THE LIMITS OF DISTURBANCE TO MATCH FINAL GRADE AS REQUIRED. INSTALL ADDITIONAL STEPS WITHIN MANHOLES AS REQUIRED.

GENERAL GRADING NOTES

- 1. THE SITE MUST BE GRADED AND PAVED SO THAT NO NEW LOW POINTS WITHOUT PROPER DRAINAGE ARE CREATED; NO PONDING SHALL OCCUR ONSITE UNLESS SPECIFICALLY NOTED OTHERWISE ON THE STORMWATER MANAGEMENT PLANS WITHIN BMP FACILITIES OR ON THE SEDIMENT CONTROL PLAN WITHIN SEDIMENT TRAPS OR BASINS.
- 2. ALL PAVED SURFACES SHALL BE AT A 0.5% MINIMUM SLOPE. ALL GRASSED AND LANDSCAPED AREAS SHALL BE AT A 1% MINIMUM SLOPE. EXCEPTIONS MAY BE MADE ONLY IF APPROVED BY HUSKA CONSULTING, LLC.
- 3. SPOT ELEVATIONS SHOWN AT TIE-IN POINTS WITH EXISTING SURFACES ARE SHOWN APPROXIMATE, AND SHALL BE FIELD VERIFIED BY THE CONTRACTOR. PROPOSED ELEVATIONS MAY BE MODIFIED WITH APPROVAL FROM HUSKA CONSULTING, LLC TO MATCH EXISTING GRADE.
- 4. SITE CONSTRUCTION MUST BE ADA COMPLIANT UNLESS SPECIFICALLY NOTED OTHERWISE. ADA ROUTES MUST HAVE LONGITUDINAL SLOPES LESS THAN 5%, AND CROSS SLOPES LESS THAN 2%. ADA RAMPS MUST HAVE A LONGITUDINAL SLOPE LESS THAN 12 H: 1V AND HAVE A LENGTH NO MORE THAN 30'. PROVIDE ADA HANDRAILS, GUARDRAILS, AND LANDINGS WHERE APPROPRIATE. ADA PARKING SPACES MUST HAVE A SLOPE LESS THAN 2% IN ANY DIRECTION.
- 5. ANY UNSUITABLE IN SITU SOIL OR MATERIAL MUST BE REMOVED OR REMEDIATED PER DIRECTION FROM THE GEOTECHNICAL ENGINEER.
- REFER TO THE SITE NOTES AND DETAILS FOR ADDITIONAL INFORMATION.

GENERAL SITE DEMOLITION NOTES

- REFER TO THE ARCHITECTURAL PLANS FOR SELECTIVE DEMOLITION RELATED TO INTERIOR RENOVATIONS. COORDINATE WITH THE DESIGN TEAM, INCLUDING THE STRUCTURAL ENGINEER, IN REGARDS TO THE STABILITY OF EXISTING STRUCTURES TO REMAIN.
- 2. THE APPROXIMATE SCALE OF ABANDONMENT AND DEMOLITION OF SITE FEATURES AND UTILITIES ARE DEPICTED BOLD OR HATCHED ON THE DEMOLITION PLAN.
- DEMOLITION OF SITE FEATURES AND UTILITIES TO REPLACE ITEMS IN KIND ARE NOT NECESSARILY SHOWN BUT ARE WITHIN THE SCOPE OF WORK.
- 4. SAWCUT EXISTING PAVEMENT TO BE REMOVED WHERE ADJACENT TO EXISTING PAVEMENT TO REMAIN. FOR CONCRETE AND GRANITE, SAWCUT AT THE NEAREST JOINT.
- ALL DEMOLITION DEBRIS MUST BE DISPOSED PER APPLICABLE LAW; DEMOLITION DEBRIS MAY ONLY BE USED FOR BACKFILL IF EXPRESS KNOWLEDGE AND PERMISSION IS GRANTED FROM THE STRUCTURAL AND GEOTECHNICAL ENGINEERS.
- 6. REFER TO THE DEMOLITION NOTES AND DETAILS FOR ADDITIONAL INFORMATION.

GENERAL SEDIMENT CONTROL NOTES

- 1. THE CONTRACTOR MUST NOTIFY THE FAIRFAX COUNTY INSPECTOR BEFORE MAKING ANY ADJUSTMENTS IN REGARDS TO THE LIMITS OF DISTURBANCE AND SEDIMENT CONTROL MEASURES TO PERFORM THE WORK AND ACCOMMODATE FIELD CONDITIONS.
- 2. WHERE NO STABILIZED CONSTRUCTION IS PROVIDED CONTRACTOR SHALL DESIGNATE VEHICLES THAT SHALL ENTER THE SITE. ALL VEHICLES LEAVING THE SITE MUST HAVE THEIR TIRES/TREADS WASHED PRIOR TO ENTERING ANY PUBLIC STREETS. WASH WATER MUST NOT BE ALLOWED TO LEAVE THE SITE.
- 3. THE LIMITS OF DISTURBANCE AND SEDIMENT CONTROL MEASURES ARE SHOWN APPROXIMATELY: PRESENTATION ON THE PLANS MAY DEVIATE SLIGHTLY FROM THE ACTUAL DESIGN INTENT FOR GRAPHICAL CLARITY.
- TEMPORARY SOIL STOCKPILES SHOULD BE PLACED AS NEEDED ON THE SITE IN COORDINATION WITH THE FAIRFAX COUNTY. INSTALL SILT FENCE AROUND THE PERIMETER OF ALL STOCKPILES AND COVER WITH A TARP OR OTHER APPROVED IMPERMEABLE SURFACE PRIOR TO RAIN EVENTS.
- 5. THE CONTRACTOR SHALL PROVIDE INLET PROTECTION FOR ALL CATCH BASINS, CURB INLETS, DRAINS, AND RISER STRUCTURES ON OR ADJACENT TO THE LIMITS OF DISTURBANCE. ANY SEWER WHICH BECOMES CLOGGED DUE TO CONSTRUCTION MUST BE PROMPTLY CLEANED AND CLEARED.
- 6. ANY AND ALL SITE STORM RUNOFF FROM DISTURBED AREAS MUST BE FILTERED OR OTHERWISE TREATED TO REMOVE SEDIMENT PRIOR TO LEAVING THE SITE. SEDIMENT MUST BE PLACED IN AN APPROVED AREA AND STABILIZED. SEDIMENT MUST NOT BE PLACED IN A FLOODPLAIN, WETLAND, WITHIN THE CRITICAL ROOT ZONE OF AN EXISTING TREE TO REMAIN. OR RPA.
- 7. NO EXISTING TREES ARE TO BE REMOVED AS PART OF THIS PROJECT, EXISTING TREES SHALL BE PROTECTED AS NEEDED AND REQUIRED BY FAIRFAX COUNTY WITH TREE PROTECTION FENCE. SEE FAIRFAX COUNTY PLATE 6-12 ON SHEET
- 8. MINIMIZE DUST GENERATION DURING CONSTRUCTION
- 9. REFER TO THE SEDIMENT CONTROL NOTES AND DETAILS FOR ADDITIONAL INFORMATION.

ADDDEVIATIONS

ABBREVI	<u>ATIONS</u>		
ABND AD	ABANDONED AREA DRAIN	MH MIN	MANHOLE MINIMUM
ADA	AMERICANS WITH	MS	MINIMUM STANDARD
	DISABILITIES ACT	NRCS	NATURAL RESOURCES
APPROX	APPROXIMATE	00	CONSERVATION SERVICE
BFP	BACKFLOW PREVENTER	OC	ON CENTER
BLDG	BUILDING	PFM	PUBLIC FACILITIES
BRL	BUILDING RESTRICTION	DI	MANUAL
BSMT	LINE BASEMENT	PL DDOD	PROPERTY LINE
BW	BOTTOM OF WALL	PROP RPA	PROPOSED RESOURCE PROTECTION
CI	CAST IRON	KFA	AREA
CO	CLEANOUT	SAN	SANITARY
CS	COMBINED SEWER	SCH	SCHEDULE
DEQ	VIRGINIA DEPARTMENT OF	STM	STORM
DLQ	ENVIRONMENTAL QUALITY	SWR	SEWER
ELEV	ELEVATION	TC	TOP OF CURB
EX	EXISTING	TW	TOP OF WALL
FFE	FIRST FLOOR ELEVATION	VB	VERTICAL BEND
НВ	HORIZONTAL BEND	VCP	VITRIFIED CLAY PIPE
HSG	HYDROLOGIC SOIL GROUP	W/	WITH
MAX	MAXIMUM	W/M	WATERMAIN
MEP	MECHANICAL	WW	WINDOW WELL
	ELECTRICAL/PLUMBING		

WETLANDS PERMIT CERTIFICATION:

I HEREBY CERTIFY THAT ALL WETLANDS PERMITS REQUIRED BY LAW WILL BE OBTAINED PRIOR TO COMMENCING LAND DISTURBING ACTIVITIES. EMRE EIREKOGU SIGNATURE: EMRE ZIREKOGLU Manager OWNER/DEVELOPER:

RESPONSIBLE LAND DISTURBER:

PHONE NO.: ADDRESS:

CERT. NO.:

Natural Resources Conservation Service

ZONING INFORMATION 1.16 AC (50,778 SF) LOT AREA **EXISTING ZONING:** PROPOSED ZONING PROPOSED USE **TOWNHOUSES** WATERSHED: ACCOTINK CREEK DISTURBANCE: 1.20 ACRES SEWER: **PUBLIC** WATER: PUBLIC MAXIMUM DENSITY (RT) 12 UNITS/ACRE PROPOSED TOWNHOME UNITS PROPOSED DENSITY 11.21 UNITS/ACRE OFF-STREET PARKING REQ. (RT) 2 SPACES/UNIT TOTAL SPACES REQ. (RT) 26 SPACES PROPOSED SPACES 26 SPACES LAND USE ACTIONS/GRANTED **ZONING INTERPRETATION -**TOWNHOMES TO FRONT PENDING APPROVAL PRIVATE STREET SPECIAL EXCEPTION - NO 10-FT PENDING APPROVAL LANDSCAPE STRIP ALONG PRIVATE STREET PFM WAIVER - 24' PRIVATE PENDING APPROVAL STREET WIDTH OPEN SPACE REQ. NONE OPEN SPACE PROVIDED NONE TYPE OF CONSTRUCTION PENDING

PARK RD TOWNHOUSE **REZONING PLANS**

LOCATION OF SITE 11004 & 11006 PARK RD FAIRFAX, VA 22306 TAX MAP #57-1-40-002 D.B. 27365, P.G. 1623 SQUARE 02 LOT 002

SHEET TITLE	SHEET#
COVER SHEET	000
EXISTING CONDITIONS	001
SITE PLAN	002
GRADING PLAN	003
FIRE PLAN	004
SITE DETAILS	005
FAIRFAX CITY DPW DETAILS	006
UTILITY PLAN	007
SANITARY SEWER CAPACITY ANALYSIS	800
FAIRFAX WATER DETAILS	009
UTILITY DETAILS	010
STORM MAIN CROSS SECTIONS	011
STORMWATER MANAGEMENT PLAN	012
STORMWATER MANAGEMENT CALCULATIONS	013
BAYFILTER DETAILS	014
DRAINAGE PLAN	015
DRAINAGE PLAN CALCULATIONS	016
TREE SURVEY	017
LANDSCAPE PLAN	018
LANDSCAPE DETAILS	019

VICINITY MAP Shirley Gate Park

1050 30TH STREET, NW WASHINGTON, DC 20007 703.425.3862

LAND SURVEYOR DOMINION SURVEYS, INC. ALEXANDRIA, VA 22309 703.619.6555

USDA-NRCS SOIL MAP



Web Boil Burvey National Cooperative Boil Survey

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
52B	Hattontown-Haymarket complex, 2 to 7 percent slopes	0.4	6.2%
95	Urban land	3.0	42.7%
105B	Wheaton - Glenelg complex, 2 to 7 percent slopes	1.3	18.3%
105C	Wheaton - Glenelg complex, 7 to 15 percent slopes	1.5	21.8%
107B	Wheaton - Meadowville complex, 2 to 7 percent slopes	0.8	11.0%
Totals for Area of Interest		7.0	100.0%

APPROVAL REVISIONS INITIAL SUBMISSION 03/04/2022 08/25/2022 SECOND SUBMISSION THIRD SUBMISSION 12/16/2022



FAIRFAX, VA 22306 TAX MAP #57-1-40-002 SQUARE 02, LOT 002

11004 & 11006 PARK RD

CLIENT EMRE ZIREKOGLU CAGLAYAN INVESTMENT GROUP 32713 LATROBE ST CHANTILLY, VA 20152 571.594.6363

CONTRACTOR

CIVIL ENGINEER

PATRICK HORGAN HUSKA CONSULTING, LLC

8808-H PEAR TREE VILLAGE COURT

Lic. No. 061930

COVER SHEET

DRAWING TITLE

EXISTING CONDIT	IONS PLAN LEGEND		
	PROPERTY LINE	G	GAS LINE
<u> </u>	BUILDING FACE	o GM	GAS METER
	DOOR	©V	GAS VALVE
	WALL	OHU	OVERHEAD UTILITY
xx	WOOD FENCE	12" CS	COMBINED SEWER
-0	IRON FENCE	12" SAN	SANITARY SEWER
	HANDRAIL	12" STM	STORM SEWER
	CURB AND GUTTER	S	COMBINED/SANITARY MANHOLE
•	BOLLARD		STORM SEWER MANHOLE
	ASPHALT PAVEMENT	⊕ OR	STORM DRAIN
	BRICK PAVEMENT	°CO	CLEANOUT
	CONCRETE PAVEMENT	°DS	DOWNSPOUT
	WOOD/LUMBER DECK	COMM	TELECOMMUNICATIONS
× 314.1	SPOT ELEVATION	\bigcirc	TELECOMMUNICATIONS MANHOLE
	CONTOUR	*	LIGHT POLE/STREET LIGHT
	TREE W/ CRZ & SRZ	¤	UTILITY POLE
	CIVE & SIVE	-•	GUY WIRE
-0-	TRAFFIC SIGN	8" DIP w	WATER LINE
——Е——	ELECTRIC LINE	-Ò ^{FH}	FIRE HYDRANT
E	ELECTRIC MANHOLE	♥ FDC	FIRE DEPT. CONN.
°EM	ELECTRIC METER	WM	WATER METER
EV	ELECTRIC VAULT	(WV)	WATER VALVE

	EAS	SEMENT INF	ORMATION TABLE	
EASEMENT#	TYPE	WIDTH (FT)	METES & BOUNDS OF CENTERLINE	DB & PG. #
1	STORM SEWER	10	N 89°30'46" W ~ 125.38'	DB 6827, PG 1808
2	SANITARY SEWER & WATER LINE	15	N 89°30'46" W ~ 125.38'	DB 1192, PG 94
3	SANITARY SEWER	10	N 01°00'00" W ~ 203.62'	DB 2765, PG1623
4	SANITARY SEWER	15	S 81°03'02" W ~ 18.93' N 06°40'40" W ~ 3.37'	DB 2765, PG1623
5	SANITARY SEWER	20	S 81°03'02" W ~ 18.93'	DB 3808, PG 269
6	SIDEWALK	6.5		DB 6550, PG 1184
7	STORM SEWER	10	S 88°47'52"E	DB 6550, PG 1190

SANITARY SEWER STRUCTURES

TOP= 451.18 INV.OUT= 438.64 TO SOUTH INV.IN= 439.48 FROM B

TOP=462.95 INV.OUT=450.36 TO E

TOP=466.32 INV.OUT=454.67 TO E INV.IN=454.91 FROM NORTH

TOP=452.21 INV.OUT=443.51 TO F INV.IN=443.56 FROM C INV.IN.=443.90 FROM D TOP=448.02 INV.OUT=441.67 TO G INV.IN= 441.82 FROM SOUTHEAST INV.IN= 442.37 FROM SOUTHEAST INV.IN= 442.22 FROM E

STORM SEWER STRUCTURES

TOP=466.61 INV.OUT=450.72 TO 7 TOP=448.82 INV. OUT=443.14 TO 2

TOP=436.45 INV. OUT=433.04 TO SOUTH INV. IN=433.14 FROM 2 INV. IN=433.15 FROM NORTH

TOP=439.44 INV.OUT=435.42 INV.IN=435.23 FROM 6 INV.IN=435.16 FROM NORTHWEST

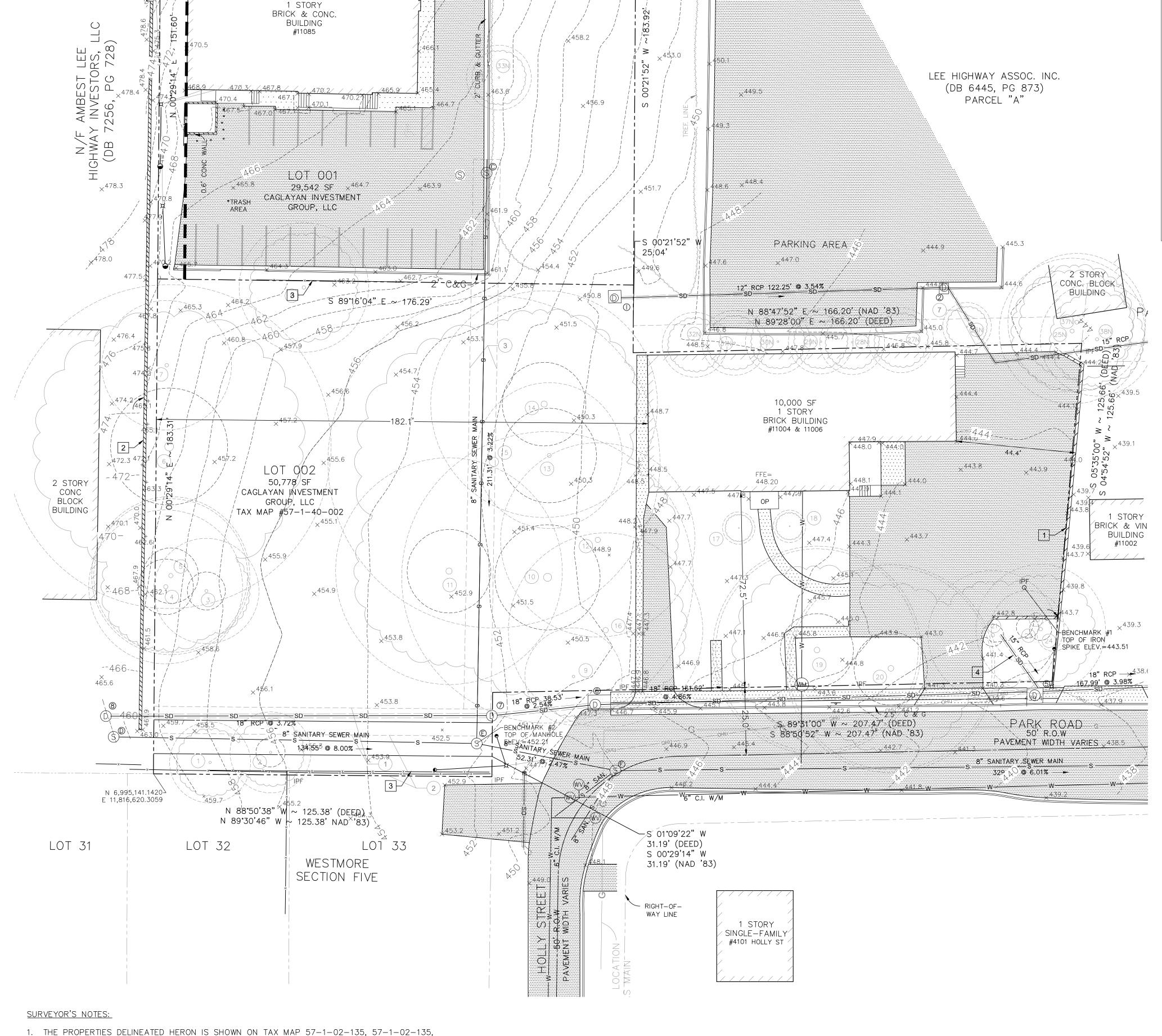
TOP=447.61 INV.OUT=443.08 TO 5 INV.IN=443.42 FROM 7 TOP=452.59 INV.OUT=444.40 TO 6 INV.IN=445.40 FROM WEST

EXISTING CONDITIONS PLAN KEYNOTES 1 EXISTING 1-FT WIDE CONCRETE WALL MAXIMUM HEIGHT: 4'=0"

2 EXISTING 1-FT WIDE CONCRETE WALL MAXIMUM HEIGHT: 10'=0"

3 WOOD FENCE HEIGHT: 6'-0"

4 UPSTREAM CONNECTION UNKNOWN. 15" RCP STORM MAIN IS ASSUMED TO COLLECT RUNOFF FROM THE EXISTING BUILDING ON LOT 002 AND THE ADJACENT PARKING LOT. CONTRACTOR TO FIELD VERIFY PRIOR TO DEMOLITION.



- 57-1-02-136, 57-1-02-137A &57-1-02-138B AND ARE ZONED C-2 COMMERCIAL.
- 2. OWNER: CAGLAYAN INVESTMENT GROUP, LLC CHANTILLY VIRGINIA 20152 DB. 25288, PG 1940, DB. 25288, PG. 1942 AND DB. 26229, PG. 2180
- 3. NO TITLE REPORT FURNISHED.
- 4. THESE PROPERTIES ARE SUBJECT TO RESTRICTIONS OF RECORD.
- 5. HORIZONTAL DATUM IS REFERENCED TO NAD '83. VERTICAL DATUM IS REFERENCED TO NGVD '29.
- 6. THESE PROPERTIES ARE NOT LOCATED WITHIN A RESOURCE PROTECTION AREA.
- 7. FENCES ARE CHAIN LINK UNLESS NOTED.
- 8. TOTAL AREA= 81,154 SQUARE FEET.

FLOODPLAIN CERTIFICATE

I HEREBY CERTIFY THAT THE PROPERTY IS NOT WITHIN 500 FEET OF A DELINEATED OR KNOWN FLOODPLAIN PER THE FEDERAL EMERGENCY MANAGEMENT AGENCY FLOOD MAP #51059C0040E.

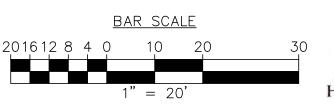
DATE

PATRICK HORGAN, P.E. LICNESE NO. 061930

APPROVAL	DATE	REVISIONS	
	03/04/2022	INITIAL SUBMISSION	
	08/25/2022	SECOND SUBMISSION	
	12/16/2022	THIRD SUBMISSION	

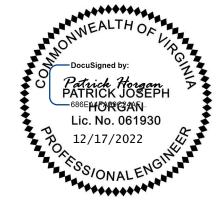
EXISTING CONDITIONS PLAN NOTES

- 1. THIS EXISTING CONDITIONS PLAN IS BASED ON A SURVEY AND AUTOCAD FILES PERFORMED AND PROVIDED BY DOMINION ENGINEERS, INC..
- 2. THE EXISTING CONDITIONS LEGEND IS APPLICABLE TO THIS SHEET ONLY. THE EXISTING CONDITIONS MAY BE DEPICTED DIFFERENTLY (GRAY SCALED) OR NOT FULLY DEPICTED ON OTHER SHEETS.
- 3. THE LOCATIONS AND DEPTHS OF EXISTING UTILITIES ARE APPROXIMATE AND BASED ON AVAILABLE RECORDS AND, WHERE INFORMATION IS NOT AVAILABLE, ASSUMPTIONS. CONTRACTOR SHALL LOCATE AND CONFIRM ALL UTILITIES WITHIN THE BOUNDS OF CONSTRUCTION PRIOR TO UNDERTAKING ANY DEMOLITION OR EXCAVATION.





NOT FOR CONSTRUCTION **REZONING PLANS** 12/16/2022



11004 & 11006 PARK RD FAIRFAX, VA 22306 TAX MAP #57-1-40-002

SQUARE 02, LOT 002

EMRE ZIREKOGLU

32713 LATROBE ST CHANTILLY, VA 20152

571.594.6363

CONTRACTOR

CIVIL ENGINEER PATRICK HORGAN

703.425.3862

703.619.6555

LAND SURVEYOR

HUSKA CONSULTING, LLC

DOMINION SURVEYS, INC.

ALEXANDRIA, VA 22309

8808-H PEAR TREE VILLAGE COURT

1050 30TH STREET, NW WASHINGTON, DC 20007

CAGLAYAN INVESTMENT GROUP

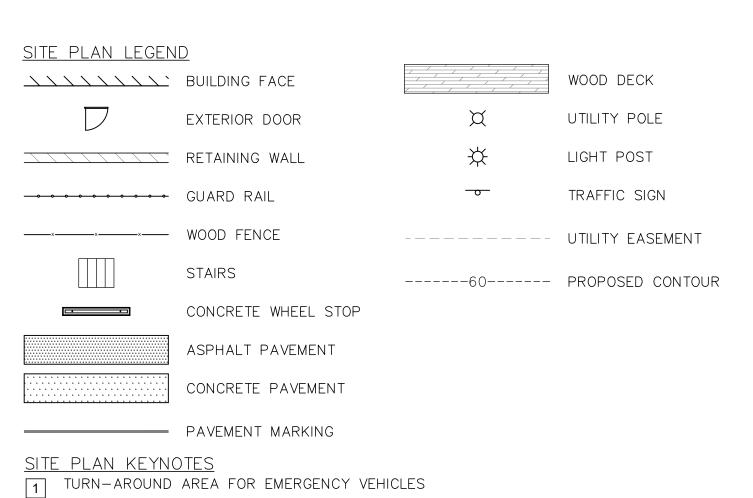
CLIENT

EXISTING CONDITIONS

DRAWING TITLE

DRAWING NO.

001



- 2 LOTS 01-07 TO HAVE 21'X34' TOWNHOMES WITH TWO CAR GARAGE
- 3 LOTS 10 & 11 TO HAVE 21'X39' TOWNHOMES WITH ONE CAR GARAGE
- 4 LOTS 08-09 & 12-13 TO HAVE 24'X36' TOWNHOMES WITH TWO CAR GARAGE
- 5 TWO 8'X22' PARALLEL PARKING SPACES RESERVED FOR LOTS 10-11
- PRIVATE ROADS, SIDEWALKS, PLAYGROUND, PARKING AREA, STORM UTILITIES, AND STORMWATER MANAGEMENT FACILITIES TO BE OWNED AND MAINTAINED BY FUTURE HOMEOWNERS ASSOCIATION.
- 5-FT CONCRETE SIDEWALK TO BE PROVIDED ADJACENT TO PARK RD ALONG PROPERTY FRONTAGE SETBACK 3-FT FROM CURB (DWP STD. 404.01). PROVIDE 3-FT WIDE GRASS STRIP BETWEEN SIDEWALK AND CURB
- 8 24-FT WIDE COMMERICAL ENTRANCE LOCATED IN HOLLY ST AND PARK RD INTERSECTION (DWP STD. 404.06)
- 9 24-FT WIDE PRIVATE ASPHALT ROAD (DWP STD. 401.01) REQUIRES VARIANCE TO
- MINIMUM WIDTH. 2-FT WIDE CONCRETE CURB AND GUTTER (VDOT STD. 201.03) 10 ADA COMPLIANT CONCRETE CURB RAMP (DWP STD. 404.04), TYPICAL
- FLARE SLOPE: 1:10 MAX (H:V) OVERHEAD BAY WINDOW PROJECTION NOT TO EXCEED 3-FT, TYPICAL
- 12 NEW STREET SIGN DENOTING PRIVATE STREET, NAME PENDING
- TRASH AND RECYCLING CONTAINERS TO BE STORED IN GARAGES, TYPICAL FOR ALL LOTS
- ADA COMPLIANT 5-FT CONCRETE SIDEWALK BUILT AGAINST BACK OF CURB (DWP STD. 404.01) TO BE PROVIDED ON BOTH SIDES OF PROPOSED PRIVATE STREET CROSS SLOPE NOT TO EXCEED 2.0%
- ADA COMPLIANT CONCRETE RAMP TO PROVIDE ACCESS TO LOT 01 AND FUTURE ACCESS TO LEE HIGHWAY. 42" ACCESSIBLE WIDTH
 - MAX. LONGITUDINAL SLOPE: 1:12 (H: V) CROSS SLOPE: 0.0%
 - GUARDRAIL/HANDRAIL TO BE PROVIDED ON BOTH SIDES
- PROPOSED LOCATION OF NEW DOMINION TRANSFORMER 6'X6' CONCRETE PAD WITH SURROUNDING WOOD FENCE AND DENSE HEDGE
- WOOD FENCE AND HEDGE HEIGHT TO BE TALL ENOUGH TO SCREEN THE EQUIPMENT FROM PARK RD AND ADJACENT LOT 01 PROPOSED NEW POWER FED TO SITE FROM EXISTING UTILITY POLE ON ADJACENT
- LOT 001 (SAME OWNER AS LOT 002) EXACT ALIGNMENT AND DESIGN TO BE COMPLETED DURING SITE PLAN REVIEW
- 18 6' WIDE ADA COMPLIANT CROSS WALK WITH MARKINGS

DESIGN TO BE PROVIDED DURING SITE PLAN REVIEW.

