

# THE LAMB CENTER

MULTIMODAL TRANSPORTATION ASSESSMENT

September 27, 2022



## The Lamb Center

# Multimodal Transportation Assessment City of Fairfax, Virginia

September 27, 2022

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# SECTION 1 INTRODUCTION

This report presents the results of a traffic impact study conducted in support of the proposed redevelopment of a site in the City of Fairfax currently developed with the 12-room Hy-Way Motel and presents an evaluation of the existing and future transportation network.

This study was conducted in accordance with a scoping agreement developed with City of Fairfax staff. The study scope was determined with City staff based on a review of key study intersections and roadways that would potentially be affected by the implementation of the proposed redevelopment and the number of new trips expected to be generated.

The subject site is located north of Fairfax Boulevard, east of Campbell Drive and west of Roanoke Street, in the City of Fairfax, Virginia, as shown on Figure 1-1.

The site consists of one (1) land parcel within the City of Fairfax:

Property ID	<u>Address</u>	<u>Acreage</u>		
48-3-09-020	9640 Fairfax Blvd.	.41 acres		

The applicant, The Lamb Center, plans to develop the site with 54 permanent supportive housing units and 1,400 SF of office space. The site plan is shown on Figure 1-2.

According to the 24VAC30-155 ("Chapter 870") regulations, all development proposals which meet certain specific trip generation thresholds are subject to the regulations as outlined in the Virginia Department of Transportation's (VDOT) Traffic Impact Analysis Regulations Administrative Guidelines ("Administrative Guidelines"). In January 2012, an amendment to the Administrative Guidelines took effect, which determined a development proposal is considered to substantially impact the transportation network if it generates 5,000 or more net new daily vehicle trips located on, or within 3,000 feet of, a VDOT maintained roadway. Based on the trips anticipated to be generated by the subject development, the development would not require a VDOT Chapter 870 compliant traffic study.

Although a traffic impact analysis is not required per 24VAC30-155, the City of Fairfax has requested the submission of a traffic study in conjunction with this development application.

This traffic study was completed in accordance with the City of Fairfax policies and guidelines and is intended to address the following issues:

- 1. Estimation of the net new vehicle trip ends generated by the planned land uses during the AM and PM commuter peak hours and during the PM school peak hour.
- 2. Determination of the effects of the proposed development on the surrounding local roadway network.

3. Identification of potential road and/or operational improvements necessary to accommodate the project.

Based on the traffic study scoping form provided in Appendix A, tasks undertaken to prepare this study included the following:

- 1. Reviewed the applicant's conceptual plans for the subject site.
- Field reviewed the subject site in order to determine existing roadway and intersection geometrics and traffic controls, access opportunities and/or constraints, and general traffic conditions.
- 3. Conducted peak hour turning movement counts obtained at the following study intersection:
  - Fairfax Boulevard/Campbell Drive
  - Fairfax Boulevard/Fairfax Circle (West)
  - Fairfax Boulevard/Fairfax Circle (East)
- 4. Calculated existing AM and PM commuter peak hour intersection levels of service at the study intersections.
- 5. Identified the number of net new peak hour trips that would be generated by the proposed mixed-use development less trips currently generated by the existing land uses based on standard Institute of Transportation Engineers (ITE) <u>Trip Generation Manual</u>, 11th Edition equations and weighted average rates.
- 6. Determined future background traffic forecasts based on regional traffic growth and estimates of traffic that would be generated by other approved/planned developments in the site vicinity.
- 7. Calculated future levels of service with and without the proposed development at the key study intersections for a proposed build-out year of 2026.

Sources of data for this analysis include traffic counts conducted by Wells + Associates Inc., information obtained from the City of Fairfax, the Institute of Transportation Engineers (ITE), VDOT, the Highway Capacity Manual 2000 (Synchro software, version 10), The Lamb Center and the files and library of Wells + Associates.

#### Conclusions

Based on the results of this traffic impact study, the following may be concluded:

 The Fairfax Boulevard/Fairfax Circle signalized intersections currently operate at an overall LOS "C" or better during the AM and PM commuter peak periods based on Highway Capacity Manual calculations. The approaches at the unsignalized intersection of Fairfax Boulevard/Campbell Drive currently operate at LOS "C" or better during the peak periods. Estimated queues would generally be accommodated within the available storage areas. The eastbound queue towards the intersection of Fairfax Boulevard would potentially extend back to Campbell Avenue during the peak periods. Commuters would be required to utilize gaps in traffic recorded by field observations to occur several times per cycle length.

- 2. A review of VDOT AADT volumes along Fairfax Boulevard in the vicinity of the site indicates a reduction in traffic volumes from 2016 to 2019. AADT volumes along Fairfax Boulevard east of Draper Drive fell from 42,000 vehicles in 2016 to 35,000 vehicles in 2019.
- 3. The approved pipeline development Wawa site at 9700 Fairfax Boulevard is anticipated to generate a net new 399 AM peak hour trips and 327 PM peak hour trips at full buildout.
- 4. Under future 2026 traffic conditions, minimal increases in delay at the study intersections are expected due to the trips generated by approved pipeline developments in the vicinity of the site and overall levels of service would remain generally consistent with existing conditions with the exception of the northbound approach at the intersection of Fairfax Boulevard/Fairfax Circle (East) which would degrade from LOS "D" to LOS "E" during the AM peak hour.
- 5. The site is currently developed with the 12-room Motel. The Applicant proposes to redevelop the site with 54 residential units and up to 1,400 SF of office.
- 6. The project is estimated to generate 23 AM peak hour trips and 29 PM peak commuter hour trips upon buildout.
- 7. Under future 2026 traffic conditions, with the development of the subject site, intersection levels of service would remain generally consistent with existing and background conditions. The analyses show that the Fairfax Boulevard signalized intersections will continue to operate at LOS "C" or better during the AM and PM peak periods. The site is estimated to have a minimal impact on network queueing. Commuters would continue to be able to utilize gaps in traffic on Fairfax Boulevard during the peak periods.
- 8. All unsignalized intersection and access drive approaches will operate at LOS "D" or better during each of the studied peak periods.
- 9. Access to the site and parking garage will be via one (1) full access driveway along Campbell Drive.
- 10. Access to the existing Motel is currently provided at two locations along Campbell Drive and one right-in/right-out driveway along Fairfax Boulevard. The Applicant intends to consolidate these access drives to a single location providing enhanced access management.