- MUTCD STOP SIGN (RT-1 30"X30") FOR SOUTHBOUND TRAFFIC ON PRIVATE ROAD AT PARK RD AND HOLLY ST INTERSECTION. SET BACK MINIMUM 4' FROM PROPOSED CROSSWALK, FINAL PLACEMENT AND
- NEW REFLECTIVE ROAD SIGN FOR NORTHBOUND TRAFFIC ON HOLLY ST: "WARNING PARK RD THRU TRAFFIC DOES NOT STOP"
- FINAL PLACEMENT AND DESIGN TO BE PROVIDED DURING SITE PLAN REVIEW.
- AREA RESERVED FOR NEW PLAYGROUND/TOT LOT, FINAL DESIGN TO BE DETERMINED DURING SITE PLAN REVIEW.
- THREE 9'X18' STANDARD PARKING SPACES FOR USE BY GUEST
- EXISTING DOMINION UTILITY POLE TO BE RELOCATED TO ALLOW FOR NEW COMMERCIAL ENTRANCE CONCRETE SIDEWALK AND ADA CONCRETE RAME COMMERCIAL ENTRANCE, CONCRETE SIDEWALK, AND ADA CONCRETE RAMP. UTILITY POLE SHALL BE RELOCATED PER CURRENT DOMINION POWER STANDARDS AND SPECIFICATIONS. FINAL DESIGN AND LOCATION TO BE DETERMINED DURING SITE PLAN REVIEW.

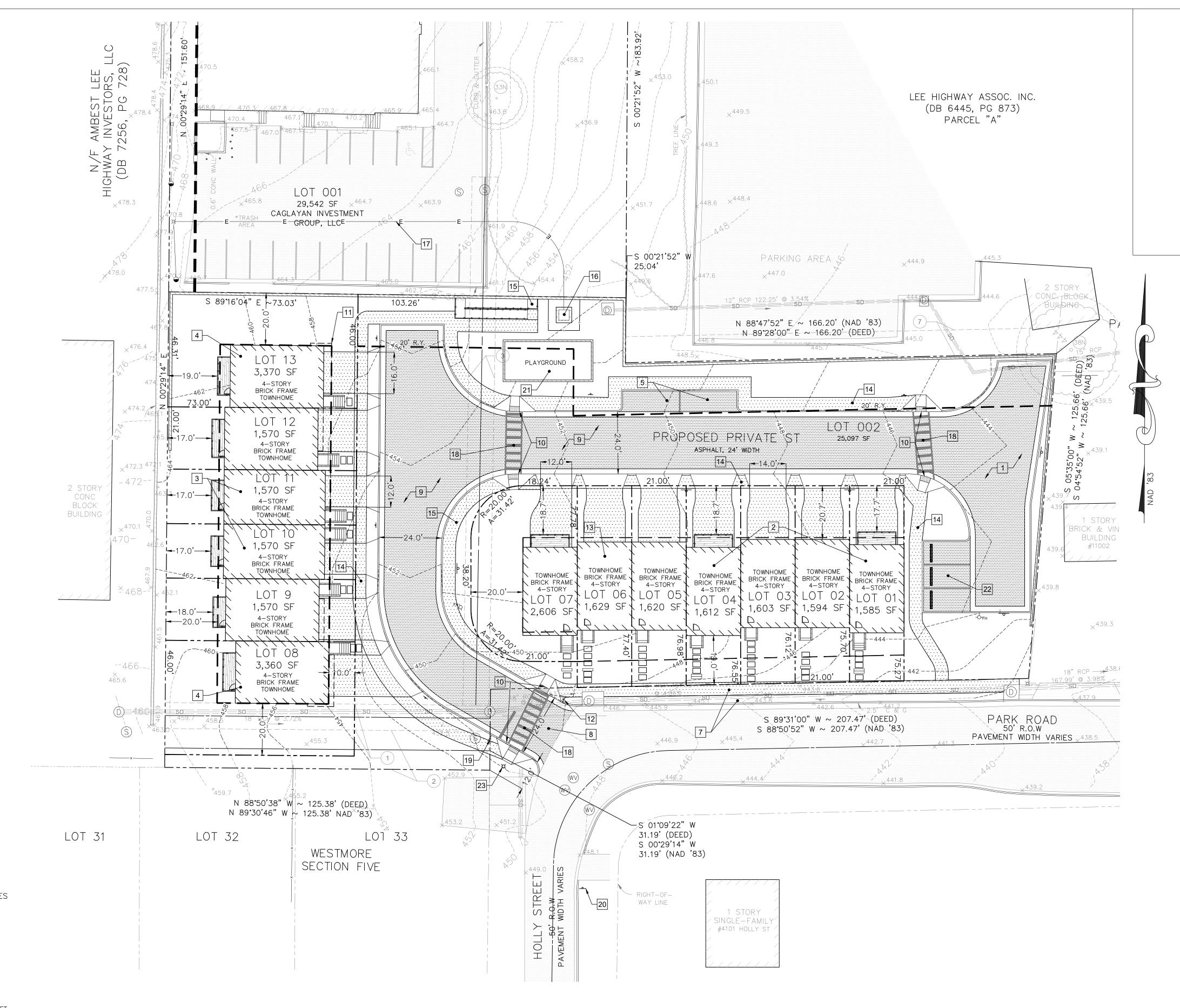
	EQUIREMENTS: RE TOWNHOUSE (RT)	
ZONING STANDARD	ALLOWED	PROPOSED
MINIMUM LOT SIZE	1,500 SF	1,585 SF
SITE AREA (ACRE)	0.4 ACRE OR 17,424 SF	1.16 ACRE OR 50,778 SF
MAXIMUM LOT COVERAGE	80%	74.70 %
MAXIMUM BUILDING COVERAGE	60%	53.41%
MAXIMUM BUILDING HEIGHT		
ADJACENT TO RESIDENTIAL USE	35-FT/3-STORY	N/A
ADJACENT TO COMMERCIAL USE	45-FT/4-STORY	45-FT/4-STORY
SETBACKS		
FRONT:	10 FEET	10 FEET (MIN.)
SIDE:	20 FEET TO STREET 0 FEET INTERIOR	20 FEET 0 FEET
REAR:	20 FEET	20 FEET (MIN.)
DENSITY	12 UNIT/ACRE	11.20 UNIT/ACR
MINIMUM LOT WIDTH	18 FEET	21.50 FEET (MIN

WIDE DRIVEWAYS CONNECT TO A 1-CAR GARAGE

OFF-STREET PARKING C	<u>ALCULATIONS</u>
USE:	RESIDENTIAL, TOWNHOUSES
OFF—STREET PARKING REQUIRED:	2.0/UNIT
PROPOSED UNITS:	13
TOTAL OFF-STREET SPACES REQUIRED:	26
TOTAL OFF-STREET SPACES PROVIDED:	26*
*16-FT WIDE DRIVEWAYS CONT	NECT TO A 2-CAR GARAGE, 12-FT

REFUSE DISPOSAL NOTE

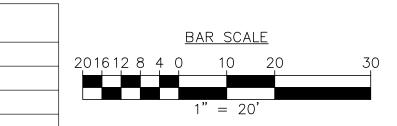
REFUSE AND RECYCLING SHALL BE STORED ON EACH INDIVIDUAL LOT AND COLLECTED WEEKLY BY PRIVATE REFUSE DISPOSAL COMPANY.



APPROVAL

SITE PLAN NOTES

- 1. ALL EXISTING FEATURES ARE NOT NECESSARILY SHOWN ON THIS PLAN. SEE EXISTING CONDITIONS PLAN.
- 2. THIS PLAN IS TO DEPICT WORK ON PRIVATE PROPERTY ONLY. NO WORK IS PROPOSED IN PUBLIC SPACE.
- 3. SPOT SHOTS ARE SHOWN PURPOSEFULLY OFFSET 0.5' FROM THE SPOT DESCRIBED FOR VISUAL CLARITY, MOREOVER, SPOTS ARE ROUNDED TO THE NEAREST 5 HUNDREDTHS.
- 4. REFER TO THE CIVIL COVER SHEET FOR ADDITIONAL INFORMATION.



REVISIONS

INITIAL SUBMISSION

THIRD SUBMISSION

SECOND SUBMISSION

03/04/2022

08/25/2022

12/16/2022



NOT FOR CONSTRUCTION REZONING PLANS 12/16/2022

12/17/2022

SITE PLAN

PATRICK JOSEPH

—686**|E|440F|R**9**93:A|4N|**F.. Lic. No. 061930

11004 & 11006 PARK RD

FAIRFAX, VA 22306

TAX MAP #57-1-40-002

SQUARE 02, LOT 002

EMRE ZIREKOGLU

32713 LATROBE ST

571.594.6363

CONTRACTOR

CIVIL ENGINEER

703.425.3862

703.619.6555

PATRICK HORGAN

LAND SURVEYOR

HUSKA CONSULTING, LLC

1050 30TH STREET, NW

WASHINGTON, DC 20007

DOMINION SURVEYS, INC.

ALEXANDRIA, VA 22309

8808-H PEAR TREE VILLAGE COURT

CHANTILLY, VA 20152

CAGLAYAN INVESTMENT GROUP

CLIENT

DRAWING TITLE

002

<u>SITE DISTANCE LEGEND</u>

SIGHT DISTANCE/TRAVEL EYE OF VEHICLE ENTERING INTERSECTION OBJ. O OBJECT ALONG PATH

OF TRAVEL

INTERSECTION SIGHT DISTANCE

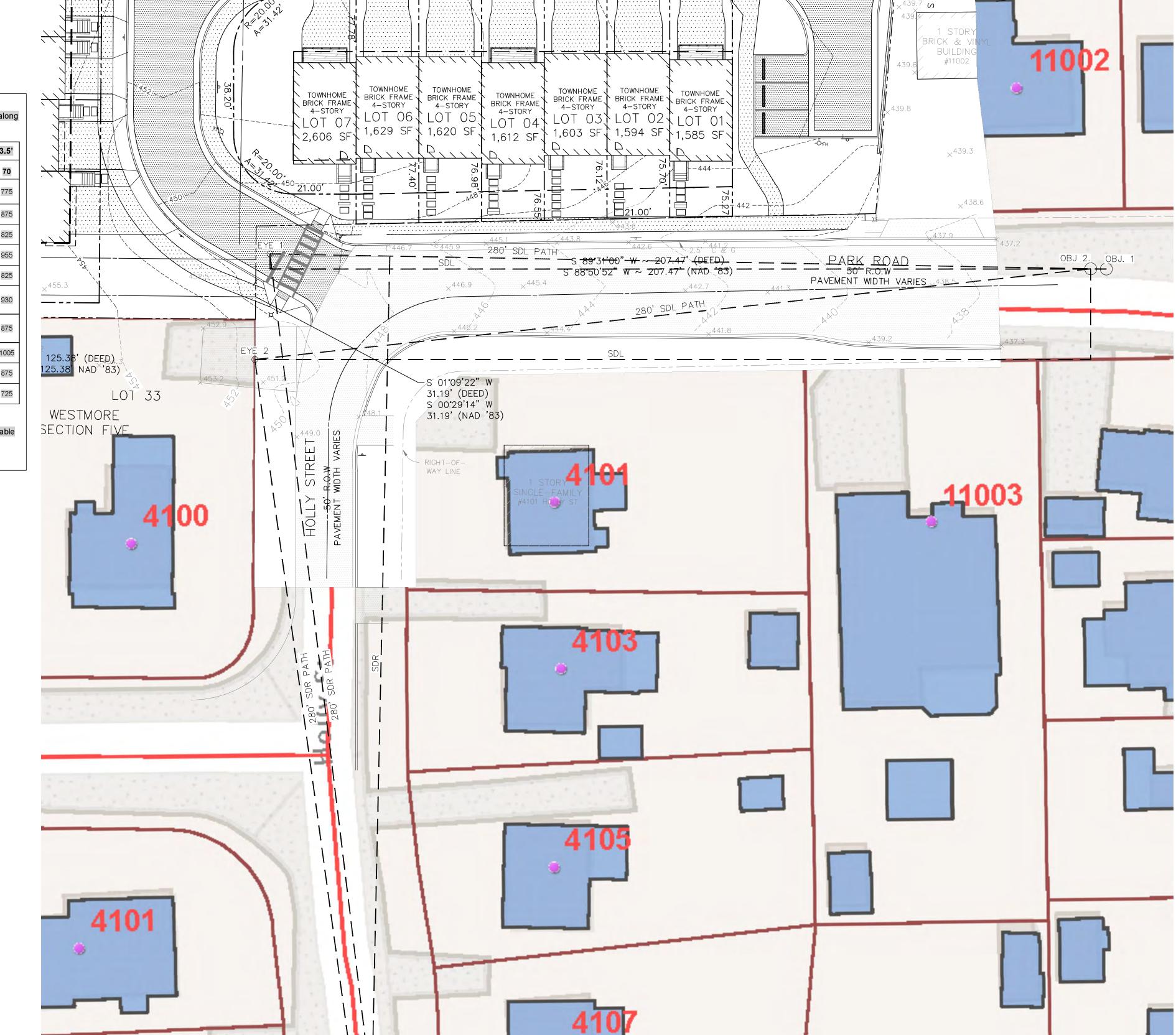
The following table shows intersection sight distance requirements for various speeds along major roads: *

Height of Eye 3.5'								H	leigh	t of O	bject	3.5
Design Speed (mph)*	*	20	25	30	35	40	45	50	55	60	65	70
SDL=SDR: 2 Lane Major Road		225	280	335	390	445	500	555	610	665	720	775
SDR: 4 Lane Major Road (Undivided) or 3 Lane		250	315	375	440	500	565	625	690	750	815	875
SDL: 4 Lane Major Road (Undivided) or 3 Lane		240	295	355	415	475	530	590	650	710	765	825
SDR: 4 Lane Major Road (Divided – 18' Median)		275	340	410	480	545	615	680	750	820	885	955
SDL: 4 Lane Major Road (Divided – 18' Median)	Feet	240	295	355	415	475	530	590	650	710	765	825
SDR: 5 Lane Major Road (continuous two-way turn- lane)	In Fe	265	335	400	465	530	600	665	730	800	860	930
SDL: 5 Lane Major Road (continuous two-way turn- lane)		250	315	375	440	500	565	625	690	750	815	875
SDR: 6 Lane Major Road (Divided – 18' Median)		290	360	430	505	575	645	720	790	860	935	1005
SDL: 6 Lane Major Road (Divided – 18' Median)		250	315	375	440	500	565	625	690	750	815	875
SDL: (Where left turns are physically restricted)		210	260	310	365	415	465	515	566	620	670	725

TABLE A1-3 INTERSECTION SIGHT DISTANCE Source: 2018 AASHTO Green Book, Chapter 9, Section 9.5.3, page 9-37 thru 9-52, Table

**For all tables, use design speed if available, if not use legal speed.

9-6 thru 9-17



APPROVAL	DATE	REVISIONS	
	03/04/2022	INITIAL SUBMISSION	20
	08/25/2022	SECOND SUBMISSION	
	12/16/2022	THIRD SUBMISSION	





NOT FOR CONSTRUCTION REZONING PLANS 12/16/2022

SITE DISTANCE **EXHIBIT**

11004 & 11006 PARK RD FAIRFAX, VA 22306 TAX MAP #57-1-40-002 SQUARE 02, LOT 002

CLIENT

EMRE ZIREKOGLU

32713 LATROBE ST

571.594.6363

CONTRACTOR

CIVIL ENGINEER

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703.619.6555

PATRICK HORGAN

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DOMINION SURVEYS, INC.

ALEXANDRIA, VA 22309

8808-H PEAR TREE VILLAGE COURT

CHANTILLY, VA 20152

CAGLAYAN INVESTMENT GROUP

DRAWING TITLE

DRAWING NO.

002A

APPROVAL

DATE

03/04/2022

08/25/2022

12/16/2022

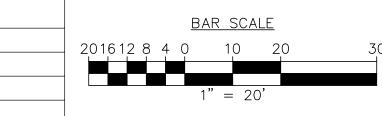
REVISIONS

INITIAL SUBMISSION

THIRD SUBMISSION

SECOND SUBMISSION

- DESCRIBED FOR VISUAL CLARITY. MOREOVER, SPOTS ARE ROUNDED TO THE NEAREST 5 HUNDREDTHS.
- 3. REFER TO THE CIVIL COVER SHEET FOR ADDITIONAL INFORMATION.





NOT FOR CONSTRUCTION REZONING PLANS 12/16/2022

GRADING PLAN

Patrick Horgan PATRICK JOSEPH 686FHORGAN

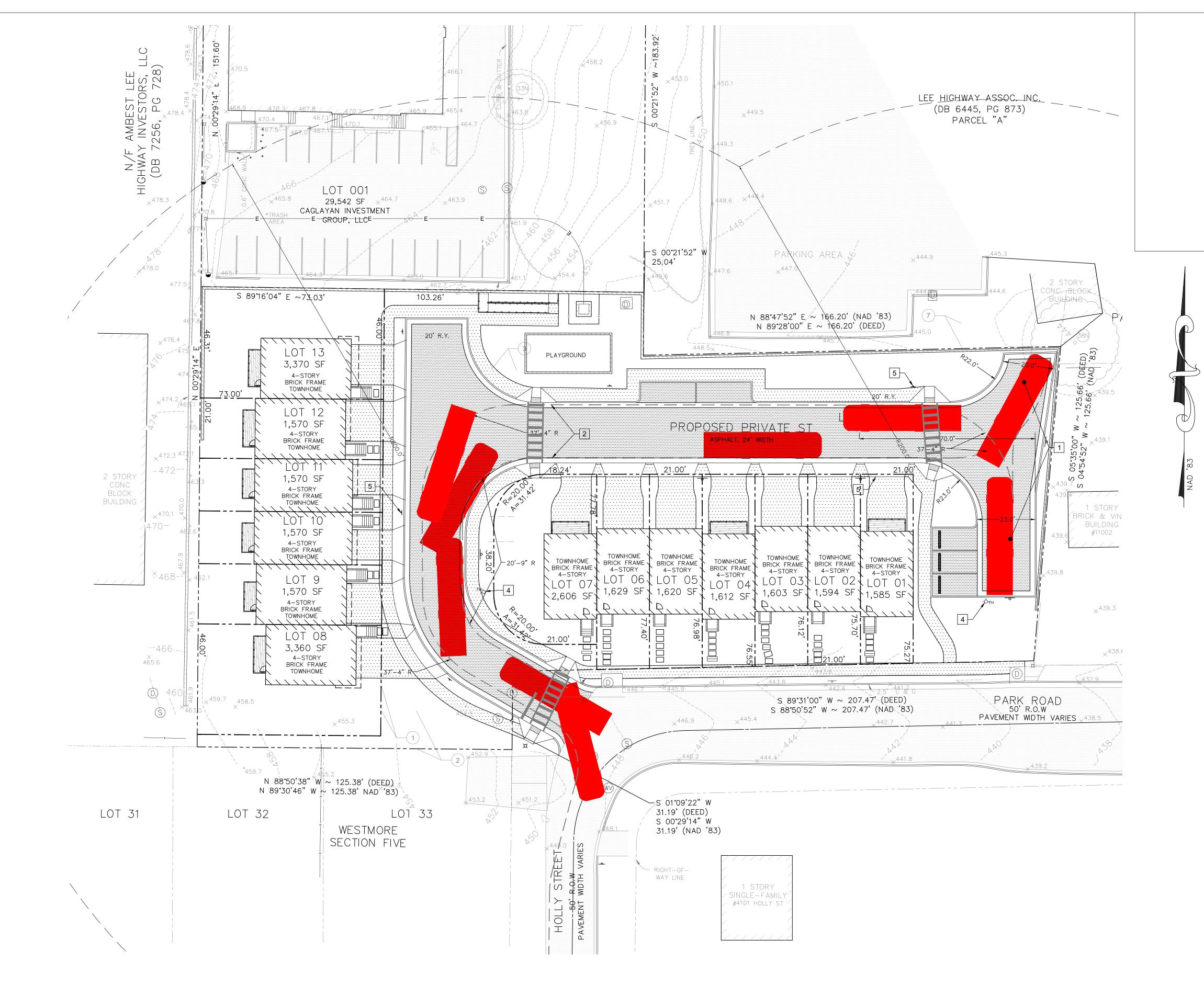
Lic. No. 061930

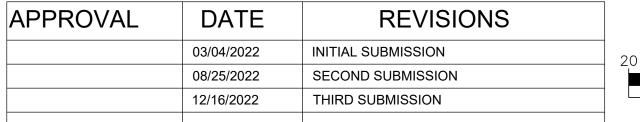
DRAWING TITLE 003

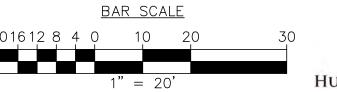
10'-2"

20'-9"

82.92"









NOT FOR CONSTRUCTION REZONING PLANS 12/16/2022

FIRE PLAN DRAWING TITLE

Patrick Horgan PATRICK JOSEPH 686EHORGAN Lic. No. 061930

11004 & 11006 PARK RD

FAIRFAX, VA 22306 TAX MAP #57-1-40-002

SQUARE 02, LOT 002

EMRE ZIREKOGLU

32713 LATROBE ST

571.594.6363

CONTRACTOR

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703.425.3862

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LAND SURVEYOR

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1050 30TH STREET, NW

WASHINGTON, DC 20007

DOMINION SURVEYS, INC.

ALEXANDRIA, VA 22309

8808-H PEAR TREE VILLAGE COURT

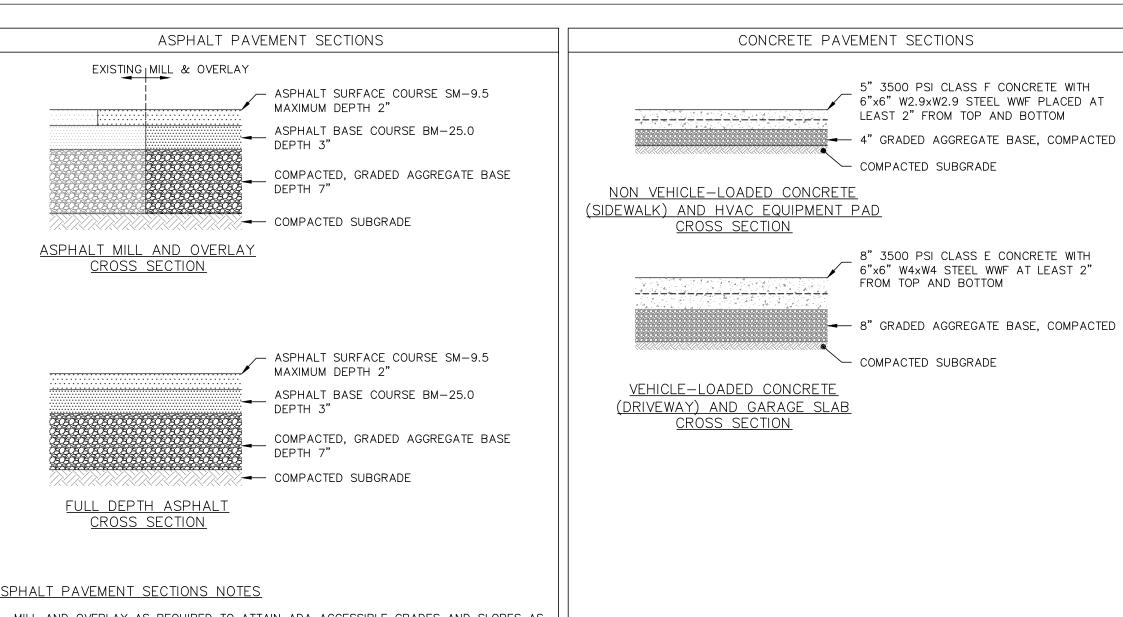
CHANTILLY, VA 20152

CAGLAYAN INVESTMENT GROUP

CLIENT

TBD

004



AND BETWEEN MILL AND OVERLAY AND EXISTING PAVEMENT SECTIONS.

- CONTRACTOR TO REFER TO THE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL & THE VIRGINIA CONSTRUCTION CODE FOR CONSTRUCTION METHODS AND MATERIALS.
- CONCRETE REINFORCEMENT MUST BE PLACED AT LEAST 2" FROM CONCRETE SURFACES AND EDGES. UTILIZE REINFORCEMENT STANDS IF REQUIRED.
- PROVIDE A LIGHT BROOM FINISH ON THE CONCRETE SURFACE.

CONCRETE JOINTS CONTROL JOINT MUST BE AT LEAST 1/4" WIDE AND THE MINIMUM DEPTH IS 1/4" OF THE CONCRETE LAYER IN QUESTION CONCRETE CONTROL JOINT <u>CROSS SECTION</u> · ¼" CHAMFER OR FILLET VEHICLE-LOADED CONCRETE EXPANSION JOINT CROSS SECTION

CONCRETE JOINTS NOTES

CONTROL JOINTS SHOULD BE SPACED EQUAL TO THE WIDTH OF THE PAVEMENT IN QUESTION TO FORM SQUARES. HOWEVER, CONTROL JOINTS SHOULD BE PLACED NO MORE THAN 6' APART.

EXPANSION JOINTS SHOULD BE PLACED NO MORE THAN 30' APART AND BE 1/2" WIDE.

- EXPANSION JOINTS SHOULD BE PLACED WHERE CONCRETE PAVEMENT ABUTS A STRUCTURE, WALL, COLUMN, FOOTING, OR CURB. EXPANSION JOINTS SHOULD CONSIST OF SEALED CORK, ASPHALT IMPREGNATED FIBER
- SHEETING, ISO STRIP OFF, OR APPROVED EQUIVALENT. DOWELS SHOULD BE GRADE 60 STEEL, AT LEAST 16" LONG, 34" MINIMUM DIAMETER, AND
- MAXIMUM 12" SPACING ON CENTER. EDGE OF DOWEL MUST BE AT LEAST 2" FROM CONCRETE SURFACE AND EDGES.

— 6'-5 5/8" — 6'-10 3/4" — ISOMETRIC VIEW

CONCRETE WHEEL STOP

CONCRETE WHEEL STOP NOTES

THE DIMENSION BETWEEN THE LONG EDGE OF THE CONCRETE WHEEL STOP CLOSEST TO THE END OF THE PARKING SPACE AND THE END OF THE PARKING SPACE IS 2.5'.