Figure 1-1
Site Location

Study Intersection



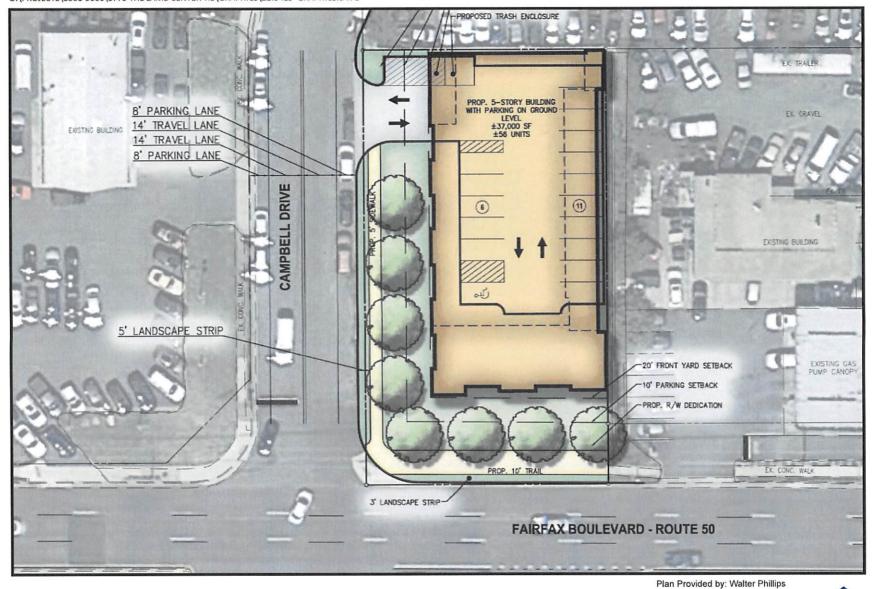


Figure 1-2 Site Plan

NORTH

The Lamb Center
City of Fairfax, Virginia

# SECTION 2 BACKGROUND INFORMATION

#### **Location and Surrounding Uses**

As shown in Figure 1-1, the site is located along Campbell Drive and Fairfax Boulevard in the City of Fairfax. Regional Access is provided by I-66 via Blake Lane, Old Lee Highway, and Lee Highway. Fairfax Boulevard/Arlington Boulevard provides access to/from I-495 (the Capital Beltway).

Properties predominantly along Fairfax Boulevard are generally commercial in nature while to the north and south of Fairfax Boulevard is mostly residential.

#### **Comprehensive Plan Land Use Recommendations**

The City's Comprehensive Plan shows the subject parcel as Commercial Corridor on the Future Land Use Map as shown on Figure 2-1.

#### **Existing Transportation Network**

<u>Existing Road Network</u>. The following are descriptions of the roadways in the vicinity of the proposed development.

<u>Fairfax Boulevard</u> is classified as an arterial roadway according to the City of Fairfax Comprehensive Plan. Within the vicinity of the subject site, Fairfax Boulevard is constructed as a six-lane, median divided roadway and a posted speed limit of 35 miles per hour. Traffic signals are provided at major cross-streets including Route 29. Based on 2019 VDOT average annual daily traffic (AADT) data, Fairfax Boulevard carries approximately 35,000 vehicles per day (vpd). This roadway currently provides access to the Hy-Way Motel via one driveway.

<u>Blake Lane</u> is classified by the Comprehensive Plan as an arterial roadway and is constructed as a four-lane, median-divided roadway with a posted speed limit of 35 miles per hour.

<u>Lee Highway (Route 29)</u> is classified by the Comprehensive Plan as an arterial roadway and is constructed as a four-lane, median-divided roadway with a posted speed limit of 35 miles per hour. Based on 2019 VDOT AADT data, Lee Highway carries approximately 35,000 vpd.

<u>Old Lee Highway</u> is classified by the Comprehensive Plan as an arterial roadway and is constructed as a four-lane, median-divided roadway with a posted speed limit of 30 miles per hour. Based on 2019 VDOT AADT data, Lee Highway carries approximately 15,000 vpd.

<u>Campbell Drive</u> is a two-lane north-south roadway. Campbell Drive currently provides access to the existing motel via two existing driveways and will continue to provide direct access to general site traffic for the proposed development via one (1) central driveway.

Existing lane use and traffic control at each of the study intersections is shown on Figure 2-2.

Public Transit Service. The site is served by the City of Fairfax's City-University Energysaver (CUE) Bus "Green Route", as shown on Figure 2-3, which provides service between the GMU campus, Old Town Fairfax, and the Vienna/Fairfax-GMU metrorail station via University Drive, Chain Bridge Road, Eaton Place, Fairfax Boulevard, Fairfax Circle, Arlington Boulevard, Nutley Street, Virginia Center Boulevard, Old Pickett Road, Pickett Road, Main Street, North Street, and George Mason Boulevard. Approximately .20 miles west of the site the City of Fairfax's City-University Energysaver (CUE) Bus "Gold Route" stop is located at the intersection of Fairfax Boulevard & Draper Drive, which provides service between the GMU campus, Old Town Fairfax, and the Vienna/Fairfax-GMU metrorail station. West of the site at the intersection of Spring Street and Fairfax Drive, there is a bus stop that provides access to the WMATA 1C bus route which runs between the Fair Oaks Mall and the Dunn Loring Metro Station.

<u>Pedestrian Facilities</u>. Concrete sidewalks are provided along both sides of Fairfax Boulevard and only on the west side of Campbell Drive. Marked crosswalks are provided across all legs of the Fairfax Boulevard/Fairfax Circle signalized intersections.

#### **Future Transportation Network**

The City of Fairfax's Comprehensive Plan provides recommended strategies for the improvement of the City's transportation network. In general, the Plan recommends strategies that will improve the operation and safety of the City's transportation system in order to achieve the larger community objectives for a vital, vibrant, and livable City. The Multimodal Transportation section of the Comprehensive plan lists a total of four (4) long-term goals for the City of Fairfax. These goals include, connecting with the region, providing viable and attractive mobility choices, integrating transportation with land use, and adopting policies and procedures for strategic transportation decision making. The plan envisions for improved safety for all users at Fairfax Circle which is located east of the site.

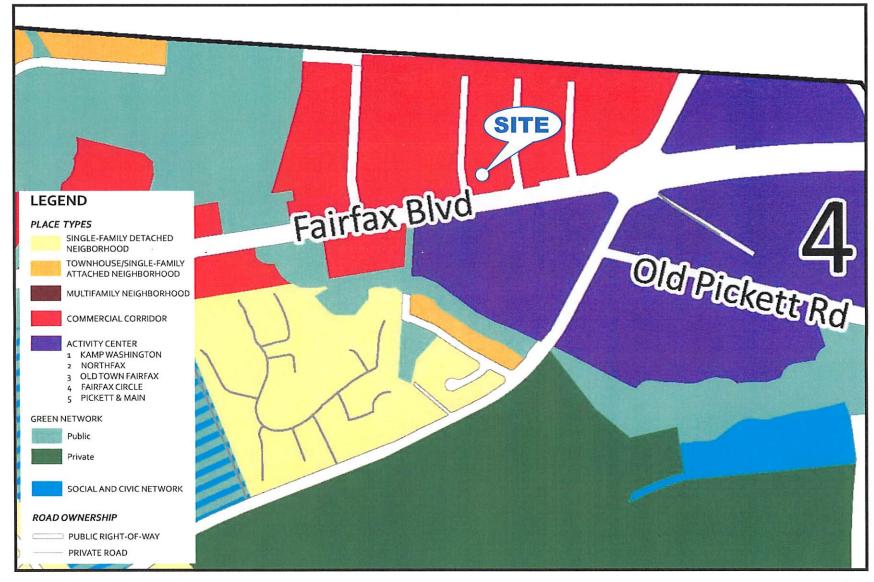


Figure 2-1 Land Use Map





Represents One Travel Lane Stop Sign Signalized Intersection Study Intersection

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**Existing Lane Use and Traffic Control** Figure 2-2

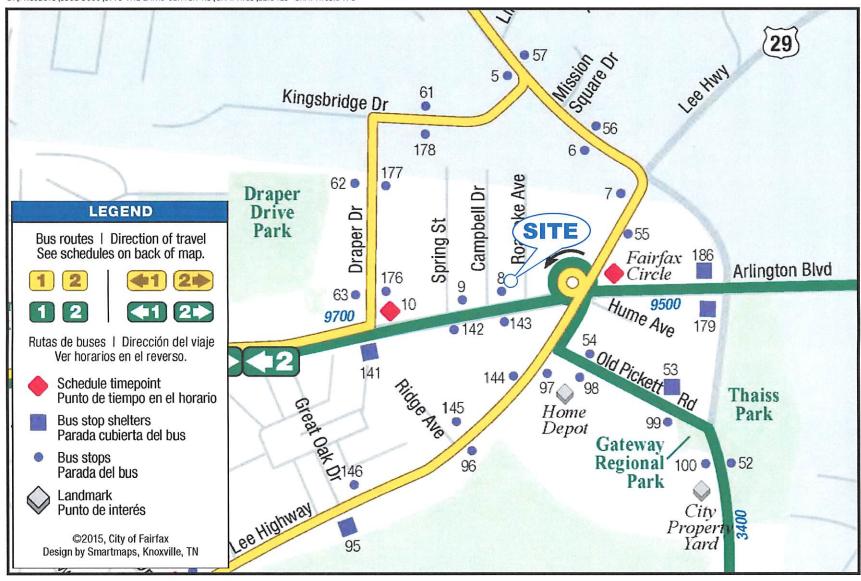


Figure 2-3
Transit Routes



# SECTION 3 STUDY SCOPE AND ANALYSIS PARAMETERS

#### Overview

The subject site is located north of Fairfax Boulevard, east of Campbell Street, and west of Roanoke Street in the City of Fairfax, Virginia. The subject property is comprised of one parcel totaling .41 acres. The parcel with the existing Motel is zoned CR (Commercial Retail).