CROSS SECTION

OF THE PARKING SPACE.

2. THE CONCRETE WHEEL STOP SHALL BE PLACED CENTERED RELATIVE TO THE WIDTH AXIS

- 3. EACH CONCRETE WHEEL STOP MUST BE SECURED WITH TWO #7 REBAR ANCHORAGE PINS WITH A MINIMUM EMBEDMENT DEPTH OF 15".
- THE STEEL REINFORCEMENT IN THE CONCRETE WHEEL STOPS (EXCLUDING THE ANCHORAGE PINS) MUST BE #3 REBAR AND AT LEAST 2" FROM ALL FINISHED SURFACES.

CONCRETE STAIR DETAILS SEE ARCHITECTURAL PLANS FOR HANDRAIL DETAILS 24" (MIN.) TYP. #4 REBAR ~ ENCASE METAL IN AT LEAST 4" OF

CONCRETE STAIR DETAILS NOTES

CONCRETE, TYP.

STAIR TREAD WIDTH IS 12" WITH A 1" RECESS. STAIR HEIGHT IS 6". REFER TO SITE PLAN FOR NUMBER OF STAIRS.

" (MIN.) TYP.

2. ALL CONCRETE CORNERS AND EDGES SHOULD HAVE A FILLET OF 1/2".

12" (MIN.)

- S. CONCRETE MUST HAVE A COMPRESSIVE STRENGTH OF 3500 PSI WITH NO BLACK PIGMENT ADDITIVE AND WITH A LIGHT BROOM FINISH.
- 4. SUBGRADE MUST BE COMPACTED TO 95% PROCTOR DENSITY.
- 5. HANDRAILS SHALL BE PAINTED MATTE BLACK.

11004 & 11006 PARK RD FAIRFAX, VA 22306 TAX MAP #57-1-40-002 SQUARE 02, LOT 002

CLIENT EMRE ZIREKOGLU CAGLAYAN INVESTMENT GROUP 32713 LATROBE ST CHANTILLY, VA 20152 571.594.6363

CONTRACTOR TBD

CIVIL ENGINEER PATRICK HORGAN HUSKA CONSULTING, LLC 1050 30TH STREET, NW WASHINGTON, DC 20007 703.425.3862

LAND SURVEYOR DOMINION SURVEYS, INC. 8808-H PEAR TREE VILLAGE COURT ALEXANDRIA, VA 22309 703.619.6555

> Patrick Horgan PATRICK JOSEPH -686E**MORCAN** Lic. No. 061930

SITE DETAILS

DRAWING TITLE

DRAWING NO.

SITE DETAILS NOTES

1. REFER TO THE CIVIL COVER SHEET FOR ADDITIONAL NOTES.



NOT FOR CONSTRUCTION REZONING PLANS 12/16/2022

005

8" 3500 PSI CLASS E CONCRETE WITH 6"x6" W4xW4 STEEL WWF AT LEAST 2" FROM TOP AND BOTTOM 8" GRADED AGGREGATE BASE, COMPACTED - COMPACTED SUBGRADE ASPHALT PAVEMENT SECTIONS NOTES MILL AND OVERLAY AS REQUIRED TO ATTAIN ADA ACCESSIBLE GRADES AND SLOPES AS CONCRETE PAVEMENT SECTIONS NOTES REQUIRED. 2. MINIMUM ASPHALT MILLING DEPTH IS 1". 3. UTILIZE A VERTICAL SAWCUT AT THE INTERFACE BETWEEN EXISTING ASPHALT PAVEMENT AND MILLE AND OVERLAY SECTION. 4. TACK COAT TO BE INSTALLED AT INTERFACE BETWEEN SURFACE AND BASE COURSE,

CONCRETE HEADER CURB DETAILS GRADED AGGREGATE BASE

CONCRETE HEADER CURB / CURB & GUTTER (PRIVATE PROPERTY)

MATERIALS AND CONSTRUCTION METHODS SHALL BE CONSISTENT WITH THE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL. CONCRETE SHALL BE 3500 PSI CLASS E CONCRETE.

COMPACTED SUBGRADE

REVISIONS

INITIAL SUBMISSION

THIRD SUBMISSION

SECOND SUBMISSION

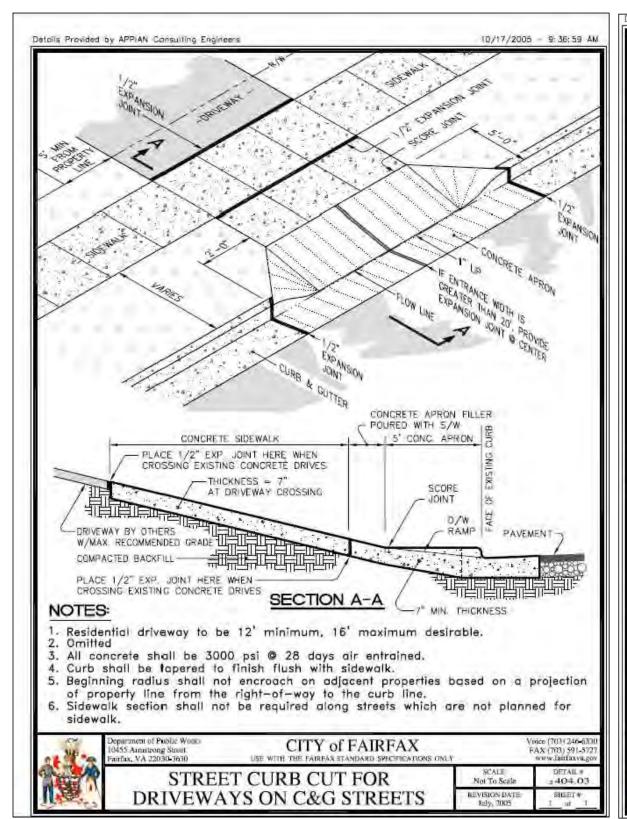
- CONSTRUCT EXPANSION JOINTS AT MAX 15' INTERVALS, AT BEGINNING AND ENDS OF ALL CURVE P.O.T'S, AT STRUCTURES, AND AT MID POINT OF ALL CURB RETURNS. INSTALL 1/2" PRE-FORMED EXPANSION JOINT FILLER,
- NON-EXTRUDING. INSTALL 1/2" EXPANSION JOINT AT BACK OF CURB WHERE ADJACENT TO CONCRETE SIDEWALK.
- INSTALL BITUMINOUS SEALANT AGAINST FACE OF GUTTER ADJACENT TO ASPHALT PAVEMENT.
- WHENEVER NEW CONCRETE CURB (AND GUTTER) MEETS EXISTING CONCRETE CURB (AND GUTTER), ASSURE CURBS ARE ON LINE AND ON GRADE.
- TRANSITION CURBS SHALL BE USED WHENEVER A DIFFERENT TYPE OF CURB IS CALLED OUT. TRANSITIONS MUST BE 10' LONG (MIN.) UNLESS NOTED

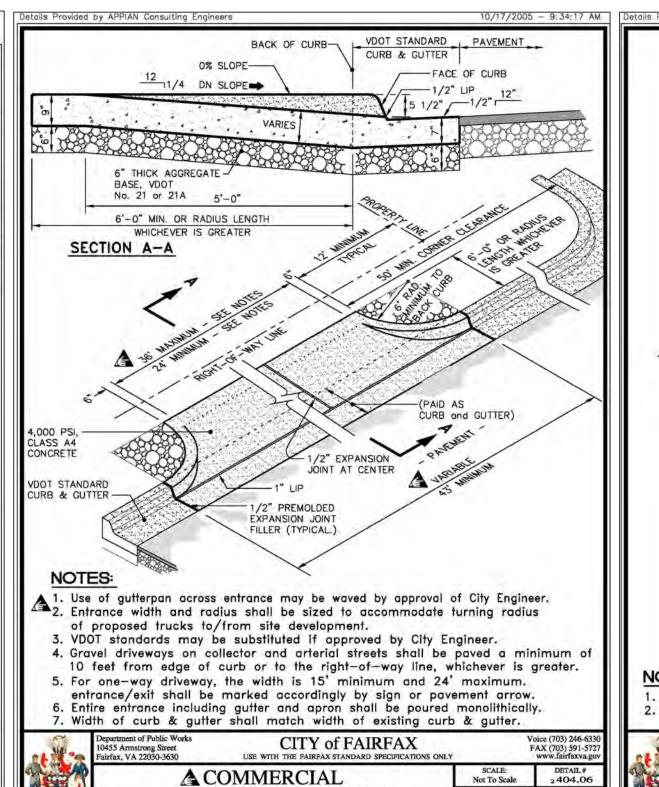
APPROVAL

03/04/2022

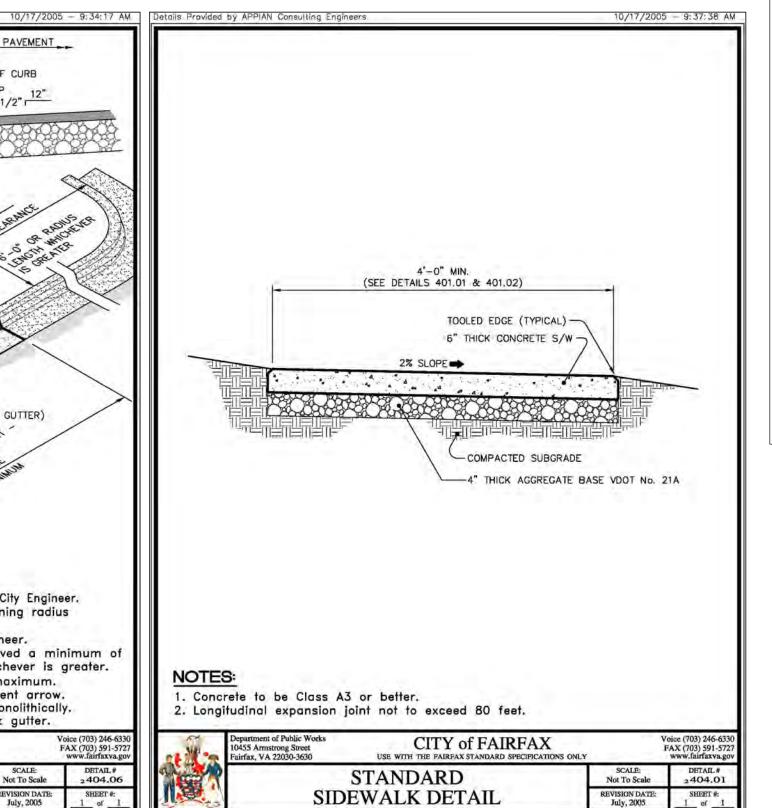
08/25/2022

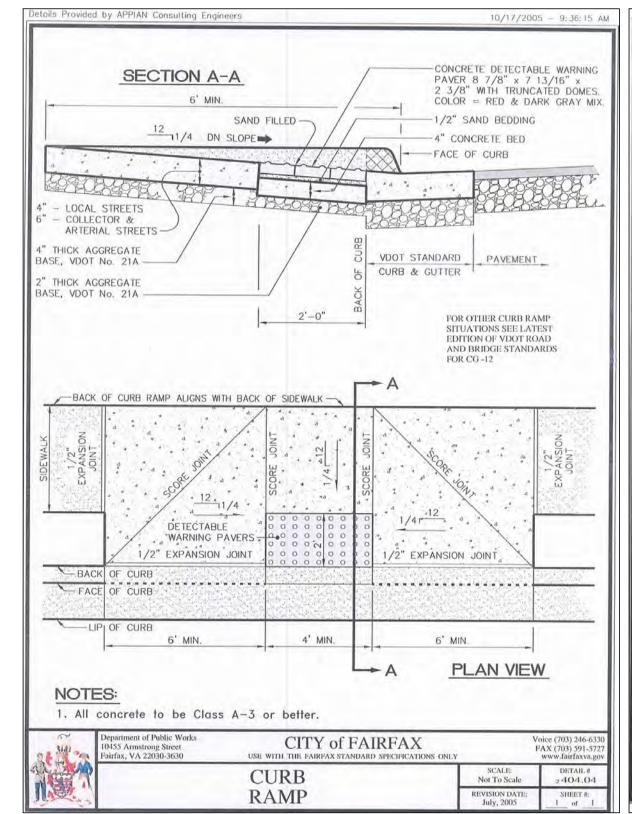
12/16/2022





REVISION DATE: July, 2005





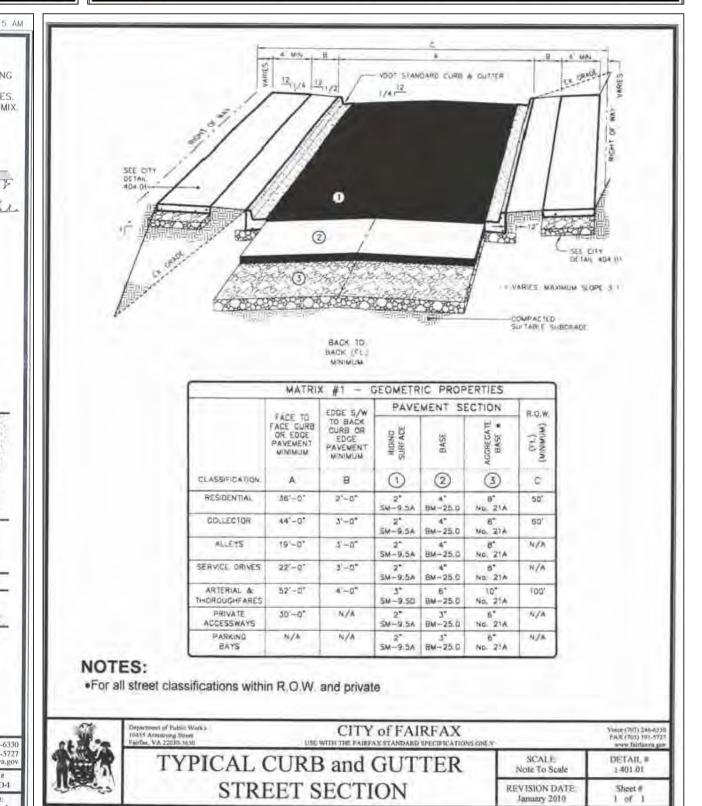
APPROVAL

03/04/2022

08/25/2022

12/16/2022

DRIVEWAY ENTRANCE



11004 & 11006 PARK RD FAIRFAX, VA 22306 TAX MAP #57-1-40-002 SQUARE 02, LOT 002

CLIENT EMRE ZIREKOGLU CAGLAYAN INVESTMENT GROUP 32713 LATROBE ST CHANTILLY, VA 20152

CONTRACTOR TBD

571.594.6363

CIVIL ENGINEER PATRICK HORGAN HUSKA CONSULTING, LLC 1050 30TH STREET, NW WASHINGTON, DC 20007 703.425.3862

LAND SURVEYOR DOMINION SURVEYS, INC. 8808-H PEAR TREE VILLAGE COURT ALEXANDRIA, VA 22309 703.619.6555

686EA15ARGAN Lic. No. 061930

FAIRFAX CITY DPW DETAILS

Patrick Horgan
PATRICK JOSEPH

NOT FOR CONSTRUCTION

DRAWING NO.

SITE DETAILS NOTES

1. REFER TO THE CIVIL COVER SHEET FOR ADDITIONAL NOTES.



HUSKA CONSULTING, LLC

REZONING PLANS 12/16/2022

DRAWING TITLE 006

Sanitary Sewer Capacity Analysis

New Development Flow

Daily Flowrate per Person 100.0 GPD Number of Proposed Bedrooms per Townhome Dwelling Max. Number of Persons per Bedroom Four Bedroom Townhome Dwellings Daily Flowrate, q_{DW} Number of Four Bedroom Townhome Dwellings Submain Sewer Peak Flow Factor, PF Proposed Design Flow, Q_D 41,600 GDP

Submain Sewer Peak Flow Factor, PF 4.00

Existing Development Flow Daily Flowrate per Person Average Number of Bedrooms per Dwelling Max. Number of Persons per Bedroom Dwellings Daily Flowrate, q_{DW} Shopping Centers Flowrate, q_{SC} Service Stations Flowrate, qss Existing Dwellings Exisitng Shopping Centers GFA 161576 SF Service Station Daily Vehicles 576.00 Vehicles

> Proposed Design Flow, Q_D 285,416 GDP 0.442 CFS

> > Total Design Flow, Q 0.506 CFS

Park Rd Townhomes 12/16/2022

Sewer Conveyance - Hydrology and Hydraulic Calculations

Sewer	Hydraulics																					Circular Channel Ratios ³							
	Pipe Inverts Pipe Parameters											Tim	e of Con	centrati	ion	Additio	nal Flow	V Velocity Flowrate			rate	Flow Area Hydr			c Radius				
Ups	tream	Down	stream	Length	Diam.	Mat'l	n	Slope	٧	Q	R	Α	T _c US _{dir} ²	Γ _c Pipe ³ 1	$\Gamma_{\rm c}$ DS _{sys} ⁴	T _c US _{sys} ⁵	${\rm Q_{add'l}}^1$	Q _{add'Lus} ²	V/V_{full}	V_{full}	Q/Q _{full}	Q_{full}	A/A _{full}	A_{full}	R/R _{full}	R_{full}			
ID	Invert	ID	Invert	(ft)	(in)			(ft/ft)	(fps)	(cfs)	(ft)	(sf)	(min)	(sec)	(min)	(min)	(cfs)	(cfs)		(fps)		(cfs)		(sf)		(ft)			
S-G	421.70	DS	413.58	406.1	8	CONC.	0.013	2.00%	4.06	0.44	0.12	0.10	5.0	100	6.7	5.0	0.00	0.00	0.83	4.90	0.26	1.71	0.30	0.35	0.74	0.17	CHANNEL		

Hydrology and Hydraulic Calculations Methodology

Note all sewer conveyance calculations shown here are for the 10 year storm event

n, Manning's roughness coefficient I, rainfall intensity V, veloc Q, flowrate R, hydraulic radius Com., compacted

At the engineer's option, an additional flowrate may be added which will propagate downstream in the system. This flowrate is not affected by time of concentration.

- The sum of the additional flowrates added to the system upstream of the run in question.
- Circular channel ratios are tabulated in the reference tab and have nested if statements that hinge on the flow type for the pipe run in question

Park Rd Townhomes

Sewer Conveyance - Hydraulic Gradeline Calculations

 JC W CT COTT	ive yarrice	Try ar a arre v	ar au chine	Calculation	13																				
From	То	WSE _{down} 1	D	A _{full}	Q	L	R _{full}	n	S _{fr}	H _{fr}	V _{out}	H _o ²	V _{in} ³	H _i ⁴	Angle ⁵	к	H _{bend} 6	Plunging ⁷	IS-1 ⁸	H _{str} ⁹	H _{total}	WSE _{up}	Top El ¹⁰	Top - WSE _{up}	Remarks
		(ft)	(in)	(sf)	(cfs)	(ft)	(ft)		(ft/ft)	(ft)	(fps)	(ft)	(fps)	(ft)	(degrees)		(ft)			(ft)	(ft)	(ft)	(ft)	(ft)	(ft)
DS	S-G	414.11	8	0.35	0.44	406.10	0.17	0.013	0.13%	0.54	4.06	0.08	4.06	0.09	0	0.00	0.00	NO	NO	0.17	0.71	421.93	433.14	11.21	ADEQUATE

g,gravity=32.2 V_i,velocity in H_i,structure inlet loss=0.35*V_i²/2g

 H_{D} , structure outlet loss=0.25(0.3 if top pipe)* $V_0^2/2g$

Hydraulic Gradeline Calculations Methodology

 H_{Δ} , structure bend loss= $K*V_i^2/2g$ H_{str} , structure loss = $H_o + H_i + H_\Delta$ H_{total} , total head loss = $H_{fr} + H_{str}$

 S_{fr} friction slope = 0.453 $Q^2n^2/A^2R^{4/3}$ H_{fr} friction loss = L* S_{fr} V_0 , velocity out

- Water surface elevation in bottom structure of pipe run. For the first (most downstream) run of HGL analysis per VDOT standards use the greater of the tailwater elevation (if known) or 80% full depth.
- Expansion loss for upper structure of pipe run. If the upstream structure is a wye, the expansion losses are taken as zero.
- Velocity of water entering pipe run. If pipe run is at the top of the system, set this to the velocity out of the pipe run. Otherwise, use upstream pipe's velocity. If multiple pipes feed in, use the inlet velocity with the greatest momentum (QxV) Contraction loss for upper structure of pipe run. If the upstream structure is a wye, the expansion losses are taken as zero.
- Angle of deflection in the horizontal plane between the upper structure of the pipe run in question and the next upstream pipe. If multiple pipes in, this is the angle of the pipe which creates the most headloss. If no pipes in, set to zero.
- Bend loss for upper structure of pipe run. By default this formula uses the listed inlet velocity. However, if multiple pipes feed into this run bend losses must be calculated for all inflowing pipes and the maximum chosen.
- If 20%+ of the total flow is coming from a curb/grate inlet, or if there's an inlet pipe with an invert greater than the crown of the outlet pipe, plunging losses apply.
- The engineer may specify IS-1 inlet shaping for a structure which allows the inlet head losses to be reduced by 50%.
- Structure loss (sum of expansion, contraction, and bend loses) for the upstream structure of the pipe run.
- Top elevation of upper structure of pipe run.



APPROVAL DATE **REVISIONS** INITIAL SUBMISSION 03/04/2022 SECOND SUBMISSION 08/25/2022 12/16/2022 THIRD SUBMISSION



NOT FOR CONSTRUCTION REZONING PLANS 12/16/2022

Patrick Horgan PATRICK JOSEPH —686EMORGAN Lic. No. 061930

11004 & 11006 PARK RD

FAIRFAX, VA 22306

TAX MAP #57-1-40-002

SQUARE 02, LOT 002

EMRE ZIREKOGLU

32713 LATROBE ST CHANTILLY, VA 20152

571.594.6363

CONTRACTOR

CIVIL ENGINEER

703.425.3862

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PATRICK HORGAN

LAND SURVEYOR

HUSKA CONSULTING, LLC 1050 30TH STREET, NW

WASHINGTON, DC 20007

DOMINION SURVEYS, INC.

ALEXANDRIA, VA 22309

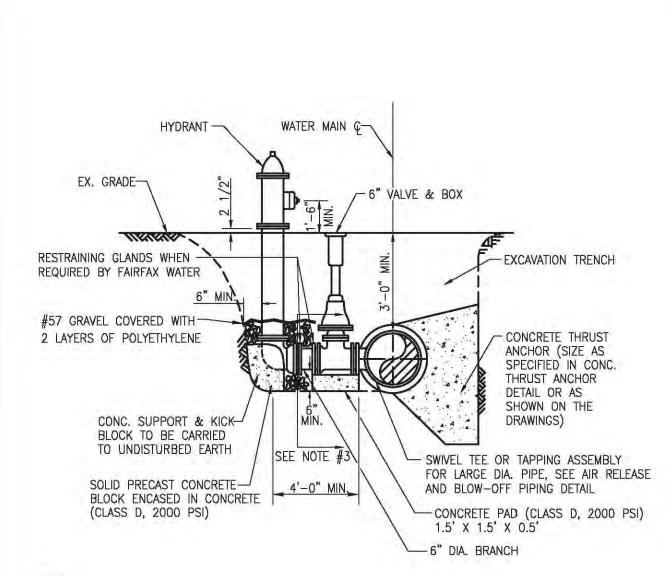
8808-H PEAR TREE VILLAGE COURT

CAGLAYAN INVESTMENT GROUP

CLIENT

SANITARY SEWER CAPACITY ANALYSIS DRAWING TITLE

800



1. IF SWIVEL TEE IS NOT USED, VALVE MUST BE RESTRAINED TO TEE WITH RESTRAINING GLANDS BY

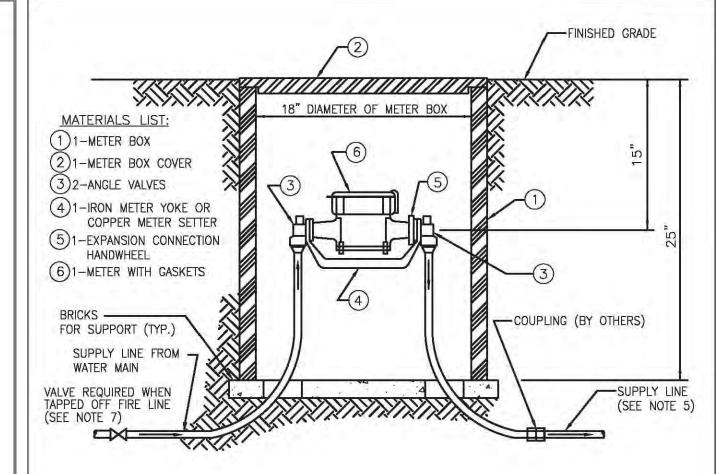
CONTRACTOR. 2. HYDRANTS SHALL HAVE SHOP-APPLIED COATINGS AS FOLLOWS:

 HYDRANT BARREL: KENNEDY SAFETY RED OR MUELLER RED #10 • TOPS AND CAPS: KENNEDY SILVER OR MUELLER SILVER #18 • WHERE INDICATED BY FAIRFAX WATER, THE TOP SHALL BE SHOP-COATED RED AND THE BARREL

AND CAPS SHALL BE SHOP-COATED SILVER IN LIEU OF THE ABOVE. 3. POLYETHYLENE ENCASEMENT TO BE INSTALLED UP TO 6-INCHES BELOW PROPOSED GRADE. ENCASEMENT

SHALL BE INSTALLED SO AS NOT TO PREVENT DISCHARGE OF WATER THROUGH HYDRANT DRAIN HOLES. 4. FOR HYDRANT LOCATION IN REGARD TO FACE OF CURB, SEE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL.

W	FAIRFAX WATER STANDARD DETAILS	SCALE: NOT TO SCALE
~ <u>~</u>	STANDARD HYDRANT INSTALLATION	DRAWING NO.:
DATE: 7/17		30



1. THE WATER METER BOX SHALL BE INSTALLED IN AN ACCESSIBLE LOCATION IN A GREEN SPACE AND SO AS NOT TO BE A TRIP HAZARD.

2. WATER METER TO BE INSTALLED BY FAIRFAX WATER AS SHOWN IN THE DIAGRAM ABOVE. METER TO BE INSTALLED BY CUSTOMER WHEN TAPPED OFF FIRE LINE.

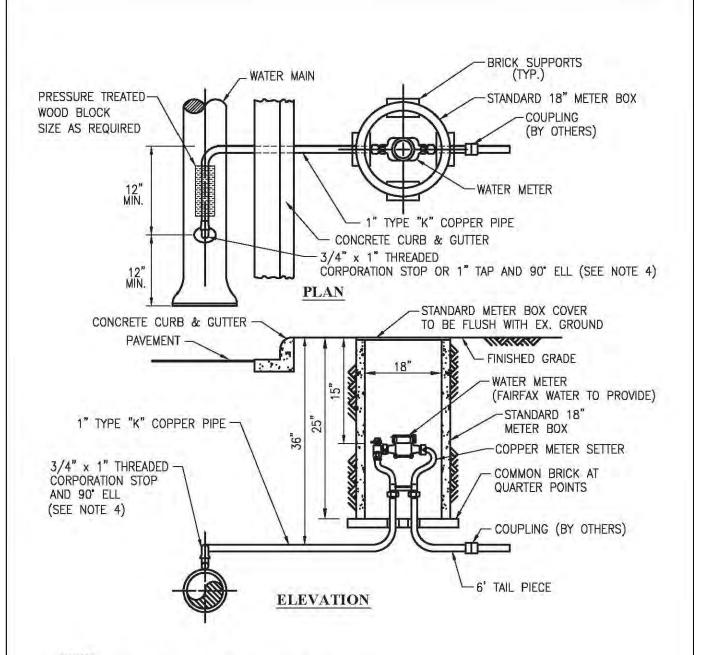
3. THE METER INSTALLATION WILL BE INSPECTED AND APPROVED BY FAIRFAX WATER. CALL 703-289-6402 FOR INSPECTION PRIOR TO PLACING LINE IN SERVICE.

4. FAIRFAX WATER TO SUPPLY ITEMS SHOWN IN MATERIALS LIST. ALL OTHER MATERIALS TO BE SUPPLIED BY THE CUSTOMER. 5. BACKFLOW PREVENTION WILL BE REQUIRED IN ACCORDANCE WITH FAIRFAX COUNTY REGULATIONS.

6. FAIRFAX WATER MAINTAINS THE SUPPLY LINE BETWEEN THE METER AND THE MAIN, METER, METER BOX, AND METER BOX COVER ONLY. FAIRFAX WATER WILL NOT MAINTAIN SUPPLY LINE WHEN TAPPED OFF FIRE LINE. 7. A 3" MINIMUM GATE VALVE WITH A 2" OPERATING NUT MUST BE INSTALLED ON THE SERVICE LINE WHEN TAPPED

OFF THE FIRE LINE. 8. NO STRUCTURES, POLES, SIGN POSTS, TREES OR SHRUBS TO BE INSTALLED WITHIN FOUR FEET OF METER CROCK.

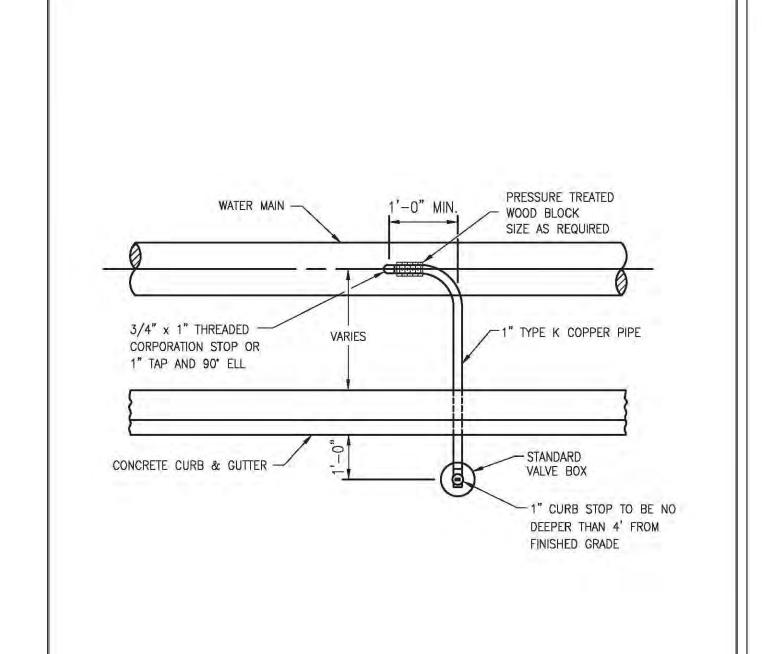
W	FAIRFAX WATER STANDARD DETAILS	SCALE: NOT TO SCALE
-U-	EXTERIOR METER INSTALLATION 5/8" THROUGH 1" METER SIZES	DRAWING NO.:
DATE: 7/17	5/6 THROUGH I METER SIZES	О



1. NO PLASTIC TUBING TO BE USED INSIDE METER BOX. 2. METER BOX, METER BOX COVER AND COPPER METER SETTER TO BE FAIRFAX WATER STANDARD TYPE. 3. THREADED SERVICE CLAMP TO BE USED ON 3" AND 20" & LARGER WATER MAINS. 4. COAT WITH PETROLATUM TAPE PER SECTION 13110 CATHODIC PROTECTION.

5. NO STRUCTURES, POLES, SIGN POSTS, TREES OR SHRUBS TO BE INSTALLED WITHIN FOUR FEET OF METER CROCK.

W	FAIRFAX WATER STANDARD DETAILS	SCALE: NOT TO SCALE
DATE: 7/17	1" SERVICE CONNECTION WITH 1" METER	DRAWING NO.:



WHERE CURB BOX IS LOCATED IN PAVEMENT, USE VALVE BOX INSTEAD OF CURB BOX.

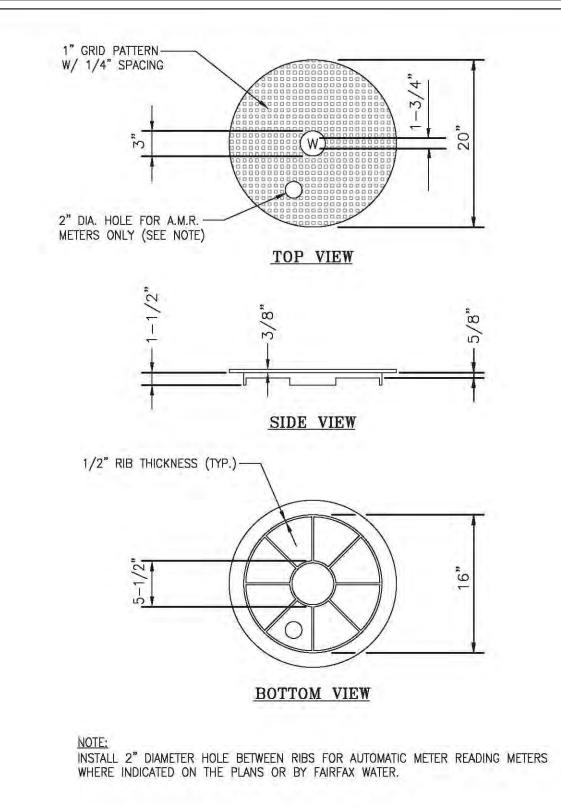
W	FAIRFAX WATER STANDARD DETAILS	SCALE: NOT TO SCALE
DATE: 7/17	1" SERVICE CONNECTION WITH CURB STOP	DRAWING NO.:

APPROVAL

03/04/2022

08/25/2022

12/16/2022



FAIRFAX WATER STANDARD DETAILS

CAST IRON METER BOX COVER

UTILITY DETAILS NOTES

DATE: 7/17

REVISIONS

INITIAL SUBMISSION

THIRD SUBMISSION

SECOND SUBMISSION

1. REFER TO THE CIVIL COVER SHEET FOR ADDITIONAL NOTES.



SCALE:

NOT TO SCALE

DRAWING NO .:

NOT FOR CONSTRUCTION **REZONING PLANS**

FAIRFAX WATER **DETAILS**

PATRICK TOSEPH —686E**MORGAN**.. Lic. No. 061930

11004 & 11006 PARK RD

FAIRFAX, VA 22306

TAX MAP #57-1-40-002

SQUARE 02, LOT 002

EMRE ZIREKOGLU

32713 LATROBE ST

571.594.6363

TBD

CONTRACTOR

CIVIL ENGINEER

703.425.3862

703.619.6555

PATRICK HORGAN

LAND SURVEYOR

HUSKA CONSULTING, LLC

1050 30TH STREET, NW

WASHINGTON, DC 20007

DOMINION SURVEYS, INC.

ALEXANDRIA, VA 22309

8808-H PEAR TREE VILLAGE COURT

CHANTILLY, VA 20152

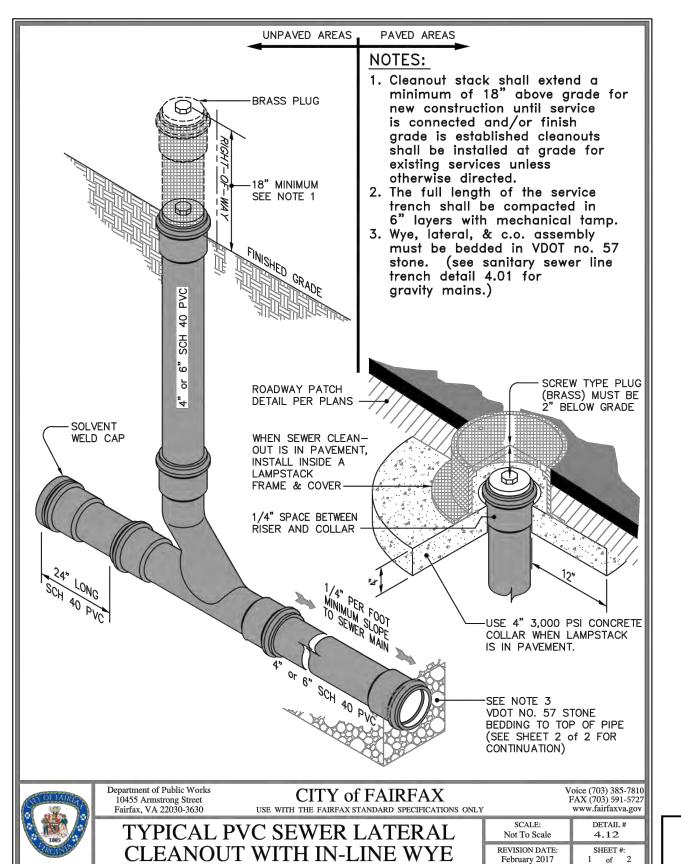
CAGLAYAN INVESTMENT GROUP

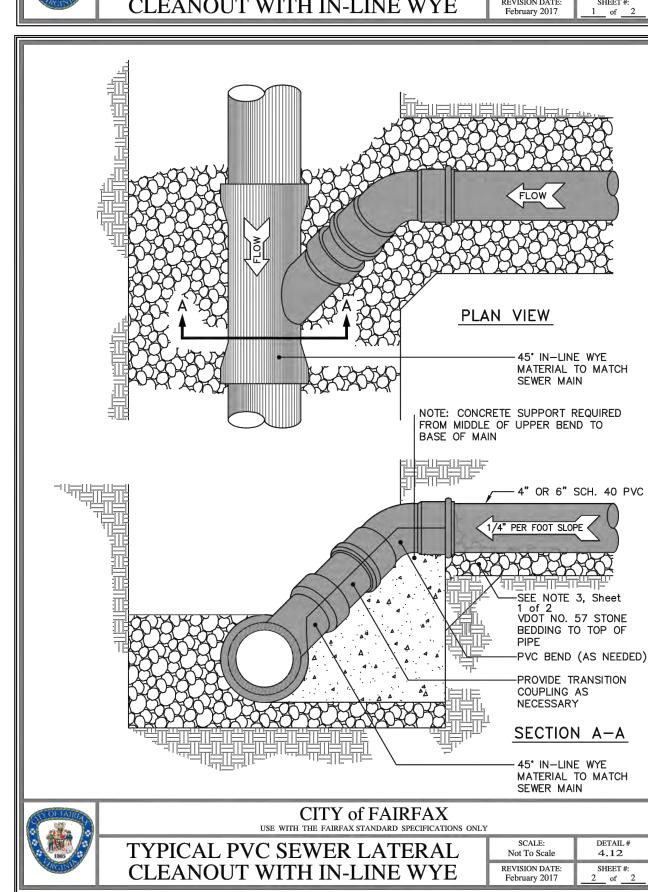
CLIENT

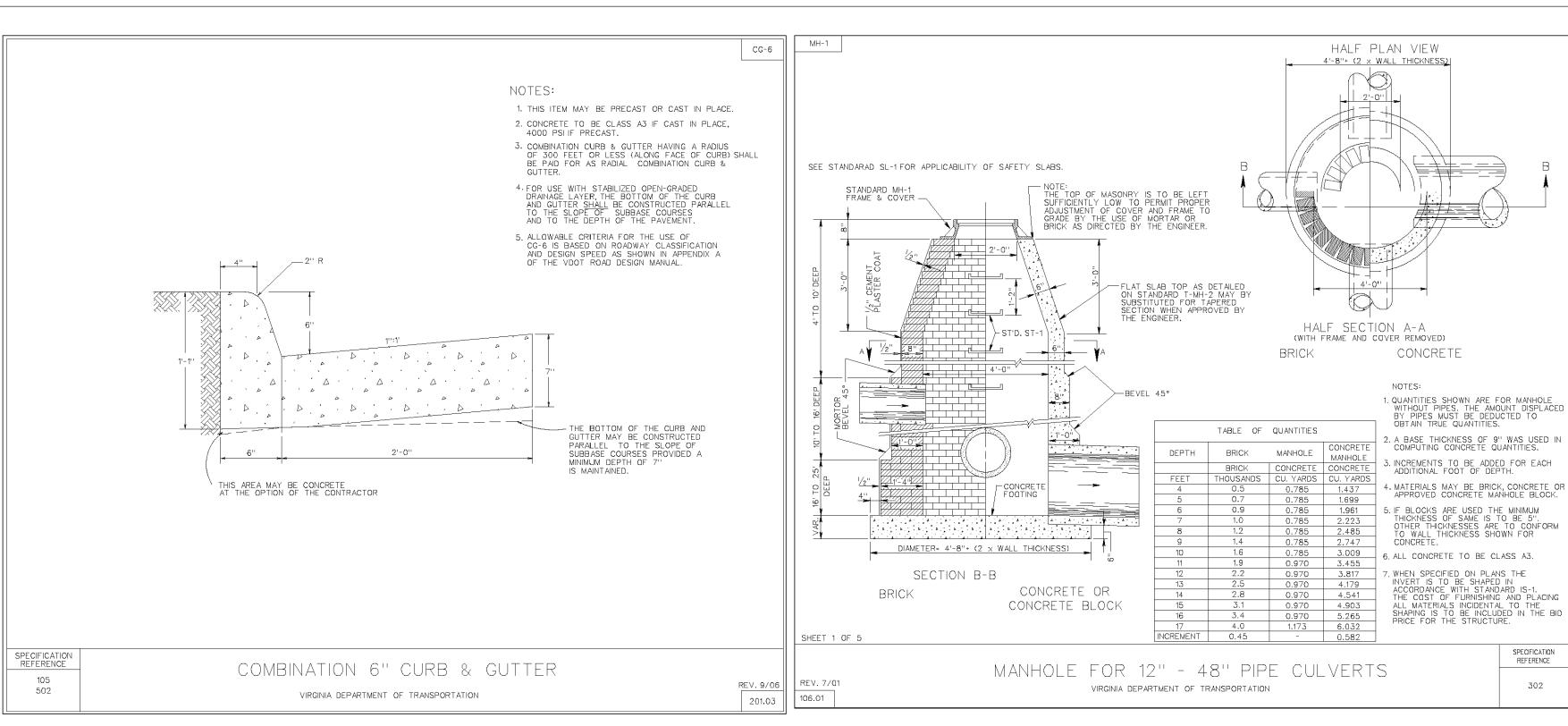
DRAWING NO.

DRAWING TITLE

12/16/2022 HUSKA CONSULTING, LLC





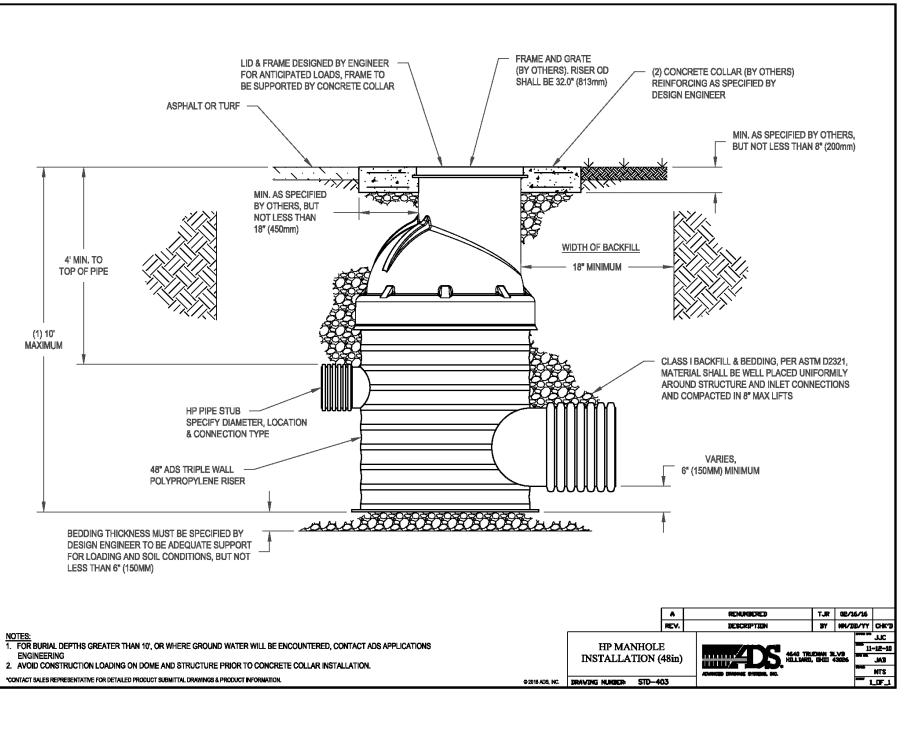


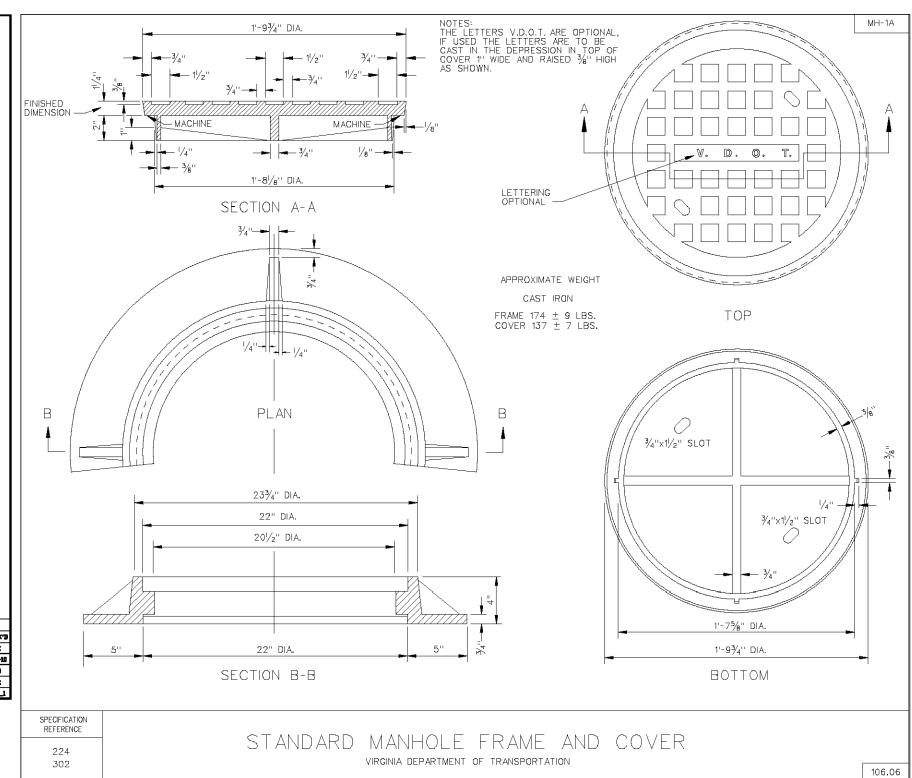
APPROVAL

03/04/2022

08/25/2022

12/16/2022





11004 & 11006 PARK RD FAIRFAX, VA 22306 TAX MAP #57-1-40-002 SQUARE 02, LOT 002

CLIENT EMRE ZIREKOGLU CAGLAYAN INVESTMENT GROUP 32713 LATROBE ST CHANTILLY, VA 20152 571.594.6363

CONTRACTOR TBD

CIVIL ENGINEER PATRICK HORGAN HUSKA CONSULTING, LLC 1050 30TH STREET, NW WASHINGTON, DC 20007 703.425.3862

LAND SURVEYOR DOMINION SURVEYS, INC. 8808-H PEAR TREE VILLAGE COURT ALEXANDRIA, VA 22309 703.619.6555

> Patrick Horgan PATRICK JOSEPH 686E44FO9RGAN Lic. No. 061930

UTILITY DETAILS

NOT FOR CONSTRUCTION DRAWING TITLE

DRAWING NO.

UTILITY DETAILS NOTES

REVISIONS

INITIAL SUBMISSION

THIRD SUBMISSION

SECOND SUBMISSION

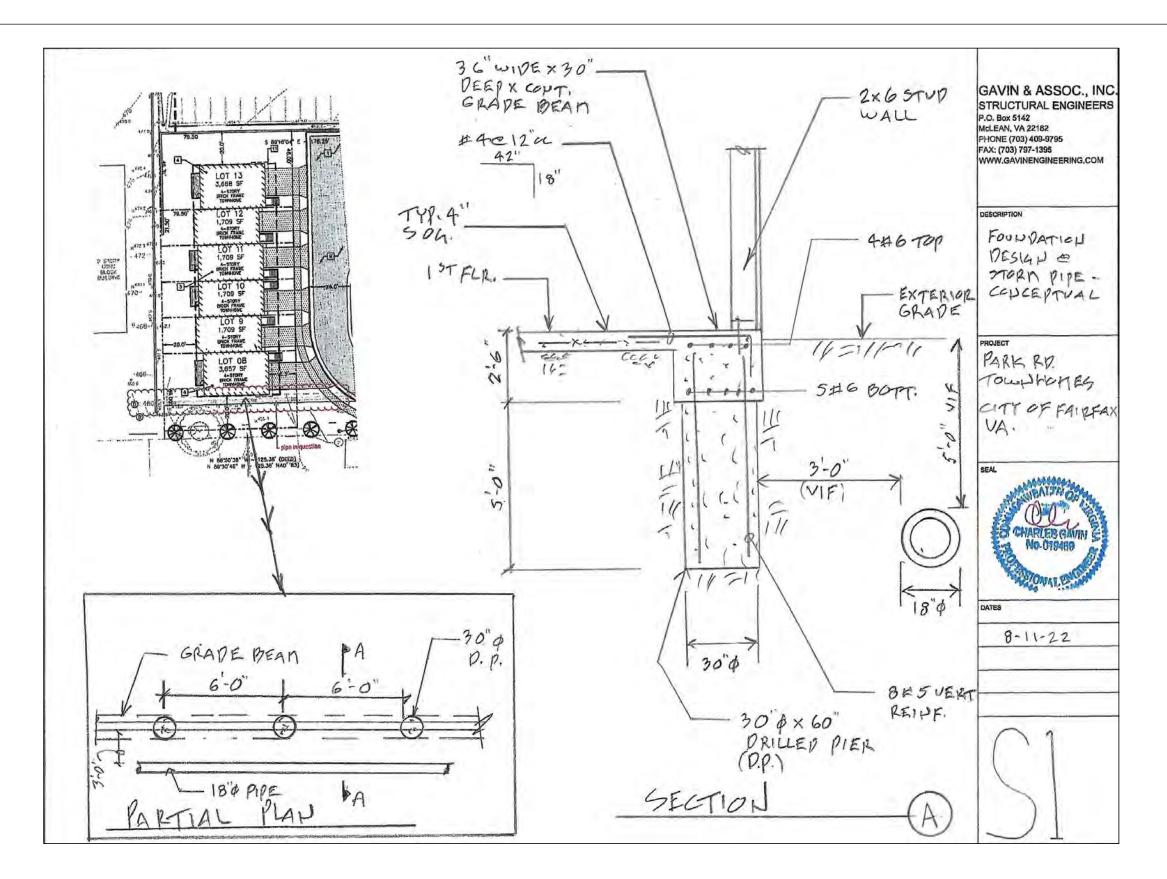
1. REFER TO THE CIVIL COVER SHEET FOR ADDITIONAL NOTES.

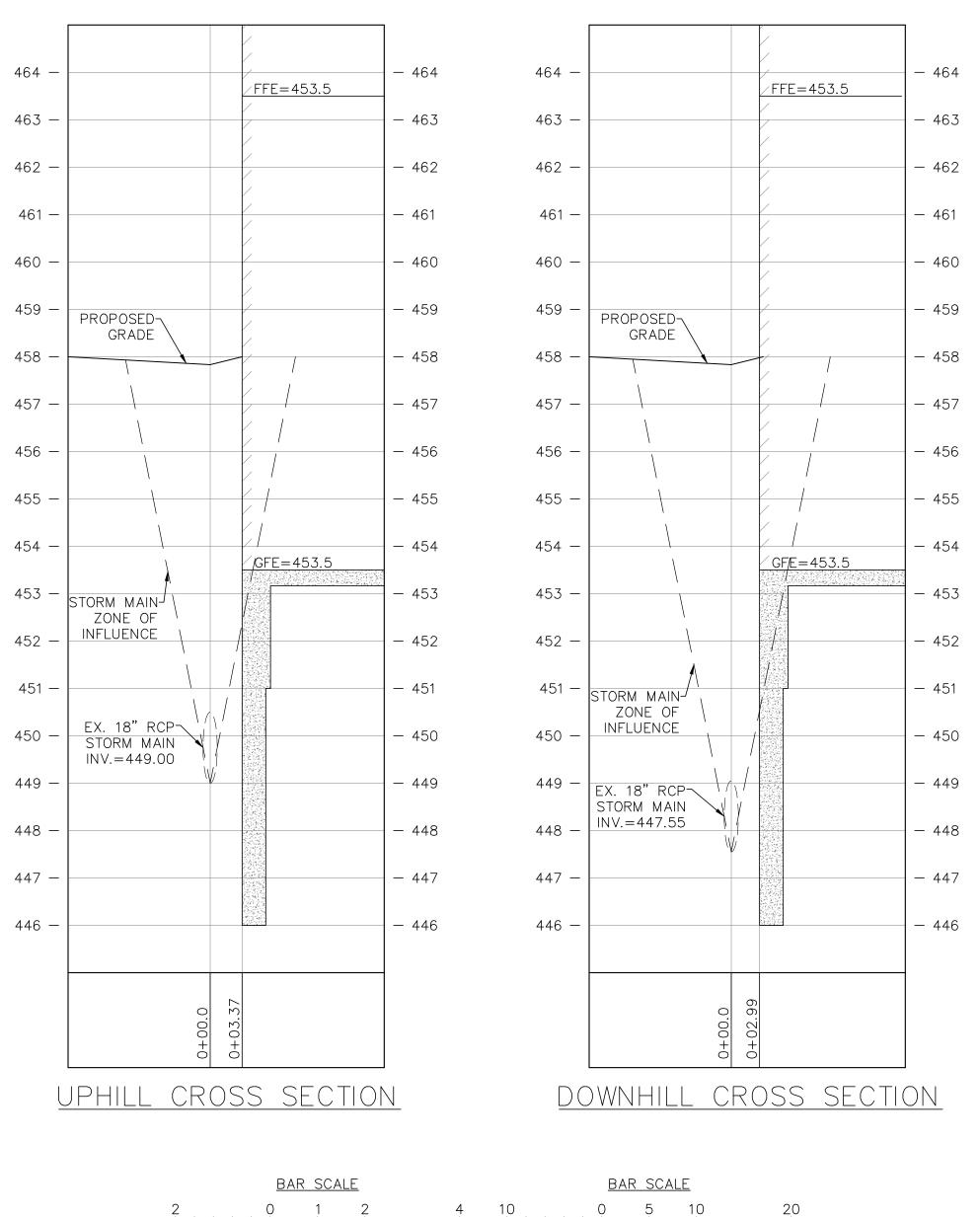


SPECIFICATION REFERENCE

302

REZONING PLANS 12/16/2022





11004 & 11006 PARK RD FAIRFAX, VA 22306 TAX MAP #57-1-40-002 SQUARE 02, LOT 002

CLIENT **EMRE ZIREKOGLU** CAGLAYAN INVESTMENT GROUP 32713 LATROBE ST CHANTILLY, VA 20152 571.594.6363

CONTRACTOR

CIVIL ENGINEER PATRICK HORGAN HUSKA CONSULTING, LLC 1050 30TH STREET, NW WASHINGTON, DC 20007 703.425.3862

LAND SURVEYOR DOMINION SURVEYS, INC. 8808-H PEAR TREE VILLAGE COURT ALEXANDRIA, VA 22309 703.619.6555

HORIZONTAL VERTICAL

UTILITY PROFILES NOTES

- 1. THE LOCATIONS AND ELEVATIONS OF EXISTING UTILITIES SHOWN IN THE PROFILES ARE SHOWN APPROXIMATELY, ARE BASED ON AVAILABLE INFORMATION PROVIDED BY THE SURVEYOR, CONTRACTOR, MISS UTILITY, AND ASSUMPTIONS FROM THE ENGINEER. CONTRACTOR TO VERIFY.
- 2. THE CONTRACTOR MUST DETERMINE THE LOCATIONS AND ELEVATIONS OF THE VARIOUS EXISTING UTILITIES, BY HAND EXCAVATION IF NECESSARY, PRIOR TO COMMENCING CONSTRUCTION. DISCREPANCIES FOUND BETWEEN FIELD CONDITIONS AND PROFILES SHALL BE REPORTED TO THE ENGINEER; PROPOSED UTILITY LOCATIONS AND ELEVATIONS MAY NEED TO BE ADJUSTED DEPENDING ON THE DISCREPANCIES, AND CONSULTATION FROM THE ENGINEER IS RECOMMENDED.
- 3. THE EXISTING AND PROPOSED GRADES ARE SHOWN APPROXIMATELY ON THE UTILITY PROFILES.
- 4. UNLESS OTHERWISE APPROVED BY THE FAIRFAX WATER INSPECTOR, MAINTAIN A MINIMUM 12" OF SEPARATION BETWEEN FAIRFAX WATER UTILITIES AND OTHER UTILITIES IN PUBLIC SPACE.
- 5. REFER TO THE CIVIL COVER SHEET FOR ADDITIONAL INFORMATION.