The primary objective of this study is to assess the impacts of the proposed development plan on the surrounding street system.

This traffic study was conducted in accordance with the scoping document and discussions with Wells + Associates, City staff, and the Applicant. A traffic study scoping meeting was held on June 6, 2022, and resulted in a scoping form dated June 16, 2022 that is provided in Appendix A. As previously noted, the revised development plan includes 54 permanent supportive housing units and up to 1,400 SF of office space. Additionally, the proposed site access is located along Campbell Drive and connects to the sites parking.

#### **Study Area**

The study area was determined based on the intersections and roadways that potentially would be affected by implementation of the proposed development plan. The following intersections were selected for analysis and evaluation:

- Fairfax Boulevard/Campbell Drive
- Fairfax Boulevard/Fairfax Circle (West)
- Fairfax Boulevard/Fairfax Circle (East)

The intersections within the study area were analyzed under AM and PM commuter peak hour conditions.

#### **Site Development Program**

The Applicant is proposing to redevelop the property with 54 permanent supportive housing residential units as well as office space up to 1,400 SF.

#### **Existing Traffic Volumes**

Existing AM and PM commuter peak hour turning movements and pedestrian counts were conducted on Tuesday, April 12, 2022, at the study intersections from 6:00 AM to 9:00 AM and from 4:00 PM to 7:00 PM. Counts were also taken from the Wawa – 9700 Fairfax Boulevard Traffic Impact Study, dated June 26<sup>th</sup>, 2019. Existing AM and PM commuter peak hour turning movements and pedestrian counts were conducted on May 2, 2019, at the study intersections of Fairfax Boulevard/Fairfax Circle from 6:30 AM to 9:30 AM and from 4:00 PM to 7:00 PM. An annual growth rate of 0.5% per year was applied to the 2019 existing volumes, to reach a baseline 2022 existing traffic volume count. The 2019 counts were compared to the 2022 counts to show that little to no impacts were observed due to the pandemic. Based on this, the baseline data and future forecasts including regional growth are considered conservative

The existing vehicular traffic volumes as described above are provided on Figure 3-1. All existing count data is included in Appendix B.



Figure 3-1
Existing Peak Hour Traffic Volumes

Study Intersection
Signalized Intersection
Represents One Travel Lane
Stop Sign

NORTH
The Lamb Center
City of Fairfax, Virginia

# SECTION 4 EXISTING CONDITIONS ANALYSIS

#### **Existing Intersection Levels of Service**

Peak hour levels of service were calculated for the study intersections based on the existing lane use and traffic controls shown on Figure 2-1, the existing traffic volumes shown on Figure 3-1, and the 2000 <u>Highway Capacity Manual</u> (HCM) analysis procedures for signalized and unsignalized intersections. The results are presented in Appendix C and summarized on Table 4-1.

The analyses shows that the signalized intersections at Fairfax Boulevard/Fairfax Circle currently operate at level of service "C" (LOS "C") or better during the AM and PM peak commuter periods. The unsignalized intersection of Fairfax Boulevard/Campbell Drive approaches operate at LOS "C" or better during the AM and PM peak hours.

#### **Existing Queue Analysis**

Existing peak hour queues for study intersection were determined using the 50<sup>th</sup> and 95th percentile queues estimated by Synchro. The 50<sup>th</sup> and 95<sup>th</sup> percentile queues of existing conditions are used to establish a datum against which to compare future conditions. The 50<sup>th</sup> percentile (or average) queue is defined as the maximum back of queue associated with a typical signal cycle. The 95<sup>th</sup> percentile queue is defined as the maximum back of queue with 95<sup>th</sup> percentile traffic volumes. The 95<sup>th</sup> percentile queue is not necessarily ever observed, it is simply based on statistical calculations. The results are presented in Appendix C and summarized in Table 4-2.

The analyses show all 95<sup>th</sup> percentile queues are adequately accommodated within the available turn-lane storage lanes. However, the eastbound through at the intersection of Fairfax Boulevard/Fairfax Circle (West) shows a queue of approximately 615 feet during the AM peak hour, this would extend back to block the southbound left turn at the intersection of Fairfax Boulevard/Campbell Drive. Commuters will have to utilize gaps in traffic to perform the southbound left turn onto Fairfax Boulevard. Field observations indicate that gaps in traffic are become several times throughout each adjacent cycle length of 220 seconds.

Table 4-1 The Lamb Center Existing Conditions Intersection Level of Service Summary  $^{1,\,2}$ 

	Existing Conditions								
Approach/ Lane Group	AM Pe	eak Hour	PM Peak Hour						
	LOS Delay (s)		LOS	Delay (s)					
1. Fairfax Boulevard/Campbe	ell Drive - Unsignali	zed							
EB Approach	А	0.1	Α	0.1					
WB Approach	Α	0.2	Α	0.3					
NB Approach	С	24.4	С	17.9					
SB Approach	С	20.4	В	14.2					
2. Fairfax Boulevard/Fairfax	Circle (West) - Signa	alized							
EB Approach	В	16.5	В	10.7					
WB Approach	Α	3.7	Α	3.5					
SB Approach	D	36.2	С	29.4					
Overall	В	18.0	В	15.6					
3. Fairfax Boulevard/Fairfax	Circle (East) - Signa	lized							
EB Approach	Α	2.6	Α	2.9					
WB Approach	В	11.5	Α	6.6					
NB Approach	D	54.9	В	17.6					
Overall	С	21.7	Α	8.0					

## Note(s):

- 1. Capacity analysis based on Highway Capacity Manual methodology, using Synchro 10.
- 2. Highting denotes that the Level of Service is at or beyond capacity.