APPROVAL	DATE	REVISIONS
	03/04/2022	INITIAL SUBMISSION
	08/25/2022	SECOND SUBMISSION
	12/16/2022	THIRD SUBMISSION



NOT FOR CONSTRUCTION REZONING PLANS 12/16/2022

PATRICK HOSEPH Lic. No. 061930 12/17/2022

STORM MAIN

CROSS SECTIONS DRAWING TITLE

STORMWATER MANAGEMENT PLAN LEGEND

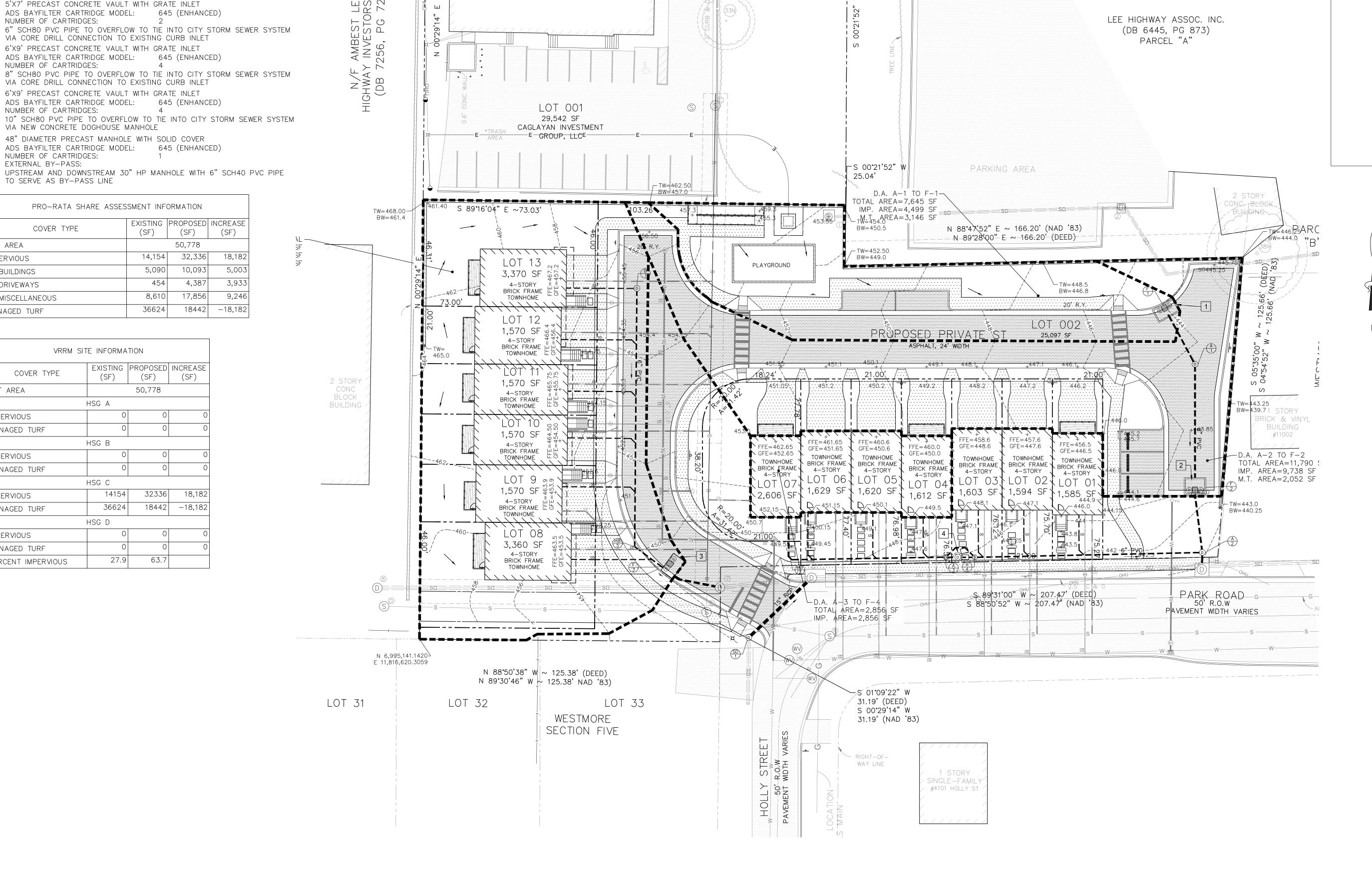
DRAINAGE DIVIDE

STORMWATER MANAGEMENT PLAN KEYNOTES

- 1 5'X7' PRECAST CONCRETE VAULT WITH GRATE INLET ADS BAYFILTER CARTRIDGE MODEL: 645 (ENHANCED) NUMBER OF CARTRIDGES:
- 6" SCH80 PVC PIPE TO OVERFLOW TO TIE INTO CITY STORM SEWER SYSTEM VIA CORE DRILL CONNECTION TO EXISTING CURB INLET
- 2 6'X9' PRECAST CONCRETE VAULT WITH GRATE INLET ADS BAYFILTER CARTRIDGE MODEL: 645 (ENHANCED) NUMBER OF CARTRIDGES:
- VIA CORE DRILL CONNECTION TO EXISTING CURB INLET 3 6'X9' PRECAST CONCRETE VAULT WITH GRATE INLET ADS BAYFILTER CARTRIDGE MODEL: 645 (ENHANCED)
- NUMBER OF CARTRIDGES: 10" SCH80 PVC PIPE TO OVERFLOW TO TIE INTO CITY STORM SEWER SYSTEM VIA NEW CONCRETE DOGHOUSE MANHOLE
- 48" DIAMETER PRECAST MANHOLE WITH SOLID COVER ADS BAYFILTER CARTRIDGE MODEL: 645 (ENHANCED) NUMBER OF CARTRIDGES: EXTERNAL BY-PASS: UPSTREAM AND DOWNSTREAM 30" HP MANHOLE WITH 6" SCH40 PVC PIPE TO SERVE AS BY-PASS LINE

PRO-RATA SHARE ASSESSMENT INFORMATION								
COVER TYPE	EXISTING (SF)	PROPOSED (SF)	INCREASE (SF)					
LOT AREA		50,778						
IMPERVIOUS	14,154	32,336	18,182					
BUILDINGS	5,090	10,093	5,003					
DRIVEWAYS	454	4,387	3,933					
MISCELLANEOUS	8,610	17,856	9,246					
MANAGED TURF	36624	18442	-18,182					

VRRM S	SITE INFORMA	TION							
COVER TYPE	EXISTING (SF)	PROPOSED (SF)	INCREASE (SF)						
LOT AREA		50,778							
HSG A									
IMPERVIOUS	0	0	0						
MANAGED TURF	0	0	0						
	HSG B								
IMPERVIOUS	0	0	0						
MANAGED TURF	0	0	0						
	HSG C								
IMPERVIOUS	14154	32336	18,182						
MANAGED TURF	36624	18442	-18,182						
	HSG D	•							
IMPERVIOUS	0	0	0						
MANAGED TURF	0	0	0						
PERCENT IMPERVIOUS	27.9	63.7							



11004 & 11006 PARK RD FAIRFAX, VA 22306 TAX MAP #57-1-40-002 SQUARE 02, LOT 002

CLIENT EMRE ZIREKOGLU CAGLAYAN INVESTMENT GROUP 32713 LATROBE ST CHANTILLY, VA 20152 571.594.6363

CONTRACTOR

CIVIL ENGINEER PATRICK HORGAN HUSKA CONSULTING, LLC 1050 30TH STREET, NW WASHINGTON, DC 20007 703.425.3862

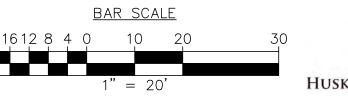
LAND SURVEYOR DOMINION SURVEYS, INC. 8808-H PEAR TREE VILLAGE COURT ALEXANDRIA, VA 22309 703.619.6555

Patrick Horgan PATRICK JOSEPH 686E**ANONROGAN**I... Lic. No. 061930

STORMWATER MANAGEMENT PLAN NOTES

1. REFER TO THE CIVIL COVER SHEET FOR ADDITIONAL INFORMATION.

APPROVAL REVISIONS 03/04/2022 INITIAL SUBMISSION SECOND SUBMISSION 08/25/2022 12/16/2022 THIRD SUBMISSION





NOT FOR CONSTRUCTION DRAWING TITLE **REZONING PLANS** 12/16/2022

STORMWATER **MANAGEMENT** PLAN

012

DocuSign Envelope ID: ECC182DD-B804-4FA1-A578-120CA02447B8 Project Name: **Park Rd Townhomes** 12/16/2022 Linear Development Project? Site Information Pre-ReDevelopment Land Cover (acres) Forest/Open Space (acres) -- undisturbed forest/open space Managed Turf (acres) -- disturbed, graded for yards or other turf to be Impervious Cover (acres) Post-Development Land Cover (acres) Forest/Open Space (acres) -- undisturbed, protected forest/open space or reforested Managed Turf (acres) -- disturbed, graded for yards or other turf to be Impervious Cover (acres) Area Check OK. Drainage Area A Drainage Area A Land Cover (acres) A Soils Forest/Open Space (acres) Managed Turf (acres) Impervious Cover (acres) Stormwate 14. Manufactur 14.a. Manufact 14.b. Manufactured Drainage Area B Drainage Area A Land Cover (acres) Forest/Open Space (acres) Managed Turf (acres) Impervious Cover (acres) Stormwater Rest Management Practices (RR = Runoff Reduction)

CLEAR ALL

data input cells constant values calculation cells final results

Post-Development Project (Treatment Volume and Loads)

Enter Total Disturbed Area (acres) → 1.16 Maximum reduction required: 20% The site's net increase in impervious cover (acres) is: 0.417401286 Post-Development TP Load Reduction for Site (lb/yr): 0.92

Check: Linear project? No

Land cover areas entered correctly?

B Soils C Soils Totals 0.00 0.84 0.84 0.32 1.17

A Soils B Soils C Soils D Soils Totals 0.00 0.42 0.74

BMP Design Specifications List: 2013 Draft Stds & Specs

Runoff Reduction Volume and TP By Drainage Area Total disturbed area entered? RUNOFF REDUCTION VOLUME ACHIEVED (ft3

> NITROGEN LOAD REDUCTION ACHIEVED (lb/yr) 0.00 0.00 0.00 0.00 0.00 0.00 **Total Phosphorus** FINAL POST-DEVELOPMENT TP LOAD (lb/yr) 0.92 TP LOAD REDUCTION REQUIRED (lb/yr) TP LOAD REDUCTION ACHIEVED (lb/yr) 0.95 TP LOAD REMAINING (lb/yr): 0.87 REMAINING TP LOAD REDUCTION REQUIRED (lb/yr): 0.00 ** TARGET TP REDUCTION EXCEEDED BY 0.04 LB/YEAR **

TP LOAD AVAILABLE FOR REMOVAL (lb/yr

TP LOAD REDUCTION ACHIEVED (lb/yr)

TP LOAD REMAINING (lb/yr

Area Checks

AREA CHECK

Site Treatment Volume (ft³) 2,898

FOREST/OPEN SPACE (a

MANAGED TURF AREA (ad

IMPERVIOUS COVER TREATED (ac

MANAGED TURF AREA TREATED (ad

IMPERVIOUS COVER (ac

Total Nitrogen (For Information Purposes) POST-DEVELOPMENT LOAD (lb/yr) 13.03

NITROGEN LOAD REDUCTION ACHIEVED (lb/yr) 0.00 REMAINING POST-DEVELOPMENT NITROGEN LOAD (lb/yr) 13.03

Totals **B** Soils C Soils **Land Cover Rv** 0.00 0.00 0.12 0.22 0.12 0.39 0.95 0.39 Total 0.51

0.74

OK.

1.17

Total Phosphorus Available for Removal in D.A. A (lb/yr) Post Development Treatment Volume in D.A. A (ft³) 1,448

CLEAR BMP AREAS

ter Best Managem	ent Practio	ces (RR = R	unoff Redu	ction)									Select from dropdown list
Practice	Runoff Reduction Credit (%)	Managed Turf Credit Area (acres)	Impervious Cover Credit Area (acres)	Volume from Upstream Practice (ft ³)	Runoff Reduction (ft³)	Remaining Runoff Volume (ft ³)	Total BMP Treatment Volume (ft ³)	Phosphorus Removal Efficiency (%)	Phosphorus Load from Upstream Practices (lb)	Untreated Phosphorus Load to Practice (lb)	Phosphorus Removed By Practice (lb)	Remaining Phosphorus Load (Ib)	Downstream Practice to b Employed
tured Treatment Devices (r	no RR)												
factured Treatment Device- Hydrodynamic	0			0	0	0	0	20	0.00	0.00	0.00	0.00	
ured Treatment Device-Filtering	0	0.12	0.39	0	0	1,448	1,448	64	0.00	0.91	0.58	0.33	

Nitrogen Removal Efficiency (%)	Nitrogen Load from Upstream Practices (lbs)	Untreated Nitrogen Load to Practice (lbs)	Nitrogen Removed By Practice (Ibs)	Remaining Nitrogen Load (lbs)
	14. Manufactur	ed BMP (no RR)		
0	0.00	0.00	0.00	0.00
0	0.00	6.50	0.00	6.50

Runoff Volume and CN Calculations

1-year storm | 2-year storm | 10-year storm Target Rainfall Event (in) 3.11 4.78 RV & CN Drainage Area A Drainage Area B Drainage Area C Drainage Area D Drainage Area E **Drainage Areas** RR (ft3) 0.00 0.00 RV wo RR (ws-in) 1.77 1.45 0.00 1-year return period 1.77 1.45 0.00 0.00 0.00 RV w RR (ws-in) CN adjusted 88 0 0 92 0 2.27 0.00 0.00 RV wo RR (ws-in) 1.92 0.00 0.00 2-year return period RV w RR (ws-in) 2.27 1.92 0.00 0.00 CN adjusted 92 88 RV wo RR (ws-in) 3.87 3.46 0.00 0.00 0.00 3.87 0.00 0.00 0.00 3.46 10-year return period RV w RR (ws-in) CN adjusted 92

11004 & 11006 PARK RD FAIRFAX, VA 22306 TAX MAP #57-1-40-002 SQUARE 02, LOT 002

CLIENT EMRE ZIREKOGLU CAGLAYAN INVESTMENT GROUP 32713 LATROBE ST CHANTILLY, VA 20152

CONTRACTOR TBD

571.594.6363

CIVIL ENGINEER PATRICK HORGAN HUSKA CONSULTING, LLC 1050 30TH STREET, NW WASHINGTON, DC 20007 703.425.3862

LAND SURVEYOR DOMINION SURVEYS, INC. 8808-H PEAR TREE VILLAGE COURT ALEXANDRIA, VA 22309 703.619.6555

Totals Land Cover Rv 0.00 0.00 0.17 0.17 0.22 0.23 0.23 0.95

Treatment Volume 2500.0 CF

Required Number of Cartridges

Convert to CFS 0.10 CFS

CLEAR BMP AREAS

otal Phosphorus Available for Removal in D.A. B (lb/yr) 0.58

Post Development Treatment Volume in D.A. B (ft³) 923

Total	0.40
Design to the second of the se	

Cartridge Model 645

Treatment Flowrate 45.00 GPM

Treatment Volume 2500.0 CF

Stormwater Best Managem	ent Pract	ices (KK =	Kunott Ke	auction)								-	Select from dropdown lis
Practice	Runoff Reduction Credit (%)	and the second of the second o	Cover Credit	Volume from Upstream Practice (ft ³)	Reduction	Remaining Runoff Volume (ft ³)	Total BMP Treatment Volume (ft ³)	Removal Efficiency	Phosphorus Load from Upstream Practices (lb)	Phosphorus Load to	Removed By	Phosphorus	Downstream Practice to
14. Manufactured Treatment Devices (no RR)												
14.a. Manufactured Treatment Device- Hydrodynamic	0			0	0	0	0	20	0.00	0.00	0.00	0.00	
14.b. Manufactured Treatment Device-Filtering	0	0.17	0.23	0	0	923	923	64	0.00	0.58	0.37	0.21	

Bayfilter (F-1)		Bayfilter (F-2)		Bayfilter (F-3)		Bayfilter (F-4)	
Runoff Reduction	0%						
Totla Phosphorous Removal	50%						
Define Drainage Area		Define Drainage Area		Define Drainage Area		<u>Define Drainage Area</u>	
Total Drainage Area (A) =	7,645 SF	Total Drainage Area (A) =	11,790 SF	Total Drainage Area (A) =	17,399 SF	Total Drainage Area (A) =	2,856 SF
Total Impervious Area (A _I) =	4,499 SF	Total Impervious Area (A _I) =	9,738 SF	Total Impervious Area (A _I) =	9,925 SF	Total Impervious Area (A _I) =	2,856 SF
Managed Turf Area (A_C) =	3,146 SF	Managed Turf Area (A_C) =	2,052 SF	Managed Turf Area (A_C) =	7,474 SF	Managed Turf Area (A_C) =	0 SF
Forested Area $(A_N) =$	0 SF	Forested Area (A_N) =	0 SF	Forested Area $(A_N) =$	0 SF	Forested Area $(A_N) =$	0 SF
		HSG	С	HSG	С	HSG	С
		Drainage Area R _v	0.79	Drainage Area R _v	0.63	Drainage Area R _v	0.90
HSG	С						
Drainage Area R _v	0.64	<u>Calculate Peak Discharge</u>		<u>Calculate Peak Discharge</u>		<u>Calculate Peak Discharge</u>	
		Total Treatment Volume (Tv)	777 CF	Total Treatment Volume (Tv)	913 CF	Total Treatment Volume (Tv)	214 CF
Calculate Peak Discharge		Runoff Volume, Qa	0.79 IN	Runoff Volume, Qa	0.63 IN	Runoff Volume, Qa	0.90 IN
Total Treatment Volume (Tv)	408 CF	CN	93.82	CN	87.69	CN	98.00
Runoff Volume, Qa	0.64 IN	Time of Concentration, Tc	6.00 MIN	Time of Concentration, To	6.00 MIN	Time of Concentration, Tc	6.00 MIN
CN	88.12	Initial abstraction, Ia	0.151	Initial abstraction, Ia	0.299	Initial abstraction, Ia	0.041
Time of Concentration, Tc	6.00 MIN	I _a /P	0.151	I _a /P	0.299	I _a / P	0.041
Initial abstraction, Ia	0.273	Unit Peak Discharge, q _u	950	Unit Peak Discharge, qu	950	Unit Peak Discharge, q _u	1000
I _a / P	0.273	Peak discharge, q _{pTv}	0.32 CFS	Peak discharge, q _{pTv}	0.37 CFS	Peak discharge, q _{pTv}	0.09 CFS
Unit Peak Discharge, qu	950						
Peak discharge, q _{pTv}	0.17 CFS	Filtering Device Sizing		Filtering Device Sizing		Filtering Device Sizing	
		Manufacturer	ADS	Manufacturer	ADS	Manufacturer	ADS
Filtering Device Sizing		Filtering Device E	Bayfilter	Filtering Device B	Bayfilter	Filtering Device B	ayfilter
Manufacturer	ADS	Cartridge Model	645	Cartridge Model	645	Cartridge Model	645
Filtering Device I	Bayfilter	Treatment Flowrate	45.00 GPM	Treatment Flowrate	45.00 GPM	Treatment Flowrate	45.00 GPM

Treatment Volume 2500.0 CF

Required Number of Cartridges

Convert to CFS 0.10 CFS

Nitrogen Removal Efficiency (%)	Nitrogen Load from Upstream Practices (Ibs)	Untreated Nitrogen Load to Practice (Ibs)	Nitrogen Removed By Practice (Ibs)	Remaining Nitrogen Load (lbs)
	14. Manufactur	ed BMP (no RR)		
0	0.00	0.00	0.00	0.00
0	0.00	4.14	0.00	4.14

Site Results (Water Quality Compliance)

D.A. B

0.00

0.23

0.23

0.17

0.17

OK.

D.A. B

0.58

0.37

0.21

D.A. C

0.00

0.00

0.00

0.00

0.00

OK.

D.A. C

0

0.00

0.00

0.00

D.A. D

0.00

0.00

0.00

0.00

0.00

OK.

D.A. D

0

0.00

0.00

D.A. E

0.00

0.00

0.00

0.00

0.00

OK.

0

0.00

0.00

0.00 0.00 0.54

D.A. E TOTAL

AREA CHECK

OK.

OK.

OK.

OK.

OK.

0

1.49

0.95

D.A. A

0.00

0.39

0.39

0.12

0.12

OK.

D.A. A

0.91

0.58

0.33

P = 1.0 inch in Virginia I _a taken from Table 4-1 of the NRCS TR-55	:			
	•	A		
$q_{pTv} = q_u \times A \times Q_a$		A is in squa	ire miles	
HSG	R_1	R_{MT}	R_F	
A	0.9	0.20	0.12	
В	0.9	0.23	0.16	
С	0.9	0.27	0.19	
D	0.9	0.29	0.24	
CN Values (from Table 2-2a USDA TR-55 &	& VRRM Spre	adsheet)		
Cover Type	CN			
Impervious	98.0			
Managed Turf, HSG C	74.0			
Residential Disctricts 1/4 acre, HSG C	75.0			

70.0

APPROVAL

DATE

03/04/2022

08/25/2022

12/16/2022

REVISIONS

INITIAL SUBMISSION

THIRD SUBMISSION

SECOND SUBMISSION

MTD - Filter Devices Methodology

Woods, fair condition, HSG C

Treatment Volume 2500.0 CF

Required Number of Cartridges

Convert to CFS 0.10 CFS

 $R_v = (A_1 \times Rv_1) + (A_2 \times Rv_2) + ...(A_n \times Rv_n)$

 $T_v = (R_v \times A) / 12$

 $Q_a = Tv / A$

STORMWATER CALCULATIONS NOTES 1. REFER TO THE CIVIL COVER SHEET FOR ADDITIONAL INFORMATION.