Table 4-2 The Lamb Center

Existing Conditions Intersection Queuing Summary 1, 2, 3, 4

	Storage		Existing C	Existing Conditions	
Approacn/ Lane Groun	Length	AM Peak Hour	ak Hour	PM Peak Hour	ık Hour
2	( <del>L</del> )	50th Pecentile	95th Pecentile	50th Pecentile	95th Pecentile
1. Fairfax Boulevard/Ca	ampbell Dr	d/Campbell Drive - Unsignalized			
EBL	100	1	1	п	1
EBTR	į	,	0	ı	0
WBL	80	ı	3	Ĩ	5
WBTR	ı	ť	0	.1	0
NBLTR	ı	1	1	1	12
SBLTR	1	,	10	ī	7
2. Fairfax Boulevard/Fa	airfax Circle	d/Fairfax Circle (West) - Signalized			
EBT	E	484	615	190	213
EBR	1	169	256	92	122
WBT	31	47	53	65	99
SBT	1	244	285	330	#412
SBR	ı	0	0	0	0
3. Fairfax Boulevard/Fa	airfax Circle	d/Fairfax Circle (East) - Signalized			
EBT	1	40	45	30	m32
WBT	ĩ	153	181	142	m211
WBR	200	51	77	51	m76
NBT		541	591	126	m142
NBR	ī	0	0	0	0m

Note(s):

1.  $\sim$  Volume exceeds capacity, queue is theoretically infinite.

2. #95th percentile volume exceeds capacity, queue may be longer.

3. m Volume for 95th percentile queue is metered by upstream signal.

4. Highting denotes that the 95th percentile queue exceeds the available storage.

# SECTION 5 ANALYSIS OF FUTURE CONDITIONS WITHOUT SITE DEVELOPMENT

#### Overview

Forecasts for traffic conditions <u>without the redevelopment of The Lamb Center</u> were estimated at the study intersection based on a composite of existing traffic regional traffic growth as described below. Future levels of service under these forecasted conditions were evaluated at the study intersections.

### **Regional Traffic Growth**

A review of VDOT AADT volumes along Fairfax Boulevard in the vicinity of the site indicates a reduction in traffic volumes from 2016 to 2019. AADT volumes along Fairfax Boulevard east of Draper Drive fell from 42,000 vehicles in 2016 to 35,000 vehicles in 2019.

In order to be conservative, existing traffic volumes were increased by 0.5% per year to the anticipated build-out of the site in 2026 as shown on Figure 5-1.

### **Traffic from Other Approved/Pending Developments**

At the request of City staff, the following approved/pending developments were included as approved (i.e., "pipeline") developments:

- Wawa –9700 Fairfax Boulevard
  - 6,049 SF Grocery Store
  - 12 Fueling Stations

As shown in Table 5-1, these pipeline developments are anticipated to generate 135 AM peak primary hour trips and 97 PM primary peak hour trips at full buildout. The pipeline development trips are shown on Figure 5-2. The proposed pass-by trips are already on the traffic network and would not impact the subject study intersections.

#### **Background Traffic Forecasts**

The existing traffic volumes depicted on Figure 3-1, regional traffic growth shown on Figure 5-1, and pipeline development trips shown on Figure 5-2 were added together to yield the background future traffic forecasts at the study intersection, shown on Figure 5-3.

Table 5-1
The Lamb Center
Pipeline Trip Generation <sup>1</sup>

Land Use	ITE Land Use Code	Size	Units/SF	<u>AN</u> In	<u>1 Peak Hou</u> Out	<u>r</u> Total	<u>PN</u> In	<u>/I Peak Hour</u> Out	Total	Weekday ADT
Existing Land Use Motel	320	55	Rooms	8	13	21	11	10	21	170
I Woter	320	33	Rooms	Ü	13			10		1,0
Wawa - 9700 Fairfax Boulevard										
Super Convenience Market/Gas Station	960	6,049	SF	252	251	503	210	209	419	5,067
Super Convenience Market/Gas Station	960	12	Fueling Positions	169	168	337	138	138	276	2,766
Average of trips based on square foota		210	210	420	174	174	348	3,917		
Pass-by Trips (63% AM/66%		-132	-132	-264	-115	-115	-230	-2,468		
Primary Trips (Total minus Pass-by)					78	156	59	59	118	1,449
Net New Trips (Proposed Land Use minus Existing Land Use)										
Net New Total Tr	ips			202	197	399	163	164	327	3,747
Net New Primary	trips			70	65	135	48	49	97	1,279

### **Background Future Levels of Service**

Peak hour levels of service were calculated for the study intersections based on the existing lane use and traffic controls, background future traffic forecasts, and the 2000 <u>Highway Capacity Manual</u> (HCM) analysis procedures for signalized and unsignalized intersections. The results are provided in Appendix D, shown on Figure 5-3, and summarized in Table 5-2.

The analyses show that the signalized intersections at Fairfax Boulevard/Fairfax Circle will continue to operate at level of service "C" (LOS "C") or better during the AM and PM peak commuter periods. The approaches to the signalized intersections will continue to operate at LOS "C" or better with the exception of the northbound approach at the intersection of Fairfax Boulevard/Fairfax Circle (East) which will degrade from an LOS "D" to an LOS "E" during the AM peak hour. All approaches at the unsignalized intersection of Fairfax Boulevard/Campbell Drive will continue to operate at LOS "C" or better during each of the peak periods.

#### **Background Future Queue Analysis**

The results are presented in Appendix D and summarized in Table 5-3. With the addition of the pipeline development and regional growth would result in nominal increases (two (2) vehicles or less) in the estimated 50th and 95th percentile queues. Peak hour queues would continue to be adequately accommodated, within the available turn lane storages. The eastbound through at the intersection of Fairfax Boulevard/Fairfax Circle (West) would continue to block the southbound left movement at the intersection of Fairfax Boulevard/Fairfax Circle (West) during the AM peak hour.

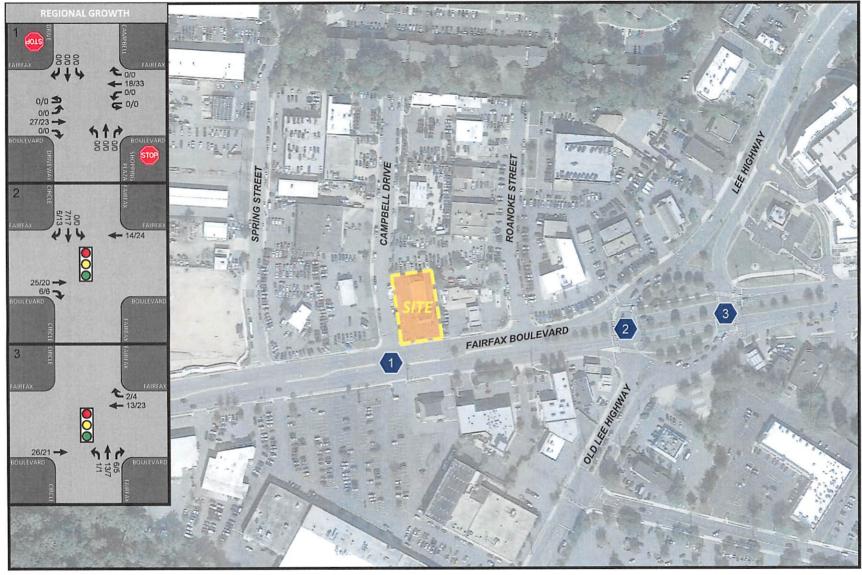


Figure 5-1 Regional Growth (2022-2026)

Study Intersection
Signalized Intersection
Represents One Travel Lane
Stop Sign

NORTH
The Lamb Center
City of Fairfax, Virginia



MORTH The Lamb Center City of Fairfax, Virginia

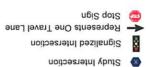


Figure 5-2 Pipeline Development Trips (Wawa - 9700 Fairfax Blvd)



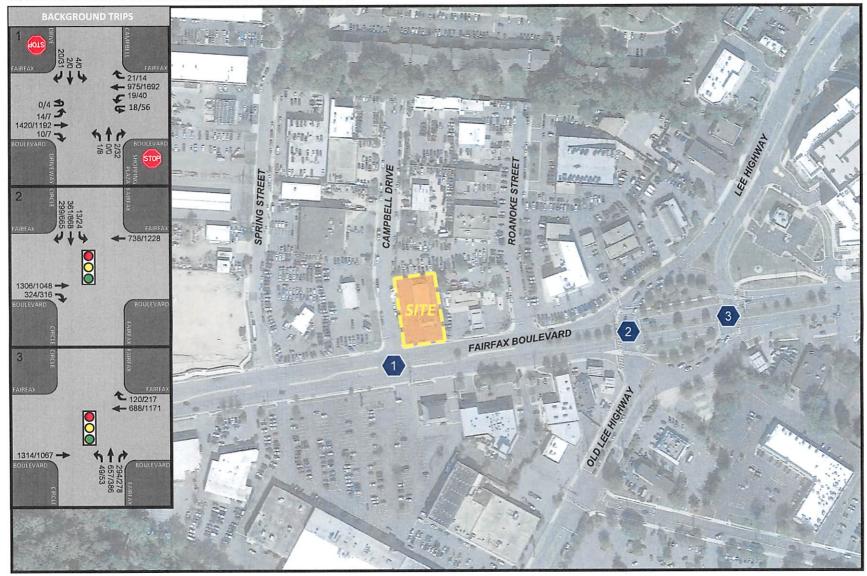


Figure 5-3
Future Conditions without Development
Peak Hour Traffic Volumes

Study Intersection
Signalized Intersection
Represents One Travel Lane
Stop Sign

NORTH
The Lamb Center
City of Fairfax, Virginia

Table 5-2
The Lamb Center
Future Conditions without Development Intersection Level of Service Summary <sup>1, 2</sup>

		Existing Co	nditions		Future Conditions without Development (2026)					
Approach/ Lane Group	AM Pe	eak Hour	PM Peak Hour		AM P	eak Hour	PM Pe	eak Hour		
<b>亚洲的亚洲</b>	LOS	Delay (s)	LOS	Delay (s)	LOS	Delay (s)	LOS	Delay (s)		
1. Fairfax Boulevard/Campbell Drive - Unsignalized										
EB Approach	Α	0.1	Α	0.1	Α	0.1	Α	0.1		
WB Approach	Α	0.2	Α	0.3	Α	0.2	Α	0.3		
NB Approach	С	24.4	С	17.9	C	23.7	С	18.8		
SB Approach	С	20.4	В	14.2	С	20.0	В	14.5		
2. Fairfax Boulevard/Fairfax	x Circle (West	) - Signalized								
EB Approach	В	16.5	В	10.7	В	17.8	В	10.9		
WB Approach	Α	3.7	Α	3.5	Α	4.0	Α	3.7		
SB Approach	D	36.2	С	29.4	С	34.8	С	31.2		
Overall	В	18.0	В	15.6	В	18.4	В	16.3		
3. Fairfax Boulevard/Fairfa	x Circle (East)	- Signalized								
EB Approach	Α	2.6	Α	2.9	Α	2.5	Α	2.9		
WB Approach	В	11.5	Α	6.6	В	12.2	Α	6.7		
NB Approach	D	54.9	В	17.6	E	55.3	В	17.9		
Overall	С	21.7	Α	8.0	С	22.0	Α	7.9		