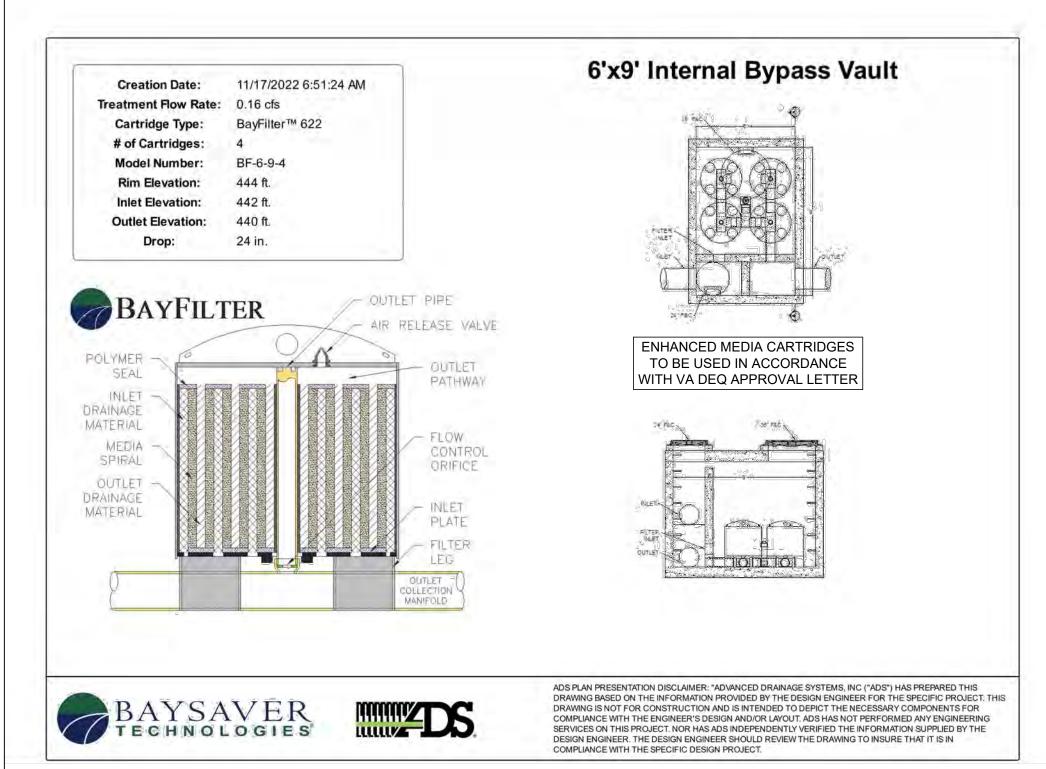
HUSKA CONSULTING, LLC

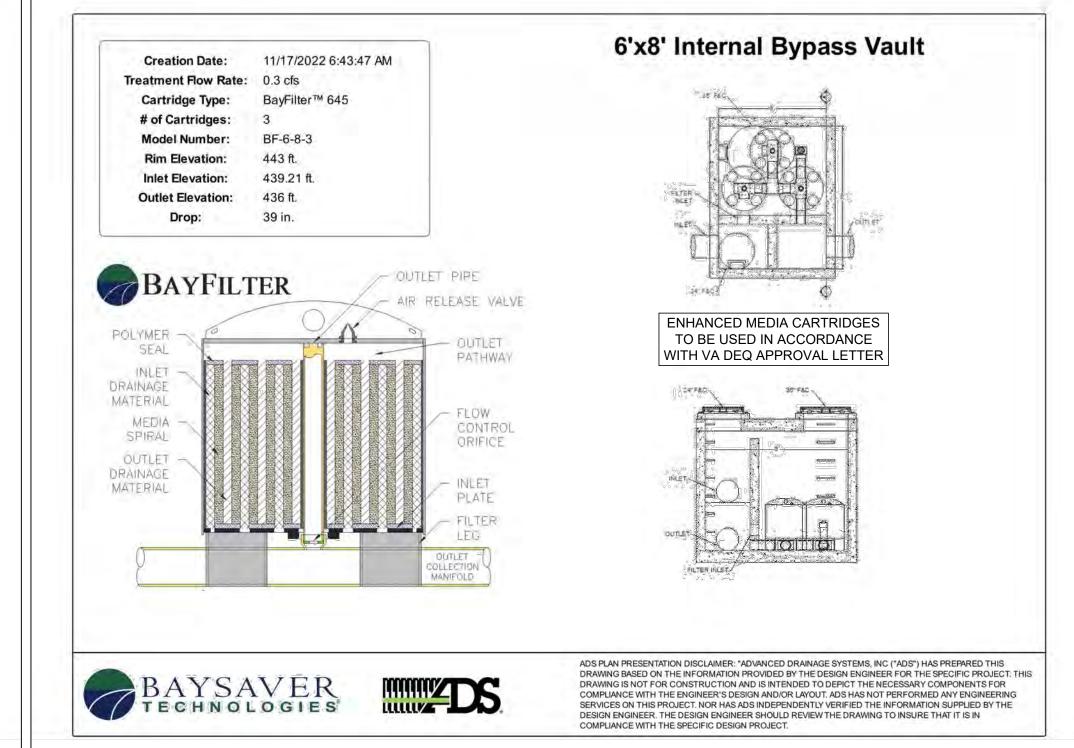
NOT FOR CONSTRUCTION REZONING PLANS 12/16/2022

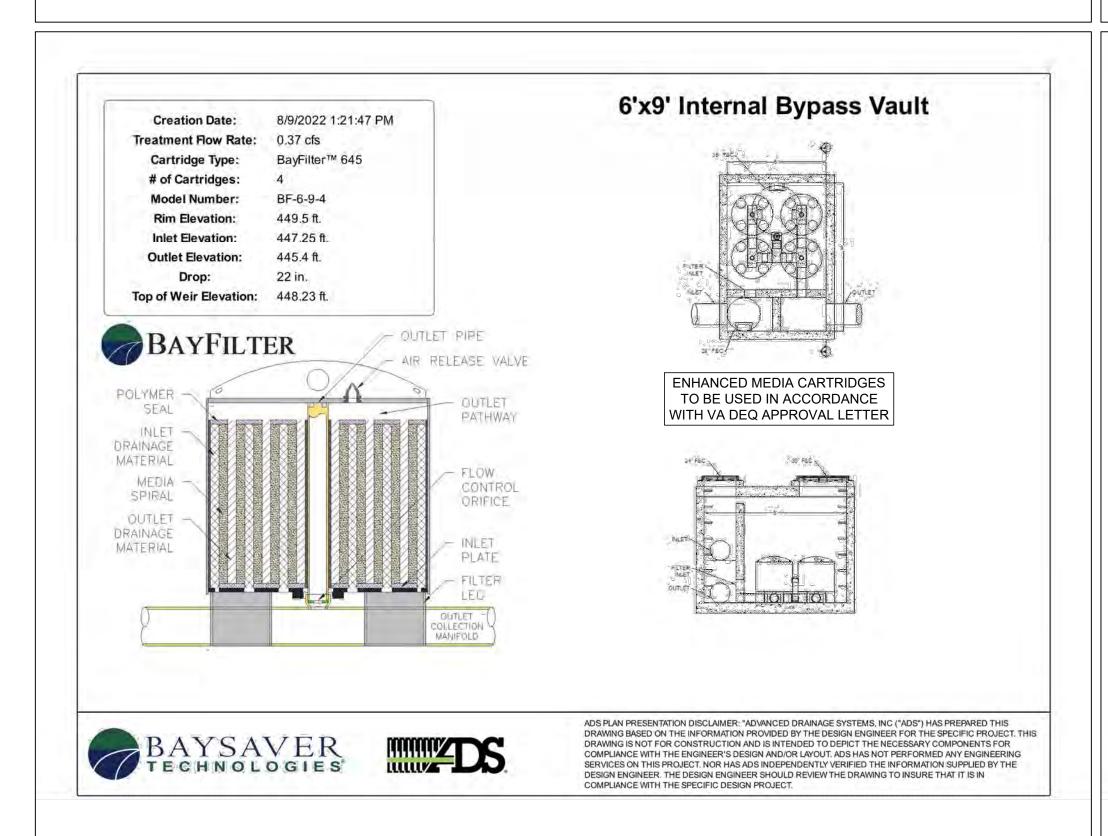
Patrick Horgan PATRICK JOSEPH Lic. No. 061930

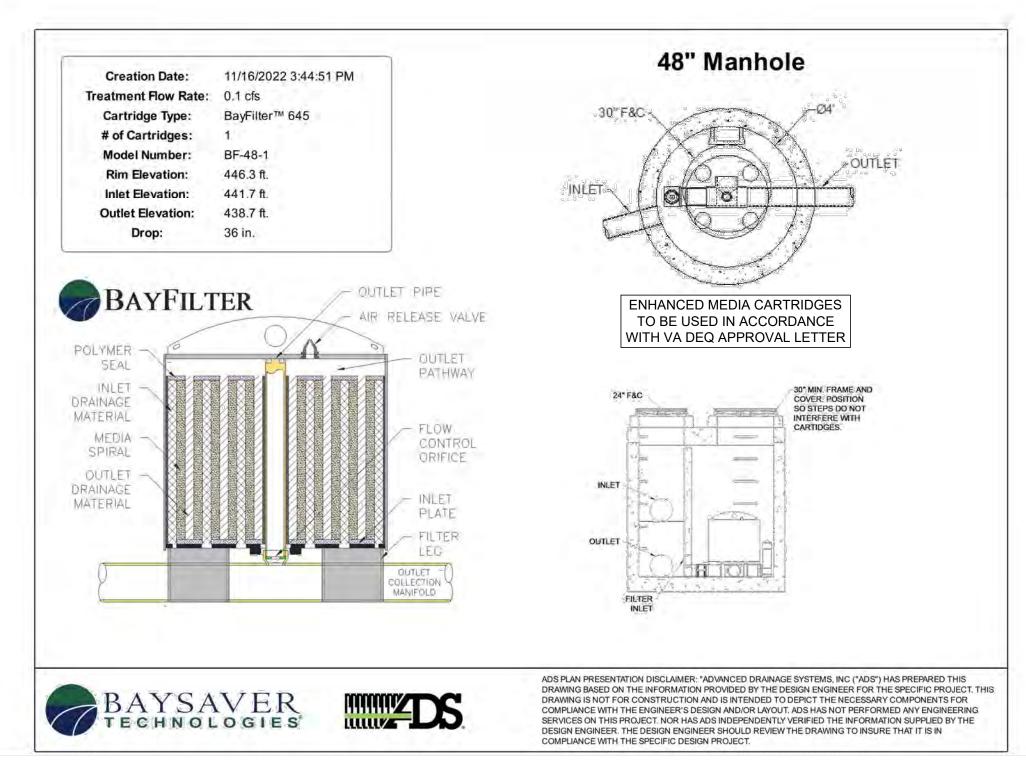
STORMWATER **MANAGEMENT** CALCULATIONS
DRAWING TITLE

013









REVISIONS

INITIAL SUBMISSION

SECOND SUBMISSION

THIRD SUBMISSION

APPROVAL

03/04/2022

08/25/2022

12/16/2022

11004 & 11006 PARK RD FAIRFAX, VA 22306 TAX MAP #57-1-40-002 SQUARE 02, LOT 002

CLIENT EMRE ZIREKOGLU CAGLAYAN INVESTMENT GROUP 32713 LATROBE ST CHANTILLY, VA 20152 571.594.6363

CONTRACTOR

CIVIL ENGINEER PATRICK HORGAN HUSKA CONSULTING, LLC 1050 30TH STREET, NW WASHINGTON, DC 20007 703.425.3862

LAND SURVEYOR DOMINION SURVEYS, INC. 8808-H PEAR TREE VILLAGE COURT ALEXANDRIA, VA 22309 703.619.6555

> Patrick Horgan PATRICK JOSEPH -686E**ATORESAT** Lic. No. 061930

BAYFILTER DETAILS

HUSKA CONSULTING, LLC

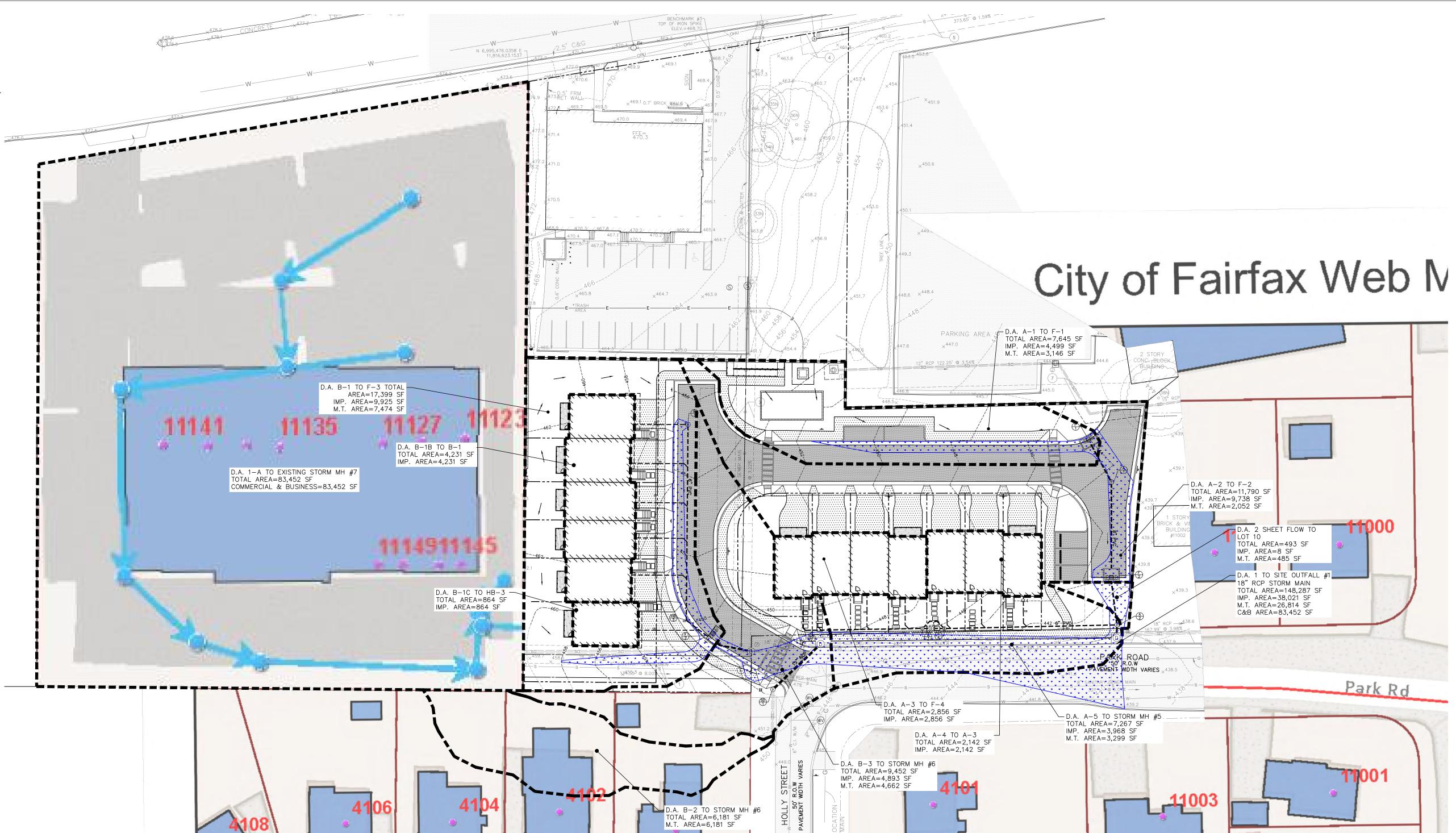
NOT FOR CONSTRUCTION REZONING PLANS 12/16/2022

DRAWING TITLE

DRAINAGE PLAN LEGEND

OVERLAND SHEET FLOW 100-YR OVERLAND RELIEF FLOW BOUNDARY

---- DRAINAGE DIVIDE



ADEQUATE OUTFALL ANALYSIS

THE SUBJECT PROPERTY CONSIST OF TWO DRAINAGE AREAS, 1 & 2, THAT IS BROKEN DOWN INTO THREE DRAINAGE AREAS (1-A, A, & B) FOR THE PURPOSES OF HYDRAULIC ANALYSIS OF THE EXISTING AND PROPOSED STORM DRAIN SYSTEM. THE THREE DRAINAGE AREAS ARE FURTHER BROKEN DOWN INTO DRAINAGE AREAS FOR EACH INDIVIDUAL STORM SEWER STRUCTURE ON THE ABOVE PLAN. THE SITE IS LOCATED IN THE ACCOTINK WATERSHED WITH A TOTAL DRAINAGE AREA 51.0 SQ. MI. (32,640 ACRES).

DRAINAGE AREA 1-A IS 83,452 SF (1.9158 ACRES) AND CONSIST ENTIRELY OF THE COMMERCIALLY ZONED LOT 0140 WEST OF THE SUBJECT PROPERTY. THE RUNOFF IN THE DRAINAGE AREA IS COLLECTED VIA AN ON SITE DRAINAGE SYSTEM AND CONNECTED TO AN EXISTING 18" RCP STORM MAIN THAT RUNS ALONG THE SOUTHERN BORDER OF THE SUBJECT PROPERTY. THE FLOW CONTINUES IN THE CITY STORM DRAIN SYSTEM EAST TO THE SITE OUTFALL, AN EXISTING 18" RCP STORM MAIN IN THE PARK RD R.O.W.

DRAINAGE AREA A IS 31,700 SF (0.7277 ACRES) AND CONSIST OF THE EASTERN PORTION OF THE SUBJECT PROPERTY. THE DRAINAGE AREA IS BROKEN DOWN INTO FIVE SUB-DRAINAGE AREAS FOR THE PURPOSES OF ANALYSIS. THE RUNOFF SHALL BE COLLECTED VIA A PROPOSED ON SITE DRAINAGE SYSTEM AND CONNECTED TO THE SITE OUTFALL VIA A NEW CORE DRILL CONNECTION TO AN EXISTING CURB INLET NEAR THE SOUTHEASTERN CORNER

DRAINAGE AREA B IS 33,032 SF (0.7583 ACRES) AND CONSIST OF THE WESTERN PORTION OF THE SUBJECT PROPERTY. THE DRAINAGE AREA IS BROKEN DOWN INTO THREE SUB-DRAINAGE AREÁS FOR THE PURPOSES OF ANALYSIS. THE RUNOFF SHALL BE COLLECTED VIA A PROPOSED ON SITE DRAINAGE SYSTEM AND CONNECTED TO THE SITE OUTFALL VIA A NEW CORE DRILL CONNECTION TO AN EXISTING MANOLE NEAR THE HOLLY DR AND PARK RD INTERSECTION.

THE LIMIT OF ANALYSIS FOR CHANNEL PROTECTION AND FLOOD PROTECTION IS THE SITE OUTFALL BECAUSE THE SITE'S CONTRIBUTING DRAINAGE AREA IS LESS THAN 1.0% OF THE TOTAL WATERSHED AREA 9VAC25-870-66.B.4.a). NATURAL DRAINAGE DIVIDES ARE HONORED IN THE POST-DEVELOPMENT CONDITION FOR BOTH CONCENTRATED AND NON-CONCENTRATED FLOW.

(1.9158+0.7277+0.7583)/32,640=0.010%

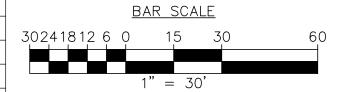
THE POST DEVELOPMENT PEAK FLOW FOR THE 2-YR STORM EVENT IS 5.88 CFS. THE EXISTING 18" RCP STORM MAIN IN THE PARK RD R.O.W HAS ADEQUATE CAPACITY TO CONVEY THE 2-YR PEAK FLOW, SEE CALCULATIONS SHEET 016 (FAIRFAX COUNTY CODE SECTION 124-4-4.B.1.a).

THE POST DEVELOPMENT PEAK FLOW RATE FROM THE 10-YR STORM EVENT IS 10.14 CFS. THERE IS NO EVIDENCE THAT THE EXISTING STORMWATER CONVEYANCE SYSTEM CURRENTLY EXPERIENCES LOCALIZED FLOODING. THE EXISTING 18" RCP STORM MAIN IN THE PARK RD R.O.W HAS ADEQUATE CAPACITY TO CONVEY THE 10-YR PEAK FLOW, SEE CALCULATIONS SHEET 016 (9VAC25-870-66.C.1).

DRAINAGE AREA 2 IS 493 SF (0.0113 ACRES). THE DRAINAGE AREA SHEET FLOWS WEST TO EAST AND LEAVES THE SUBJECT PROPERTY AS SHEET FLOW. THE TOTAL IMPERVIOUS AREA IS REDUCED IN THE POST DEVELOPMENT CONDITION. THEREFORE, THE PEAK FLOW RATE IN THE POST DEVELOPMENT CONDITION IS LESS THAN THAT OF THE PRE DEVELOPMENT CONDITION, SEE CALCULATIONS SHEET 016 (9VAC25-870-66.D). NATURAL DRAINAGE DIVIDES ARE HONORED IN THE POST-DEVELOPMENT CONDITION FOR BOTH CONCENTRATED AND NON-CONCENTRATED FLOW.

IT IS THE OPINION OF THE ENGINEER THAT REQUIREMENTS FOR THE ADEQUACY OF THE DOWNSTREAM DRAINAGE SYSTEM HAVE BEEN MET; AND IT IS THE ENGINEER'S PROFESSIONAL OPINION THAT NO ADJACENT OR DOWNSTREAM PROPERTIES WILL SUFFER ADVERSE IMPACTS DUE TO THIS PROPOSED DEVELOPMENT ACTIVITY.

APPROVAL	DATE	REVISIONS	
	03/04/2022	INITIAL SUBMISSION	
	08/25/2022	SECOND SUBMISSION	
	12/16/2022	THIRD SUBMISSION	





NOT FOR CONSTRUCTION REZONING PLANS

DRAINAGE PLAN

Patrick Horgan PATRICK JOSEPH

HORGAN

Lic. No. 061930 12/17/2022

11004 & 11006 PARK RD FAIRFAX, VA 22306 TAX MAP #57-1-40-002 SQUARE 02, LOT 002

CLIENT

EMRE ZIREKOGLU

32713 LATROBE ST CHANTILLY, VA 20152

571.594.6363

CONTRACTOR

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LAND SURVEYOR

HUSKA CONSULTING, LLC

DOMINION SURVEYS, INC.

ALEXANDRIA, VA 22309

8808-H PEAR TREE VILLAGE COURT

1050 30TH STREET, NW WASHINGTON, DC 20007

CAGLAYAN INVESTMENT GROUP

DRAWING TITLE

12/16/2022 HUSKA CONSULTING, LLC DRAWING NO. DocuSign Envelope ID: ECC182DD-B804-4FA1-A578-120CA02447B8 TR-55 Calculations - Water Quantity Compliance D.A. 2 Sheet Flow to Lot 10 1-year 24-hr rainfall depth, P1 = Total Area **Water Quantity Compliance Methodology** 2-year 24-hr rainfall depth, P2 = 351 SF Rainfall depths per the Virginia Stormwater Management Handbook Vol. II Pre Dev. Managed Turf, A_{MT} = 10-year 24-hr rainfall depth, P10 = 485 SF Initial abstraction, $Ia = 0.2 \times S$ Post Dev. Managed Turf, A_{MT} = Thus, $Q = ((P - 0.2 \times S)^2) / (P + 0.8 \times S)$ Pre Dev. Impervious Cover, A_{IMP} = D.A. 1 to Site Outfall #1 Post Dev. Impervious Cover, AIMP = Potential max. retention, S = 1000/CN - 10 Total Area 148,279 SF 80.9 Unit peak discharge, qu = 500 Pre Dev. Curve Number, CN_{pre} = Pre Dev. Managed Turf, A_{MT} = Drainage area (mi^2), Am = (Aexcom + Aeximp) / (660) Post Dev. Curve Number, CN_{post} = 74.4 Post Dev. Managed Turf, A_{MT} = Peak discharge, q = qu x Am x Q x Fp Pre Dev. Potential Max. Abstraction, Spre = 2.36 IN Pre Dev. Impervious Cover, A_{IMP} = Post Dev. Potential Max. Abstraction, Spost = CN Values (from Table 2-2a USDA TR-55 & VRRM Spreadsheet) Post Dev. [mpervious Cover, $A_{IMP} =$ 0.99 IN Pre Dev. 1-yr Adj. Runoff, Q1pre = Commerical & Business Area, Ac&B **Cover Type** Post Dev. 1-yr Adj. Runoff, Q1post = 0.67 IN 98.0 Pre Dev. Curve Number, CN_{pre} = Impervious Pre Dev. 2-yr Adj. Runoff, Q2pre = 1.39 IN 74.0 Post Dev. Curve Number, CN_{post} = Post Dev. 2-yr Adj. Runoff, Q2post = Managed Turf, HSG C 1.00 IN 70.0 Pre Dev. Potential Max. Abstraction, Spre = Woods, fair condition, HSG C Pre Dev. 10-yr Adj. Runoff, Q10pre = 2.78 IN Post Dev. Potential Max. Abstraction, Spost = Post Dev. 10-yr Adj. Runoff, Q10post = 2.22 IN Pre Dev. 1-yr Adj. Runoff, Q1pre = Pre Dev. 1-yr Peak Runoff Flowrate, q1pre = 0.009 CFS Post Dev. 1-yr Adj. Runoff, Q1post = Post Dev. 1-yr Peak Runoff Flowrate, q1post = 0.006 CFS Pre Dev. 2-yr Adj. Runoff, Q2pre = 1.96 IN Pre Dev. 2-yr Peak Runoff Flowrate, q2pre = 0.012 CFS Post Dev. 2-yr Adj. Runoff, Q2post = 2.21 IN Post Dev. 2-yr Peak Runoff Flowrate, q2post = 0.009 CFS Pre Dev. 10-yr Adj. Runoff, Q10pre = 3.52 IN Pre Dev. 10-yr Peak Runoff Flowrate, q10pre = 0.025 CFS

Δ1-yr Peak Runoff, Q_{1Δ}

Δ2-yr Peak Runoff, Q_{2Δ}

Δ 10-yr Peak Runoff, Q_{10Δ}

Post Dev. 10-yr Peak Runoff Flowrate, q10post =

11004 & 11006 PARK RD FAIRFAX, VA 22306 TAX MAP #57-1-40-002 SQUARE 02, LOT 002

CLIENT EMRE ZIREKOGLU CAGLAYAN INVESTMENT GROUP 32713 LATROBE ST CHANTILLY, VA 20152 571.594.6363

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CIVIL ENGINEER PATRICK HORGAN HUSKA CONSULTING, LLC 1050 30TH STREET, NW WASHINGTON, DC 20007 703.425.3862

LAND SURVEYOR DOMINION SURVEYS, INC. 8808-H PEAR TREE VILLAGE COURT ALEXANDRIA, VA 22309 703.619.6555

12/17/2022 Park Rd Townhomes wer Conveyance - Hydraulic Gradeline Calculations

			e - Hydro		·		Hydra	ulics											Hydr	ology							Cir	cular Chai	nnel Ratio	s ⁸			Flow Type	From	То	WSE _{down} 1		A _{full}
		Pipe Inv	verts			Pipe	e Parame	eters			Flow	, Parame	ters					Drainag	e Area ¹			,	Addition	al Flow	Velo	ocity	Flow	rate	Flow	Area	Hydraulic	Radius				(ft)	(in)	(sf)
Up	strea	· .	Downs	tream	Length			n	Slope	100	v	Q	R	Α	Imp.	M.T.	B&C	_	s	Runoff	Peak Q		-	ک _{add'l} US ⁷ ۱			Q ₁₀₀ /Q _{full}		A ₁₀₀ /A _{full}		R ₁₀₀ /R _{full}			SD 6	SD 1	446.15	15	1.
ID.		nvert	ID	Invert	(ft)	(in)			(ft/ft)	(in/hr)	(fps)	(cfs)	(ft)	(sf)	(SF)	(SF)	(SF)	CN	(in)	(in)	(cfs)	(cfs)	(cfs)	(cfs)	100. 10.1	(fps)	-2001 -1011	(cfs)	1001 1011	(sf)	100. 1411	(ft)						1
F-1	\rightarrow	140.00	A1	439.78	22.22	8	PVC	0.011	1.00%	3.11	 	· ·	0.11		4446	3156		88.04		1 92	0.26		0.00	0.00	0.75	4.09	0.18	\longrightarrow	0.24	0.35	0.65	1/	CHANNEL	SD 4	SD 5	429.93	18	1.
A1	-	439.68	F-2	439.22	_		PVC	0.011	1.00%	3.11			0.11		0	0	0	0.00		0.00	_		0.00	0.00	0.75	4.09	0.18			0.35			CHANNEL	SD 5	SD 6	436.71	18	1.
F-2		436.00	A2	435.70	22.16		PVC	0.011	1.35%	3.11			0.15		9760	2065	0	93.81	0.66	2.44			0.00	0.00	0.83	5.52	0.26	3.01		0.55			CHANNEL	SD 6	SD 7	444.44	18	1.
A2		\rightarrow	SD 5	435.52	+	10	PVC	0.011	2.00%	3.11			+		0	0	0	0.00	+	0.00			0.00	0.00	0.85	6.71	0.28	3.66		0.55	-		CHANNEL	SD 7	F3	445.69	10	0.
	<u> </u>			144142			1	0.022	2.0075	J.	7.14		V. 2.0				Ť	V. V	0.00	4.44	2.44		0.00		V.00	4.7.2		3.00	4.42	7.77	<u> </u>	V.L.		F3	HB3	447.56	6	0.
HB2	4	145.40	A3	443.80	66.05		PVC	0.011	2.42%	3.11	3.64	0.15	0.07	0.04	2856	n	ο	98.00	0.20	2.88	0.15	0.15	0.00	0.00	0.69	5.26	0.14	1.03	0.19	0.20	0.58	0.13	CHANNEL	HB3	B1	448.79	6	0.1
A3		141.80	F-4	441.70	2.00		PVC	0.011	5.00%	3.11		0.15	0.06	-	0	n	0	0.00		0.00			0.00	0.00	0.61	7.55	0.10	1.48		0.20			CHANNEL					
F-4	-	438.70		438.55	+ +	- 6	PVC	0.011	2.46%	3.11		0.15	0.07	0.03	0	n	0	0.00	t	0.00			0.00	0.00	0.69	5.30	0.14	1.04		0.20	-		CHANNEL	A2	A4	436.95	6	0
A4	-	438.45	A2	436.60	+ +		PVC	0.011	2.08%	3.11			0.09	0.06	2142	n	0	98.00		2.88			0.00	0.00	0.84	4.87	0.27	0.96		0.20	-		CHANNEL	A4	F-4	438.86	6	0.2
	- '	150.15	n _E	430.00	00.00		1,00	0.011	2.0078	3.11	4.00	Ų.ZU	0.05	Ų.UU	2172			30.00	0.20	2.00	Ų.11	Ų.ZU	0.00	0.00	0.01	7.07	0.27	0.50	0.30	0.20	0.75	0.13	CHANTEL	F-4	A3	441.98	6	0.2
81	4	452.30	НВЗ	448 35	104.59	6	PVC	0.011	3.78%	3.11	4.75	0.22	0.08	0.04	4231	n	0	98.00	0.20	2.88	0.22	0.22	0.00	0.00	0.72	6.56	0.17	1.29	0.21	0.20	0.62	N 13	CHANNEL	А3	HB2	444.11	6	0.2
HB3		148.35	F3	447.25				0.011	7.01%	3.11			0.07	0.04	864	n	0	98.00		2.88	0.04		0.00	0.00	0.69	8.94	0.15						CHANNEL					
F3			SD 7	445.15		10	PVC		1.41%	3.11			0.14	0.14	4830	7474	0	83.42		1.57	0.35		0.00	0.00	0.77	5.65	0.20	3.08		0.55			CHANNEL	SD 5	A2	436.71	10	0.5
SD 7			SD 6	443.42	_	18	RCP	0.013	2.90%	3.11			0.27		0.000	, ., ,	83452			2.46	3.68		0.00	0.00	0.81	10.12	0.24			1.77			CHANNEL	A2	F-2	436.93	10	0.1
SD 6	_		SD 5		161.62		RCP	0.013	4.86%	3.11		4.70	0.26		4893	4662	03152	86.29		1 78	0.30		0.00	0.00	0.78	13.10	0.20		0.26	1.77			CHANNEL	F-2	A1	439.64	8	0.3
SD 5			SD 4		167.99		RCP	0.013	3.98%	3.11			0.29		3968	3299	0	87.10		1.84	 		0.00	0.00	0.85	11.86	0.29			1.77			CHANNEL	A1	F-1	440.20	8	0.3
							112																											Hydraulic	Gradeline	e Calculation	ıs Method	ology
SD 1	L 4	446.50	SD 6	444.55	32.12	1 5	RCP	0.012	6.07%	3.11	2.81	0.11	0.04	0.02	0	6181	0	74.00	3.51	0.98	0.11	0.11	0.00	0.00	0.20	14.05	0.01	17.24	0.02	1.23	0.13	0.31	CHANNEL			0.453Q ² n ² /A ² l		H _{fr} ,fricti

0.020 CFS

-0.003 CFS

-**0.003** CFS

-0.005 CFS

Hydrology and Hydraulic Calculations Methodology Note all sewer conveyance calculations shown here are for the 100 year storm event

Post Dev. 10-yr Adj. Runoff, Q10post =

Pre Dev. 1-yr Peak Runoff Flowrate, q1pre =

Pre Dev. 2-yr Peak Runoff Flowrate, q2pre =

Pre Dev. 10-yr Peak Runoff Flowrate, q10pre =

Post Dev. 10-yr Peak Runoff Flowrate, q10post = 10.136 CFS

Δ1-yr Peak Runoff, Q_{1Δ}

Δ2-yr Peak Runoff, Q_{2Δ}

Δ10-yr Peak Runoff, Q_{10Δ}

12/17/2022

Post Dev. 1-yr Peak Runoff Flowrate, q1post =

Post Dev. 2-yr Peak Runoff Flowrate, q2post =

Park Rd Townhomes

n, Manning's roughness coefficient I, rainfall intensity V, velocity Q, flowrate R, hydraulic radius A, flow area

3.81 IN

3.964 CFS

5.879 CFS

0.602 CFS

0.788 CFS

98.00 for impervious areas and are 74 for managed turf, HSG C 94 for business & commercial, HSG C Design Storm is 100-yr, 24-hr: TR-55 CN values are

Time of concentration of flow to upstream structure of run by direct, overland flow if an inlet. Else take as 5 minutes. If not 5 minutes, provide separate To calculation justification

Flow time in pipe from upstream structure in run to downstream structure in run

Time of concentration of flow to downstream structure via storm sewer system

Controlling time of concentration of flow to upstream structure At the engineer's option, an additional flowrate may be added which will propagate downstream in the system. This flowrate is not affected by time of concentration.