#### Note(s):

<sup>1.</sup> Capacity analysis based on Highway Capacity Manual methodology, using Synchro 10.

<sup>2.</sup> Highting denotes that the Level of Service is at or beyond capacity.

Table 5-3
The Lamb Center
Future Conditions without Development Intersection Queuing Summary 1, 2, 3, 4

	Storage		Existing C	onditions		Future Conditions without Development (2026					
Approach/ Lane	Length	AM Peak Hour 50th 95th		PM Peak Hour		AM Pea	ak Hour	PM Peak Hour			
Group	(ft)			50th	95th	50th	95th	50th	95th		
		Pecentile	Pecentile	Pecentile	Pecentile	Pecentile	Pecentile	Pecentile	Pecentile		
1. Fairfax Boulevard/C	ampbell Di	rive - Unsignali	zed								
EBL	100	-	1	=:	1	-	2	-	1		
EBTR	-	-	0	-	0	% <u>=</u>	0	-	0		
WBL	80	-	3	-	5	-	3	-	6		
WBTR	-	-	0	-	0	-	0	-	0		
NBLTR	-	-	1	-	12	.=	1		12		
SBLTR	-	-	10	-	7	7.5	9	-	7		
2. Fairfax Boulevard/Fairfax Circle (West) - Signalized											
EBT	-	484	615	190	213	527	665	199	223		
EBR	-	169	256	92	122	194	290	100	132		
WBT	-	47	53	65	66	53	60	67	68		
SBT	-	244	285	330	#412	245	284	338	#441		
SBR	-	0	0	0	0	0	0	0	0		
3. Fairfax Boulevard/Fairfax Circle (East) - Signalized											
EBT	-	40	45	30	m32	34	37	30	m31		
WBT	-	153	181	142	m211	165	195	146	, m220		
WBR	200	51	77	51	m76	53	79	52	m77		
NBT	-	541	591	126	m142	561	612	124	m147		
NBR	-	0	0	0	m0	0	0	0	m0		

### Note(s):

- 1. ~ Volume exceeds capacity, queue is theoretically infinite.
- 2. # 95th percentile volume exceeds capacity, queue may be longer.
- 3. m Volume for 95th percentile queue is metered by upstream signal.
- 4. Highting denotes that the 95th percentile queue exceeds the available storage.

# SECTION 6 SITE ANALYSIS

#### Overview

Trips anticipated to be generated by the proposed development plan were forecasted and assigned to the surrounding roadway network. The generation, distribution, and assignment of site trips were based on the proposed redevelopment plan and program, as well as the locations of the future site entrance in relation to the surrounding roadway network.

#### **Existing Site Trips**

As stated previously, the site is currently developed with the 12-room Motel. The redevelopment plan calls for the elimination of the Motel. The existing trips associated with the site were removed from the network as summarized on Figure 6-1 based on existing traffic patterns.

#### **Proposed Site Access**

The site plan provided on Figure 1-2 shows that access is proposed via one (1) driveway along Campbell Drive and connects to the sites parking.

#### **Trip Generation**

Overview. Trip generation estimates for the AM and PM peak hours, as well as the average daily traffic, were derived from the standard Institute of Transportation Engineers (ITE) trip generation rates, as published in the Trip Generation Manual, 11<sup>th</sup> edition. The "Affordable Housing" (223) land use code was used for the proposed residential units. For purposes of this assessment, the "Small Office Building" (712) land use code was used for the office component. Affordable Housing was used because there was not an applicable land use code for the proposed permanent supportive housing type. Permanent supportive housing would generate fewer trips compared to standard affordable housing, so the analysis completed herein should be considered conservative.

The trip generation analysis for the existing and proposed uses is presented in Table 6-1. When compared to the existing use on site, the proposed development plan would result in an overall increase of 19 additional AM peak hour trips, an overall increase of approximately 25 additional trips during the PM peak hour and approximately 240 additional daily trips. For purposes of this study, the existing trips that were counted at the existing site driveways were removed throughout the study intersections as shown on Figure 6-1, and the total 23 AM peak hour trips and 29 PM peak hour trips for the proposed uses were added to the road network.

It should be noted that no reduction in site generated trips due to transit mode split was taken in this analysis. However, it is anticipated that the project would take advantage of public transit opportunities available in the proximity of the site. The Applicant currently subsidizes transit use for users of The Lamb Center.