The sum of the additional flowrates added to the system upstream of the run in question.

Circular channel ratios are tabulated in the reference tab and have nested if statements that hinge on the flow type for the pipe run in question For BMP overflows manually enter the adjust curve number from the VRRM worksheet

Park Rd Townhomes 12/17/2022

Sewer Conveyance - Hydrology and	d Hydraulic Calculations
	Hydra

						Hydra	ulics											Hydr	ology							Cir	rcular Cha	nnel Ratio	os ⁸			Flow Type	From	То	WSE _{down} 1	D
	Pipe I	nverts			Pipe	e Parame	eters			Flow	/ Parame	ters					Drainag	e Area ¹				Additiona	al Flow	Velo	ocity	Flow	/rate	Flow	Area	Hydrau	lic Radius				(ft)	(in)
Ups	tream	Downs	stream	Length	Diam.	Mat'l	n	Slope	100	V	Q	R	Α	Imp.	M.T.	в&С	CN	S	Runoff	Peak Q	ΣQDA	Q _{addT} Q	l _{add1} US	V ₁₀₀ /V _{full}	V _{roll}	Q ₁₀₀ /Q _{full}	Q _{full}	A ₁₀₀ /A _{full}	A _{full}	R ₁₀₀ /R _{ful}	II R _{full}		SD 6	SD 1	446.45	15
ID	Invert	ID	Invert	(ft)	(in)				(in/hr)	(fps)	(cfs)	(ft)	(sf)	(SF)	(SF)	(SF)	CN	(in)	(in)	(cfs)	(cfs)		(cfs)		(fps)		(cfs)		(sf)		(ft)		(
F-1	440.00	A1	439.78	22.22	8	PVC	0.011	1.00%	4.78	3.68	0.47	0.14	0.13	4446	3156	, , ,	88.04	1.36	3.46	0.47	0.47	0.00	0.00	0.90	4.09	0.33	1.43	0.37	0.35	5 0.85	- ' '-	CHANNEL	SD 4	SD 5	429.93	18
A1	439.68	F-2	439.22	_	8	PVC	0.011	1.00%	4.78	3.68	0.47	0.14	0.13	0	0		0.00	0.00		0.00	 	0.00	0.00	0.90	4.09	_	_	 	1		_	CHANNEL	SD 5	SD 6	437.16	18
F-2	436.00		435.70	22.16	10	PVC	0.011	1.35%	4.78		1.34	0.19	0.25	9760	2065		93.81	0.66		0.86	-	0.00	0.00	0.96	5.52	_	3.01			_	_	CHANNEL	SD 6	SD 7	444.61	18
A2	435.60		435.52	_	10	PVC	0.011	2.00%	4 78	6.59	1.74	0.20	0.26	0	0		0.00	0.00		0.00		0.00	0.00	0.98	6.71	1			 	_		CHANNEL	SD 7	F3	445.79	10
	100.00	55.5	100.02	1107			0.022	2.0070		0.00	2., 1	0.20	0.20	-		<u> </u>	0.00	0.00	0.00	0.00		0.00	0.00	0.50	0.,,	0.10	3.00	0.10	0.5.	0,54	0,22		F3	HB3	447.59	6
НВ2	445.40	А3	443.80	66.05	6	PVC	0.011	2.42%	4.78	4.21	0.23	0.09	0.05	2856	0		98.00	0.20	4.54	0.23	0.23	0.00	0.00	0.80	5.26	0.23	1.03	0.28	0.20	0.71	1 0.13	CHANNEL	HB3	B1	448.93	6
A3	441.80		441.70	2.00	-6	PVC	0.011	5.00%	4.78			0.08	0.04	2030	0		0.00	0.00		0.00		0.00	0.00	0.71	7.55	1		 	1		+	CHANNEL	(1
F-4	438.70		438.55	_		PVC	0.011	2.46%	4.78	4.24	0.23	0.09	0.05	0			0.00	0.00		0.00	 	0.00	0.00	0.80		_	1.04		+	+	_	CHANNEL	A2	A4	437.73	6
	438.45		436.60			PVC	0.011	2.08%	4.78		0.23	0.03	0.03	2142	0		98.00	0.20		0.00	0.23	0.00	0.00	0.80	4.87	1	0.96	 				CHANNEL	A4	F-4	438.89	6
A4	430.43	A2	430.00	00.00	0	PVC	0.011	2.00%	4.70	4.04	0.41	0.11	0.09	2142	U		90.00	0.20	4.54	0.17	0.41	0.00	0.00	0.95	4.67	0.43	0.90	0.44	0.20	0.92	2 0.13	CHANNEL	F-4	A3	442.01	6
D.1	452.20	LIDA	440.05	104 50		D) (C	0.011	2.700/	4.70	5 50	0.24	0.00	0.00	4224		 	00.00	0.20	4.54	0.24	0.24	0.00	0.00	0.04	6.50	0.27	1 20	0.20	0.20	0 0 70	0.13	CHANNEL	A3	HB2	444.14	6
81	452.30	HB3	+	104.59		PVC	0.011	3.78%	4.78	5.50		0.09	0.06	4231	0		98.00	0.20		0.34	_	0.00	0.00	0.84	6.56		1.29	 	-			CHANNEL	1			Ī
HB3	448.35	- 	447.25			PVC	0.011	7.01%	4.78		0.42	0.09	0.06	864	- 4-4		98.00	0.20	+	0.07		0.00	0.00	0.81	8.94		1.76	†	•	+	+	CHANNEL	SD 5	A2	437.16	10
F3	445.40	SD 7	445.15		10	PVC	0.011	1.41%	4.78		1.08	0.18	0.22	4830	/4/4	L C	83.42	1.99		0.67	-	0.00	0.00	0.92	5.65		3.08	 		_		CHANNEL	A2	F-2	437.73	1
SD 7	444.40	 	443.42	+ +	18	RCP	0.013	2.90%	4.78	+	7.20		0.74	0	0	83452	94.00	0.64	+ +	6.12		0.00	0.00	0.94	-	i		 	1		+	CHANNEL	F-2	A1	439.72	- <u>20</u>
SD 6	443.08	SD 5	435.23	161.62	18	RCP	0.013	4.86%	4.78	11.92	8.01	0.32	0.68	4893	4662	C	86.29	1.59	3.29	0.56	8.01	0.00	0.00	0.91	13.10	0.35	23.15	0.38	1.77	7 0.86		CHANNEL		F-1	440.28	
SD 5	435.42	SD 4	428.73	167.99	18	RCP	0.013	3.98%	4.78	11.72	10.19	0.36	0.86	3968	3299	C	87.10	1.48	3.37	0.44	10.19	0.00	0.00	0.99	11.86	0.49	20.96	0.48	1.77	7 0.97	7 0.38	CHANNEL	A1		440.28	
																																	<u>Hydraulic</u>	<u>Gradeline</u>	e Calculation	ns Method
SD 1	446.50	SD 6	444.55	32.12	15	RCP	0.012	6.07%	4.78	5.62	0.24	0.08	0.06	0	6181	C	74.00	3.51	2.19	0.24	0.24	0.00	0.00	0.40	14.05	0.02	17.24	0.05	1.23	3 0.25	5 0.31	CHANNEL	S _{fr} , friction	ı slope = C	0.453Q ² n ² /A ²	R ^{4/3}

Note all sewer conveyance calculations shown here are for the 100 year storm event n, Manning's roughness coefficient I, rainfall intensity V, velocity Q, flowrate R, hydraulic radius A, flow area

TR-55 CN values are **98.00** for impervious areas and are **74** for managed turf, HSG C 94 for business & commerical, HSG C Design Storm is 100-yr, 24-hr:

Time of concentration of flow to upstream structure of run by direct, overland flow if an inlet. Else take as 5 minutes. If not 5 minutes, provide separate Tc calculation justification

Flow time in pipe from upstream structure in run to downstream structure in run

Time of concentration of flow to downstream structure via storm sewer system Controlling time of concentration of flow to upstream structure

At the engineer's option, an additional flowrate may be added which will propagate downstream in the system. This flowrate is not affected by time of concentration.

The sum of the additional flowrates added to the system upstream of the run in question.

Circular channel ratios are tabulated in the reference tab and have nested if statements that hinge on the flow type for the pipe run in question

For BMP overflows manually enter the adjust curve number from the VRRM worksheet

ype	From	То	WSE _{down} 1	D	A _{full}	Q	L	R _{full}	n	S _{fr}	H _{fr}	V _{out}	H _o ²	V _{in} ³	H _i ⁴	Angle⁵	К	H _{bend} 6	Plunging ⁷	IS-1 ⁸	H _{str} ⁹	H _{total}	WSE _{up}	Top El ¹⁰	Top - WSE _{up}	Remarks
			(ft)	(in)	(sf)	(cfs)	(ft)	(ft)		(ft/ft)	(ft)	(fps)	(ft)	(fps)	(ft)	(degrees)		(ft)			(ft)	(ft)	(ft)	(ft)	(ft)	(ft)
	SD 6	SD 1	446.15	15	1.23	0.11	32.12	0.31	0.012	0.00%	0.00	2.81	0.04	2.81	0.00	0	0.00	0.00	YES	NO	0.05	0.05	446.56	450.00	3.44	ADEQUATE
EL	SD 4	SD 5	429.93	18	1.77	5.98	167.99	0.38	0.013	0.32%	0.54	10.13	0.40	10.22	0.00	90	0.70	0.36		NO	0.76	1.30	435.96	439.44	3.48	ADEQUATE
EL	SD 5	SD 6	436.71	18	1.77	4.70	161.62	0.38	0.013	0.20%	0.32	10.22	0.41	8.20	0.00	45	0.47	0.06	NO	NO	0.46	0.79	443.53	447.61		ADEQUATE
EL	SD 6	SD 7	444.44	18	1.77	4.29	33.80	0.38	0.013	0.17%	0.06	8.20	0.26	4.33	0.00	45	0.47	0.14	YES	NO	0.51	0.57	444.89	452.59	7.70	ADEQUATE
EL	SD 7	F3	445.69	10	0.55	0.61	17.67	0.21	0.011	0.06%	0.01	4.33	0.07	6.19	0.00	45	0.47	0.28	YES	NO	0.45	0.46	445.70	449.50	3.80	ADEQUATE
	F3	HB3	447.56	6	0.20	0.26	15.69	0.13	0.011	0.16%	0.02	6.19	0.15	4.75	0.00	45	0.47	0.16	NO	NO	0.31	0.34	448.48	455.30	6.82	ADEQUATE
EL	HB3	B1	448.79	6	0.20	0.22	104.59	0.13	0.011	0.11%	0.11	4.75	0.09	4.75	0.00	0	0.00	0.00	YES	YES	0.06	0.17	452.43	455.30	2.87	ADEQUATE
EL																										
EL	A2	A4	436.95	6	0.20	0.26	88.80	0.13	0.011	0.15%	0.13	4.08	0.06	3.67	0.07	0	0.00	0.00	NO	YES	0.07	0.20	438.62	446.00	7.38	ADEQUATE
EL	A4	F-4	438.86	6	0.20	0.15	6.09	0.13	0.011	0.05%	0.00	3.67	0.05	4.61	0.12	0	0.00	0.00	NO	NO	0.17	0.17	438.86	446.30	7.44	ADEQUATE
	F-4	А3	441.98	6	0.20	0.15	2.00	0.13	0.011	0.05%	0.00	4.61	0.08	3.64	0.07	0	0.00	0.00	NO	YES	0.08	0.08	441.98	446.30	4.32	ADEQUATE
EL	A3	HB2	444.11	6	0.20	0.15	66.05	0.13	0.011	0.05%	0.03	3.64	0.05	3.64	0.07	0	0.00	0.00	NO	NO	0.12	0.16	445.53	449.70	4.18	ADEQUATE
EL																										
EL	SD 5	A2	436.71	10	0.55	1.04	4.07	0.21	0.011	0.16%	0.01	5.73	0.13	4.58	0.11	90	0.70	0.18	NO	YES	0.21	0.22	436.72	440.40	3.68	ADEQUATE
EL	A2	F-2	436.93	10	0.55	0.78	22.16	0.21	0.011	0.09%	0.02	4.58	0.08	3.08	0.05	0	0.00	0.00	YES	NO	0.17	0.19	436.95	443.00	6.05	ADEQUATE
EL	F-2	A1	439.64	8	0.35	0.26	46.04	0.17	0.011	0.03%	0.02	3.08	0.04	3.08	0.05	0	0.00	0.00	NO	YES	0.04	0.06	439.87	443.65	3.78	ADEQUATE
EL	A1	F-1	440.20	8	0.35	0.26	22.22	0.17	0.011	0.03%	0.01	3.08	0.04	3.08	0.05	0	0.00	0.00	YES	NO	0.11	0.12	440.21	444.00	3.79	ADEQUATE

 H_{Λ} , structure bend loss= $K*V_i^2/2g$ H_{str} , structure loss = $H_o + H_i + H_\Delta$ H_{total} , total head loss = $H_{fr} + H_{str}$

V_o,velocity out

Water surface elevation in bottom structure of pipe run. For the first (most downstream) run of HGL analysis per VDOT standards use the greater of the tailwater elevation (if known) or 80% full depth.

Expansion loss for upper structure of pipe run. If the upstream structure is a wye, the expansion losses are taken as zero. Velocity of water entering pipe run. If pipe run is at the top of the system, set this to the velocity out of the pipe run. Otherwise, use upstream pipe's velocity. If multiple pipes feed in, use the inlet velocity with the greatest momentum (QxV)

Contraction loss for upper structure of pipe run. If the upstream structure is a wye, the expansion losses are taken as zero.

 H_o , structure outlet loss=0.25(0.3 if top pipe)* $V_0^2/2g$

g,gravity=32.2

g,gravity=32.2 V_i,velocity in

۷_i,velocity in

H_i, structure inlet loss=0.35*V_i²/2g

Angle of deflection in the horizontal plane between the upper structure of the pipe run in question and the next upstream pipe. If multiple pipes in, this is the angle of the pipe which creates the most headloss. If no pipes in, set to zero.

Bend loss for upper structure of pipe run. By default this formula uses the listed inlet velocity. However, if multiple pipes feed into this run bend losses must be calculated for all inflowing pipes and the maximum chosen.

If 20%+ of the total flow is coming from a curb/grate inlet, or if there's an inlet pipe with an invert greater than the crown of the outlet pipe, plunging losses apply.

The engineer may specify IS-1 inlet shaping for a structure which allows the inlet head losses to be reduced by 50%.

 H_{fr} , friction loss = $L*S_{fr}$

Structure loss (sum of expansion, contraction, and bend loses) for the upstream structure of the pipe run.

Top elevation of upper structure of pipe run.

Park Rd Townhomes		12/17/2022
Causer Canuaranaa	Hudraulia Gradalina C	alculations

	Park Rd To	ownnome	es		12/1/	/2022																				
	Sewer Cor	nveyance	- Hydraulic (Gradeline	Calculation	ıs																				
low Type	From	То	WSE _{down} ¹	D	A _{full}	Q	L	R _{full}	n	S _{fr}	H _{fr}	V_{out}	H _o ²	V _{in} ³	H _i ⁴	Angle⁵	К	$H_{bend}^{}^{}}}$	Plunging ⁷	IS-1 ⁸	H _{str} ⁹	H _{total}	WSE _{up}	Top El ¹⁰	Top - WSE _{ul}	Remarks
			(ft)	(in)	(sf)	(cfs)	(ft)	(ft)		(ft/ft)	(ft)	(fps)	(ft)	(fps)	(ft)	(degrees)		(ft)			(ft)	(ft)	(ft)	(ft)	(ft)	(ft)
	SD 6	SD 1	446.45	15	1.23	0.24	32.12	0.31	0.012	0.00%	0.00	5.62	0.15	5.62	0.00	0	0.00	0.00	YES	NO	0.19	0.19	446.63	450.00	3.38	ADEQUATE
	SD 4	SD 5	429.93	18	1.77	10.19	167.99	0.38	0.013	0.94%	1.58	11.72	0.53	11.92	0.00	90	0.70	0.47	NO	NO	1.01	2.59	436.15	439.44	3.29	ADEQUATE
HANNEL	SD 5	SD 6	437.16	18	1.77		161.62	0.38	0.013	0.58%	0.94	11.72	0.55	9.51	0.00	45	0.70	0.47	 	NO	0.78	1.72	443.70	447.61	3.92	ADEQUATE
HANNEL	SD 6	SD 7	444.61	18	1.77		33.80	0.38	0.013	0.47%	0.16	9.51	0.35	5.19	0.00	45	0.47	0.23		NO	0.78	0.87	445.06		7.53	ADEQUATE
HANNEL	SD 7	F3	445.79	10	0.55		17.67	0.30	0.013	0.17%	0.03	5.19	0.10	7.24		45	0.47	0.38	 	NO	0.63	0.66	445.82	449.50	3.68	ADEQUATE
HANNEL	F3	HB3	447.59		0.20	0.42	15.69	0.13	0.011	0.39%	0.06	7.24	0.20	5.50	0.00	15	0.47	0.22	+	NO	0.42	0.48	448.51	455.30	6.79	ADEQUATE
	HB3	B1	448.93		0.20	0.42	104.59	0.13	0.011	0.33%	0.28	5.50	0.12	5.50	0.00	0	0.47	0.22	 	NO	0.42	0.48	452.47	455.30	2.83	ADEQUATE
HANNEL	1103	D1	440.33		0.20	0.54	104.55	0.13	0.011	0.2770	0.20	3.30	0.12	3.30	0.00		0.00	0.00	11.5	110	0.13	0.44	432.47	433.30	2.03	ADEQUATE
HANNEL	A2	A4	437.73	6	0.20	0.41	88.80	0.13	0.011	0.38%	0.34	4.64	0.08	4.24	0.10	0	0.00	0.00	NO	NO	0.18	0.52	438.68	446.00	7.32	ADEQUATE
HANNEL	A4	F-4	438.89	6	0.20	0.23	6.09	0.13	0.011	0.12%	0.01	4.24	0.07	5.36	0.16	0	0.00	0.00	1	NO	0.13	0.23	438.90	446.30	7.40	ADEQUATE
HANNEL	F-4	A3	442.01	6	0.20	0.23	2.00	0.13	0.011	0.12%	0.00	5.36		4.21	0.10	0	0.00	0.00		NO	0.21	0.21	442.01	446.30	4.29	ADEQUATE
	A3	HB2	444.14	6	0.20	0.23	66.05	0.13	0.011	0.12%	0.08	4.21	0.07	4.21	0.10	0	0.00	0.00		NO	0.16	0.25	445.56	449.70	4.14	ADEQUATE
HANNEL	7.0	1102			5.20	0.25	50.05	0.10	0.011	0.122,0	5.55		0.07		0.10		0.00	0.00			0.20	5.25	1 15150	113170		112207112
HANNEL	SD 5	A2	437.16	10	0.55	1.74	4.07	0.21	0.011	0.45%	0.02	6.59	0.17	5.32	0.15	90	0.70	0.23	NO	NO	0.56	0.58	437.18	440.40	3.22	ADEQUATE
HANNEL	A2	F-2	437.73	10	0.55	1.34	22.16	0.21	0.011	0.27%	0.06	5.32	0.11	3.68	0.07	0	0.00	0.00		NO	0.24	0.30	437.79		5.21	ADEQUATE
HANNEL	F-2	A1	439.72	8	0.35		46.04	0.17	0.011	0.11%	0.05	3.68	0.05	3.68	0.07	0	0.00	0.00	 	NO	0.13	0.18	439.94	443.65	3.71	ADEQUATE
HANNEL	A1	F-1	440.28	<u>-</u> 8	0.35	0.47	22.22	0.17	0.011	0.11%	0.02	3.68	0.05	3.68	0.07	0	0.00	0.00	+	NO	0.16	0.19	440.30	444.00	3.70	ADEQUATE

Hydraulic Gradeline Calculations Methodology H_{fr} , friction loss = $L*S_{fr}$

 H_{str} , structure loss = $H_o + H_i + H_\Delta$ H_{total} , total head loss= $H_{fr} + H_{str}$ H_∆, structure bend loss=K*V_i²/2g

Water surface elevation in bottom structure of pipe run. For the first (most downstream) run of HGL analysis per VDOT standards use the greater of the tailwater elevation (if known) or 80% full depth.

Expansion loss for upper structure of pipe run. If the upstream structure is a wye, the expansion losses are taken as zero.

Velocity of water entering pipe run. If pipe run is at the top of the system, set this to the velocity out of the pipe run. Otherwise, use upstream pipe's velocity. If multiple pipes feed in, use the inlet velocity with the greatest momentum (QxV)

 H_o , structure outlet loss=0.25(0.3 if top pipe)* V_0^2 /2g

Contraction loss for upper structure of pipe run. If the upstream structure is a wye, the expansion losses are taken as zero.

Angle of deflection in the horizontal plane between the upper structure of the pipe run in question and the next upstream pipe. If multiple pipes in, this is the angle of the pipe which creates the most headloss. If no pipes in, set to zero.

Bend loss for upper structure of pipe run. By default this formula uses the listed inlet velocity. However, if multiple pipes feed into this run bend losses must be calculated for all inflowing pipes and the maximum chosen.

V_o,velocity out

If 20%+ of the total flow is coming from a curb/grate inlet, or if there's an inlet pipe with an invert greater than the crown of the outlet pipe, plunging losses apply.

The engineer may specify IS-1 inlet shaping for a structure which allows the inlet head losses to be reduced by 50%.

Structure loss (sum of expansion, contraction, and bend loses) for the upstream structure of the pipe run. Top elevation of upper structure of pipe run.

APPROVAL	DATE	REVISIONS
	03/04/2022	INITIAL SUBMISSION
	08/25/2022	SECOND SUBMISSION
	12/16/2022	THIRD SUBMISSION



H_i, structure inlet loss=0.35*V_i²/2g

NOT FOR CONSTRUCTION **REZONING PLANS** 12/16/2022

DRAINAGE PLAN **CALCULATIONS** DRAWING TITLE

Patrick Horgan PATRICK JOSEPH

Lic. No. 061930

016

Tree List for 11004Park Rd. Fairfax City, VA

Prepared by Bill Becker, ISA Certified Arborist # MA-0216A November 18, 2021 Lot size = 50,788 s. f. Existing canopy = 25,200 s. f. Preserved canopy = 0 s. f.

** denotes written permission to be obtained before removal.

Tree #	Common Name Botanical name	DBH Hgt.	Condition	Life Exp.	Preservation Measures	Canopy Sq. Ft.
1	Black Locust Robinia pseudoacacia	10"	Fair	>10	Remove – within limits of disturbance.	N/A
2	Black Locust Robinia pseudoacacia	10"	Fair	>10	Remove – within limits of disturbance.	N/A
3	Black Locust Robinia pseudoacacia	13"	Fair	>10	Remove – within limits of disturbance.	N/A
4	Tulip Poplar Liriodendron tulipifera	19"	Dead	0	Remove – within limits of disturbance.	N/A
5	Tulip Poplar Liriodendron tulipifera	19"	Dead	0	Remove – within limits of disturbance.	N/A
6	Pin Oak Quercus palustris	40"	Fair	>7	Remove – within limits of disturbance.	N/A
7	Black Locust Robinia pseudoacacia	16"	Dead	0	Remove – within limits of disturbance.	N/A
8	Bradford Pear Pyrus calleryana	16"	Fair	>10	Remove – within limits of disturbance.	N/A
9	Black Locust Robinia pseudoacacia	12"	Poor	<3	Remove – within limits of disturbance.	N/A
10	Wild Cherry Prunus serotina	36"	Poor	<3	Remove – within limits of disturbance.	N/A
11	Tulip Poplar Liriodendron tulipifera	28"	Fair	>10	Remove – within limits of disturbance.	N/A
12	Tulip Poplar Liriodendron tulipifera	24"	Fair	>10	Remove – within limits of disturbance.	N/A
13	White Ash Fraxinus americana	18"	Fair	>10	Remove – within limits of disturbance.	N/A
14	Red Maple Acer rubrum	28"	Fair	>10	Remove – within limits of disturbance.	N/A
15	Tulip Poplar Liriodendron tulipifera	41"	Fair	>10	Remove – within limits of disturbance.	N/A
16	Tulip Poplar Liriodendron tulipifera	24"	Fair	>10	Remove – within limits of disturbance.	N/A
17	Sugar Maple Acer saccharum	4"	Good	>10	Remove – within limits of disturbance.	N/A
18	Sugar Maple Acer saccharum	4"	Good	>10	Remove – within limits of disturbance.	N/A
19	Tulip Poplar Liriodendron tulipifera	25"	Good	>10	Remove – within limits of disturbance.	N/A
20	Pin Oak Quercus palustris	25"	Good	>10	Remove – within limits of disturbance.	N/A
21	Eastern Redcedar Juniperus virginiana	16"	Good	>10	Remove – within limits of disturbance.	N/A

22	Eastern Redcedar Juniperus virginiana	9"	Good	>10	Remove – within limits of disturbance.	N/A
23	Eastern Redcedar Juniperus virginiana	16"	Good	>10	Remove – within limits of disturbance.	N/A
24	Wild Cherry Prunus serotina	4"	Poor	<3	Remove – within limits of disturbance.	N/A
25N	White Pine Pinus strobus	18"	Dead	0	Remove with permission – close to limits of disturbance. **	N/A
26N	Leyland Cypress Cupressocyparis leylandii	6"	Good	>10	Remove with permission – close to limits of disturbance. **	N/A
27N	Leyland Cypress Cupressocyparis leylandii	8"	Good	>10	Remove with permission – close to limits of disturbance. **	N/A
28N	Leyland Cypress Cupressocyparis leylandii	8"	Good	>10	Remove with permission – close to limits of disturbance. **	N/A
29N	Leyland Cypress Cupressocyparis leylandii	8"	Good	>10	Remove with permission – close to limits of disturbance. **	N/A
30N	Leyland Cypress Cupressocyparis leylandii	8"	Good	>10	Remove with permission – close to limits of disturbance. **	N/A
31N	Leyland Cypress Cupressocyparis leylandii	8"	Good	>10	Remove with permission – close to limits of disturbance. **	N/A
32N	Leyland Cypress Cupressocyparis leylandii	8"	Fair	>10	Remove with permission – close to limits of disturbance. **	N/A
33N	Black Locust Robinia pseudoacacia	8"	Fair	>10	Save – install protective fence.	N/A
34N	Black Walnut Juglans Nigra	8"	Fair	>10	Save – install protective fence.	N/A
35N	White Mulberry Morus alba	8"	Fair	>10	Save – install protective fence.	N/A
36N	Black Walnut Juglans Nigra	16"	Fair	>10	Save – install protective fence.	N/A
37N	White Pine Pinus strobus	17"	Fair	>7	Remove with permission – close to limits of disturbance. **	N/A
38N	Black Walnut Juglans nigra	14"	Good	>10	Save – install protective fence.	N/A

Becker Landscaping & Tree Service 10698 Moore Dr. Manassas, Va. 20111 703-330-5204

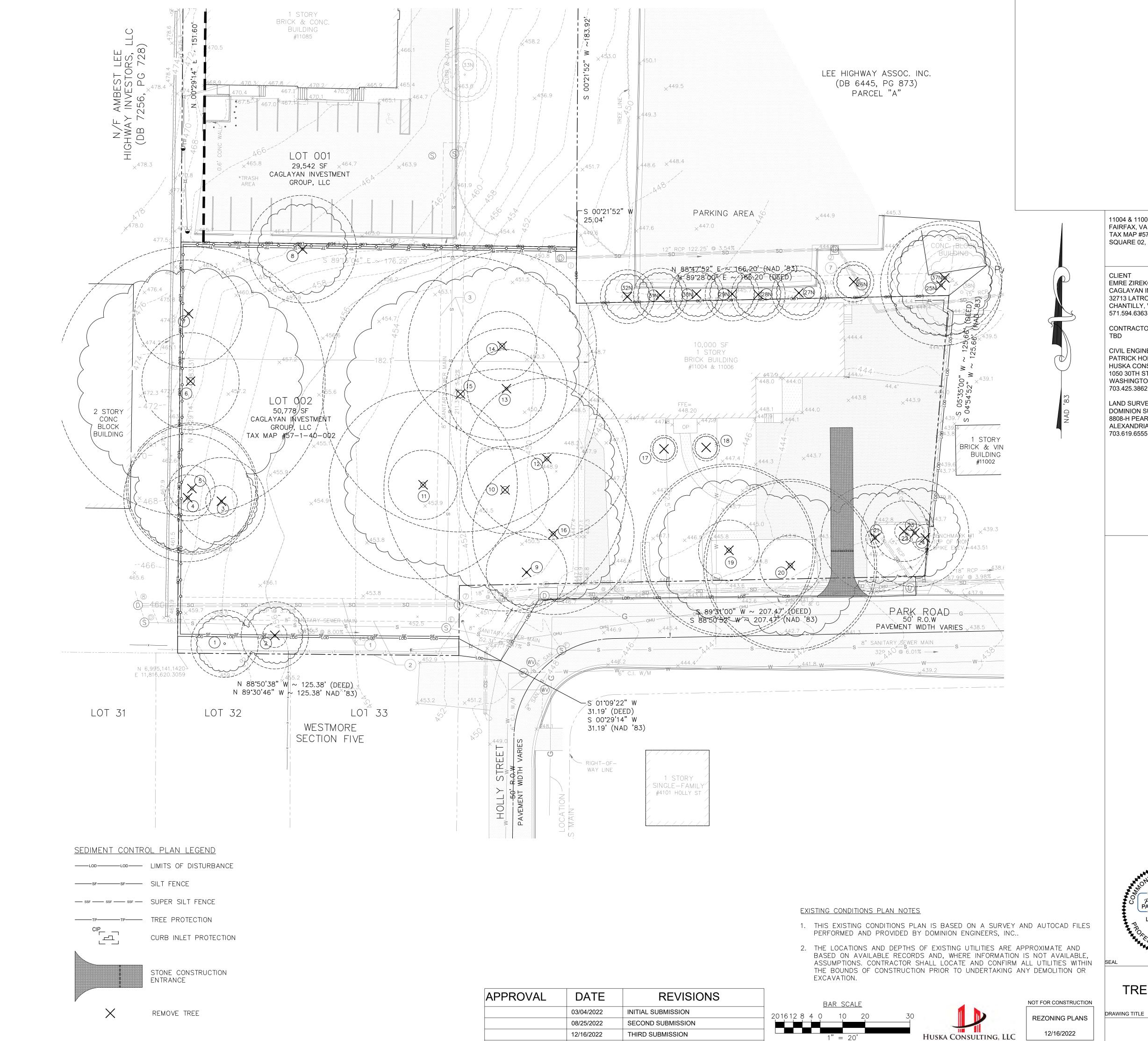
> Narrative of Tree Preservation 11004 Park Rd., Fairfax City, Va.

- All tree preservation activities shall be done according to the Fairfax City Erosion and Sediment Control Manual dated April, 2014 and meet industry standards as specified by the International Society of Arboriculture and the American National Standards Institute. Any treatments or activities specified not meeting these standards will be as specified and approved by the Fairfax City Urban Forester.
- Excavation and demolition shall occur. Prior to excavation super silt fence which will also function as tree protective fence shall be installed. Signs shall be placed every 50' indicating the tree protection areas. No activity, materials or equipment shall go beyond the tree protective fence which shall remain in place until completion of construction.
- The existing trees on the lot are predominately Upland Forest with some Landscaped Tree Canopy in fair to poor condition. Many trees are covered in vines. Several are dead.
- The canopy coverage requirements will be met through the planting of trees. There are no "Heritage", "Specimen", "Memorial" or "Street" trees on this lot or
- neighboring lots. There are no proffered conditions, development plans, conceptual/final development

plans, special permits, special exceptions or variance approvals.

Bill Becker

ISA Certified Arborist # MA – 0216A November 18, 2021



11004 & 11006 PARK RD

FAIRFAX, VA 22306 TAX MAP #57-1-40-002 SQUARE 02, LOT 002

EMRE ZIREKOGLU

32713 LATROBE ST CHANTILLY, VA 20152

571.594.6363

CONTRACTOR

CIVIL ENGINEER PATRICK HORGAN

703.425.3862

703.619.6555

LAND SURVEYOR

HUSKA CONSULTING, LLC

DOMINION SURVEYS, INC.

ALEXANDRIA, VA 22309

8808-H PEAR TREE VILLAGE COURT

Patrick Horgan PATRICK JOSEPH

⁻⁶⁸⁶FACTROSAN ...

Lic. No. 061930

TREE SURVEY

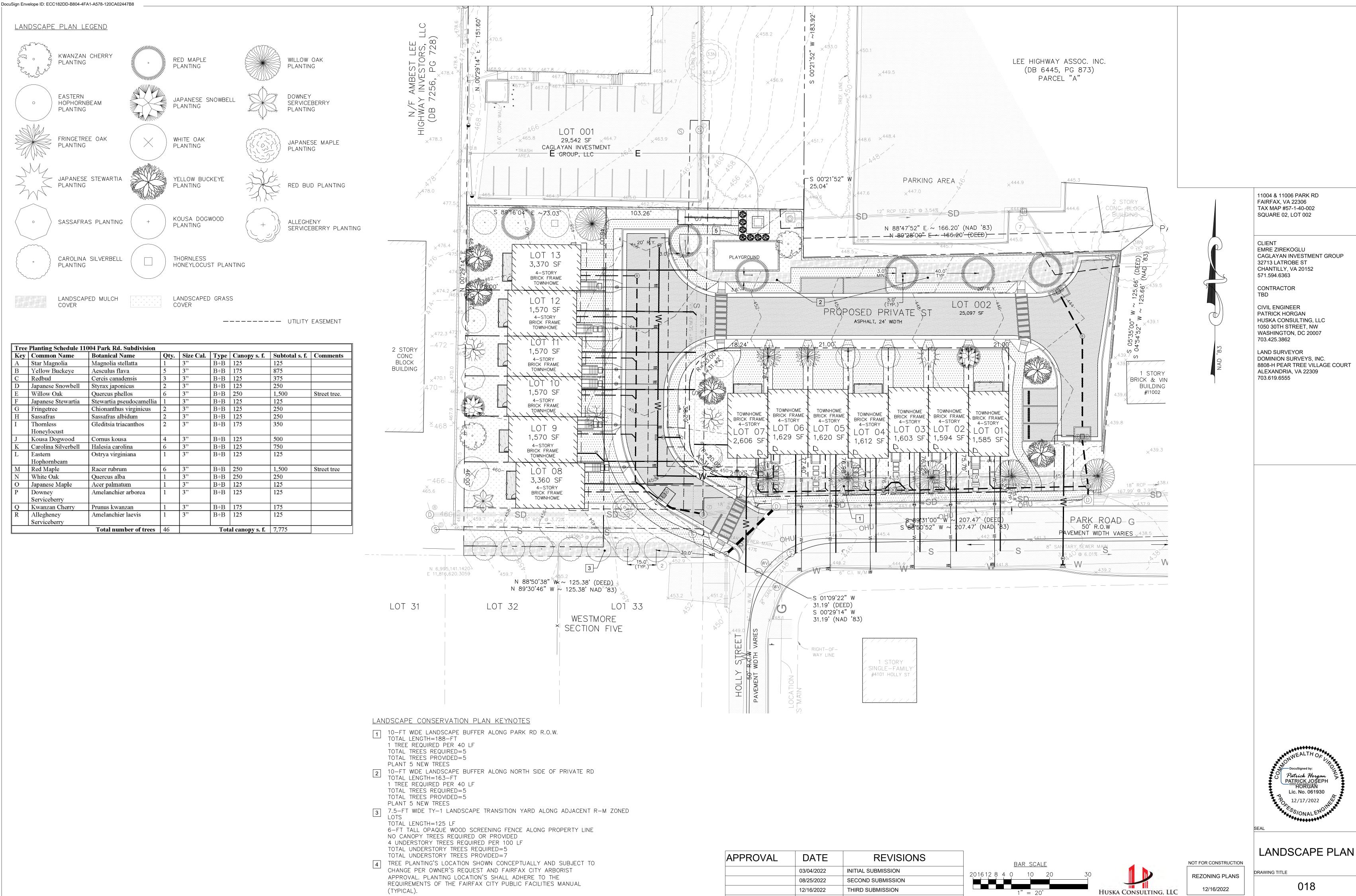
017

DRAWING NO.

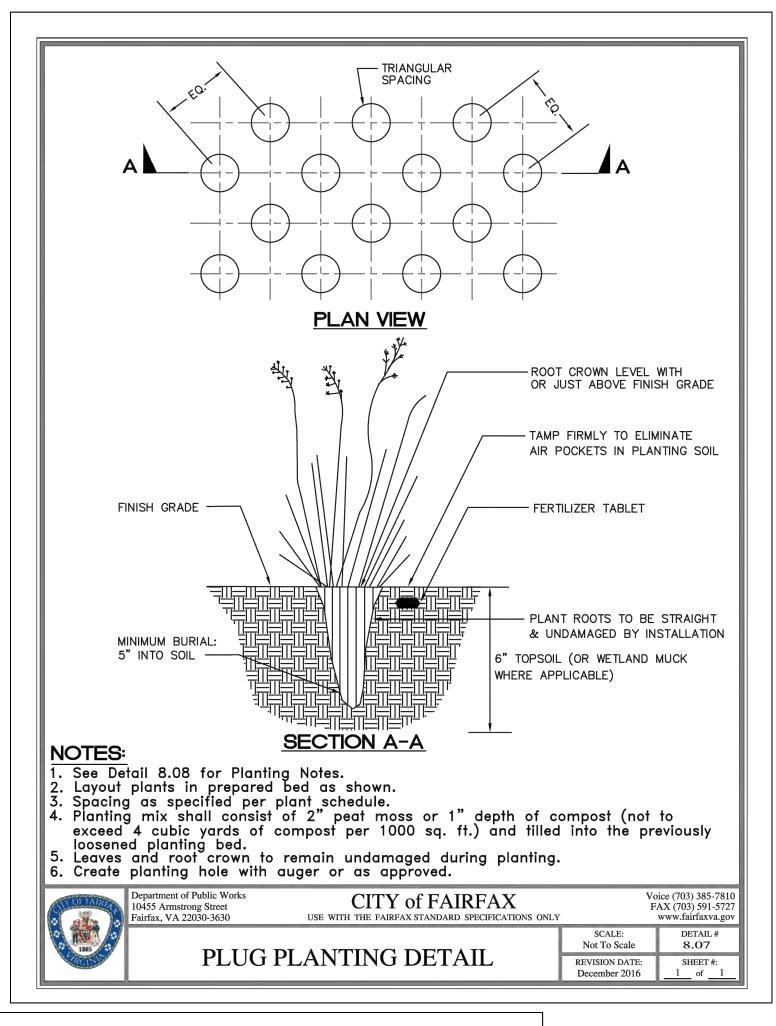
1050 30TH STREET, NW WASHINGTON, DC 20007

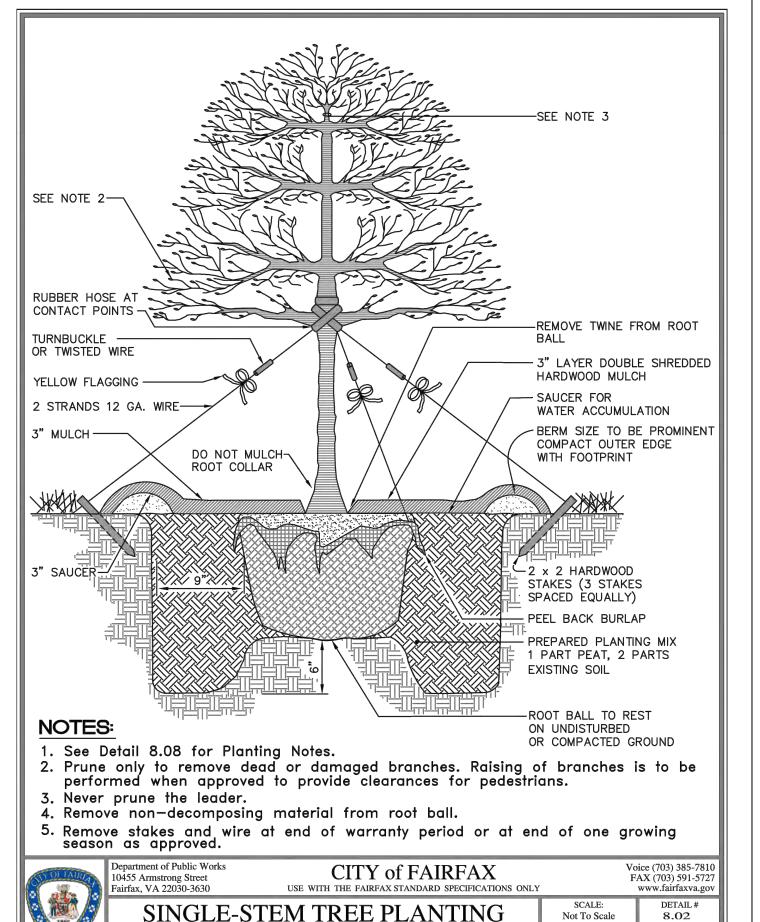
CAGLAYAN INVESTMENT GROUP

CLIENT



018

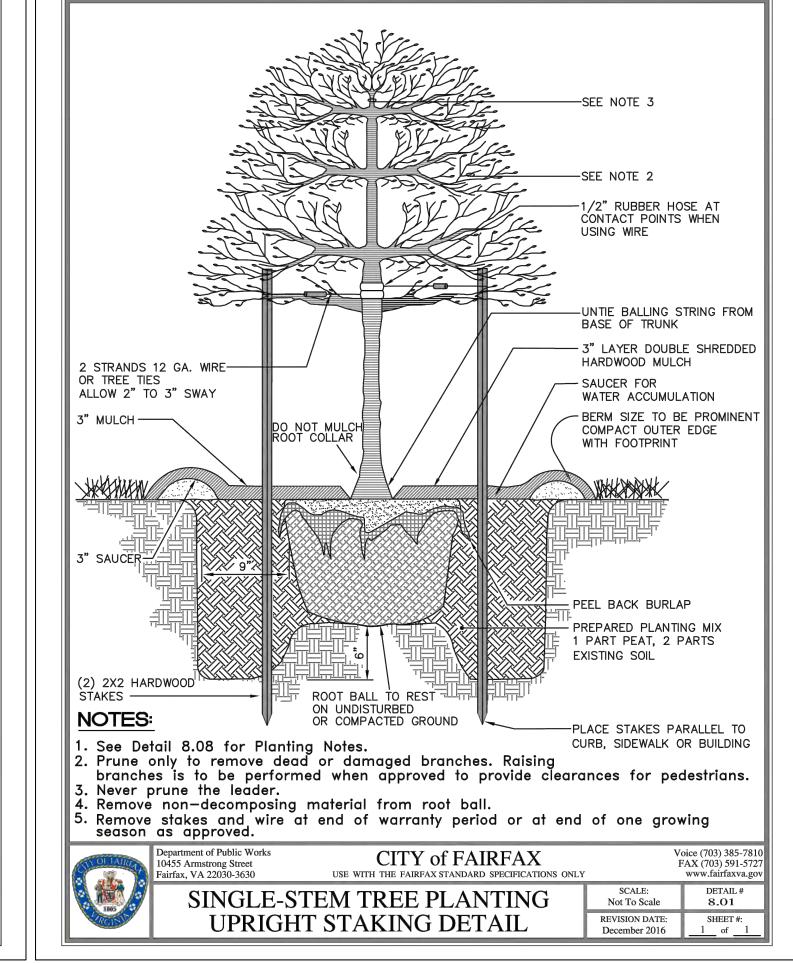


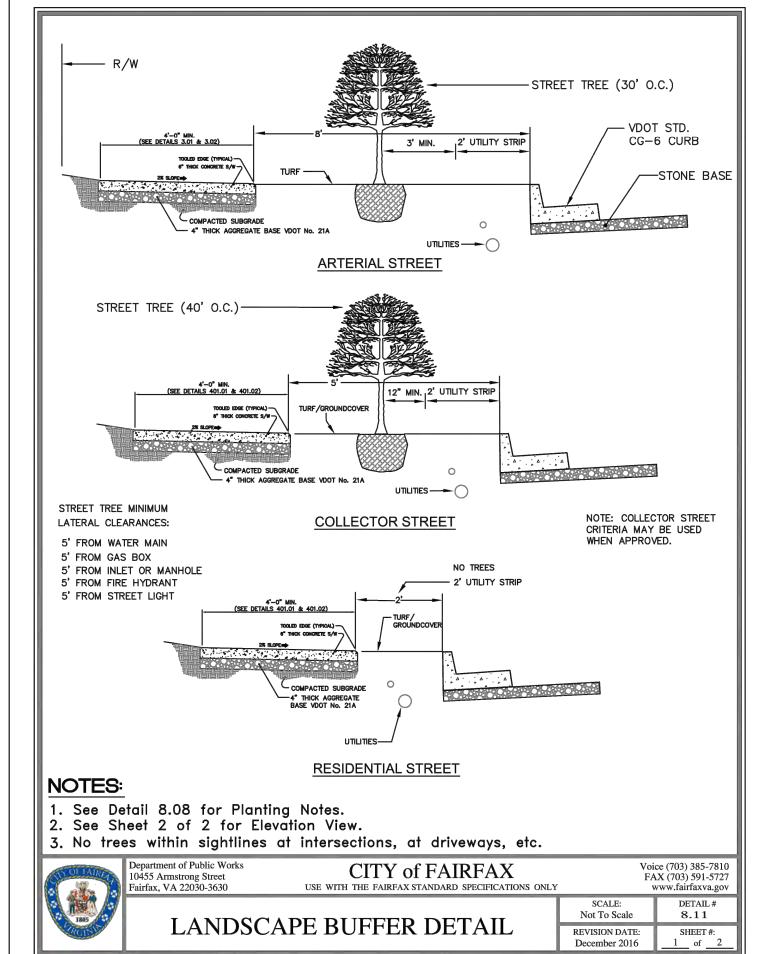


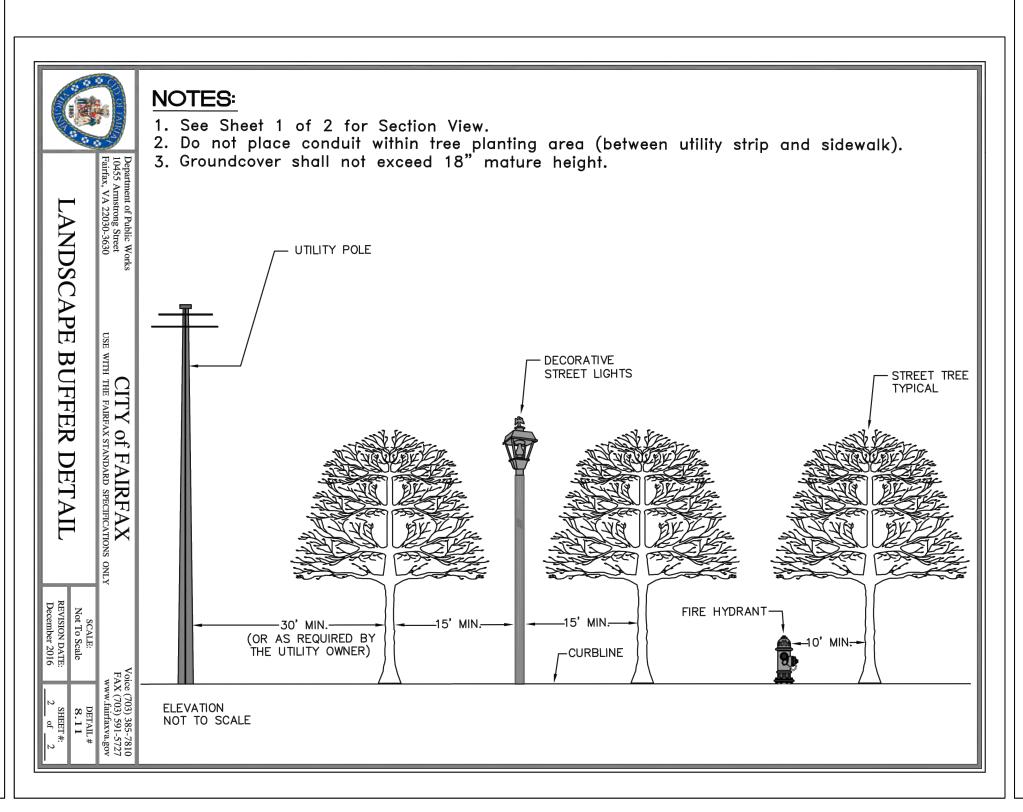
SINGLE-STEM TREE PLANTING

and STAKING DETAIL

REVISION DATE: December 2016







APPROVAL

03/04/2022

08/25/2022

12/16/2022

Ι.	General				
		Landscape Specifications: Landscape specifications shall below. Any item or procedure not mentioned below shal the Landscape specification guidelines published by the L Association (latest edition).	l be as spec	ified in	
	2.	ant Materials: The Landscape Contractor shall furnish and install and/or g, ball, burlap, and transplant plant materials called for on the drawings ad/or listed in the plant schedule. The City reserves the right to inspect ant material at the nursery source, before off—loading at the project site, and in storage or prior to installation.			
	when in storage or prior to installation. 3. Plant Names: Plant names used in the plant schedule shall be identified accordance with Hortus Third, by L.H. Bailey, 1976. 4. Plant Standards: Plant materials shall be equal to or better than the requirements of the "American Standard for Nursery Stock" (ANSI Z60.1 latest edition), as published by the American Association of Nurseryman (hereinafter referred to as AAN standards). Plants shall be typical of the species and variety, shall have a normal habit of growth, and shall be quality, sound, vigorous, well branched, and with healthy, well—furnished systems. They shall be free of disease, insect pests, and mechanical injuries.				
		 (A) Plants shall be nursery grown and shall have been g same climatic conditions as the location of the subje least two years before planting. Neither heeled—in plants or transplanted trees when specified architect may be used, provided that locations and sepermit proper balling. 	ect project for at plant, nor plants from by the landscape		
	5.	 Materials for Planting: (A) Stakes for guying trees shall be sound oak or other approved hardwood. Three stakes spread 120—degrees apart shall be used when detailed. Notch stakes for wire. See details. Trees located between sidewalk and curb shall have two stakes. (B) Tree Guys: Provide wire ties and guys of 2—strand, twisted, pliable galvanized steel wire not lighter than 12—gauge with zinc coated turnbuckles. Provide w—ply garden hose not less than 0.5—inch hose size, cut to lengths to protect tree trunks from damage by wires. Provide 14—gauge wire for trees less than 12—ft. height. Wire for guy may be twisted when a turnbuckle is not specified by the landscape architect. Use of tree ties in lieu of wire is acceptable. (C) Mulching: Mulch shall consist of double shredded hardwood mulch. 			
	6.				
Depositment of Public Works					
OIIY O	FAIR	Department of Public Works 10455 Armstrong Street Fairfax, VA 22030-3630 USE WITH THE FAIRFAX STANDARD SPECIFICATIONS ONLY	F.A.	ce (703) 385-7810 XX (703) 591-5727 www.fairfaxva.gov	
	1805	PLANTING NOTES	SCALE: Not To Scale	DETAIL# 8.08	

DETAIL

REVISIONS

INITIAL SUBMISSION

THIRD SUBMISSION

SECOND SUBMISSION

11004 & 11006 PARK RD FAIRFAX, VA 22306 TAX MAP #57-1-40-002 SQUARE 02, LOT 002

CLIENT EMRE ZIREKOGLU CAGLAYAN INVESTMENT GROUP 32713 LATROBE ST CHANTILLY, VA 20152

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> Patrick Horgan PATRICK JOSEPH 686**FA151R9GAN** Lic. No. 061930

LANDSCAPE **DETAILS**

DRAWING TITLE

HUSKA CONSULTING, LLC

REVISION DATE: December 2016

SHEET #: 1 of 3

NOT FOR CONSTRUCTION REZONING PLANS 12/16/2022