Table 6-1
The Lamb Center
Trip Generation <sup>1</sup>

Land Use	ITE Land Use			<u>AN</u>	Л Peak Houi		<u>PN</u>	1 Peak Hou		Weekday
<b>《全界》(2015年)</b>	Code	Size	Units/SF	In	Out	Total	In	Out	Total	ADT
Existing Program										
Motel - General Urban/Suburban	320	12	Rooms	1	3	4	2	2	4	40
Proposed Program										
Affordable Housing - General Urban/Suburban	223	54	DU	6	13	19	15	10	25	260
Small Office Building - General Urban/Suburban	712	1,400	SF	2	2	4	2	2	4	20
Difference: Proposed minus Existing				7	12	19	15	10	25	240

#### Notes:

<sup>1.</sup> Trip Generation based on the Institute of Transportation Engineers' (ITE) <u>Trip Generation Manual</u>, 11th Edition.

#### **Site Trip Distribution**

As agreed upon in the scope with City staff, site trip distribution used in the analysis was based on existing travel patterns and engineering judgment. For purposes of this analysis, the following distribution was used in the forecasting of future site traffic:

To/From:	AM/PM
North on Lee Highway:	20%/10%
East on Fairfax Boulevard:	35%/20%
South on Old Lee Highway:	10%/5%
West on Fairfax Boulevard:	<u>35%/65%</u>
TOTAL	100%/100%

Figure 6-2 graphically illustrates this trip distribution.

#### **Site Trip Assignments**

The assignments of the total vehicle trips generated upon the future build-out of The Lamb Center redevelopment was based on the above distribution and are depicted on Figure 6-2.

#### **Parking Assessment**

Per the Zoning Ordinance, the following parking requirements are applicable to the site:

- 1 space per efficiency unit
- 1 space per 1-bedroom unit
- 2 spaces per 2-bedroom unit
- 1 space per 300 SF of office

Based on the following chart, the proposed 54 permanent supportive housing units and 1,400 SF of office would require approximately 77 parking spaces.

Type	Proposed	Required Spaces	
Efficient Units	41 units	51.25 spaces	
1-Bedroom	10 units	15 spaces	
2-Bedroom	3 units	6 spaces	
Office	1,400 SF	4.67 spaces	
Subtotal		77 spaces	
Proposed		18 spaces	

As shown on the plan, 18 parking spaces are proposed. The following information supports the reduction request of supplied parking spaces on the site.

<u>Affordable Housing.</u> Affordable units provide a reduced likelihood of vehicle ownership. Other nearby jurisdictions and national urban areas have provided for a standard parking reduction for affordable housing uses ranging from 30-50% from the standard market rate requirements depending on the level of affordability, typically between 30 and 60% AMI.

Vehicle ownership declines consistently with the level of AMI. The proposed permanent supportive housing type typically houses residents between 0 and 30% AMI, with the majority of residents designated towards the lower end of the AMI range. The Lamb Center serves poor and chronically homeless individuals in Fairfax City and surrounding jurisdictions.

<u>Permanent Supportive Housing.</u> The proposed site is unique to Northern Virginia and the City of Fairfax as an exclusively permanent supportive housing residential type. The parking ratios outlined in the Zoning Ordinance were based on research, data, and analysis associated with market rate developments. Based on information from other similar sites, approximately 10 percent of residents are anticipated to own a vehicle. This is consistent with the decline of vehicle parking requirements in other areas based on a linear decline of parking demand based on AMI level.

With the office uses supplying the required 5 parking spaces, approximately 13 spaces would be designated for the residential uses. Ten percent of the 54 proposed units would require 6 parking spaces, totaling 11 spaces to meet the parking demand.

Age-Restricted Housing Calculations. By nature of the population, many of the residents will be older adults. Based on the Institute of Transportation Engineers (ITE) Parking Generation, 5th Edition, Senior Adult Housing would require 0.61 spaces per unit and Mid-Rise Multifamily would require 1.31 spaces per unit. This is a reduction of 0.7 spaces per unit, or approximately 53%, from standard multifamily to senior adult housing. Therefore, estimates based on a multifamily zoning classification would provide significantly more parking than the demand of the age-restricted units. The Lamb Center will serve many older residents. Similar to the reduced likelihood of vehicle ownership of solely affordable units, affordable and age-restricted units would result in a combined reduced parking demand at the subject site.

<u>Transportation Demand Management (TDM)</u>. The Applicant currently subsidizes bus fare for specific purposes (such as medical appointments and interviews), as required at the bus stop adjacent to the site and connecting transit services. Most residents currently and will continue to utilize public transit subsidized by the Applicant. The Applicant will continue to serve its residents with increased comprehensive transportation options and subsidies, reducing the need and the likelihood of vehicle ownership. In addition, the Fairfax City CUE bus is free of charge within the City. The Lamb Center also recommends a bench with an armrest divider, bollard lighting, and a garbage receptable at the adjacent CUE bus stop to help further promote and encourage bus transit use.

<u>Short-Term Parking</u>. Based on discussions with the fire department, the approximately three (3) to four (4) street frontage parking spaces along Campbell Street will be designated as short-term parking to help meet short-term parking requirements. The on-street short-term parking will serve visitors to both office and residential uses on the site.

Similar Housing Sites. Based on the information provided below in table 6-2, it shows that for six (6) similar affordable & supportive housing sites within the region, that the maximum amount of parking spaces occupied compared to the total amount of units within the building, results in an average parking occupancy of 0.24. The proposed site contains 54 units with a total supply of 13 parking spaces for residents, resulting in a 0.24 parking ratio for the site. This further supports that with the site fully occupied that the maximum amount of parking spaces needed for residents would have to be, at most, 13 parking spaces, in order to be in line with similar parking occupancies for affordable & supportive housing sites within the region. The applicant will continue to seek offsite parking spaces.

Table 6-2
The Lamb Center
Similar Site Average Parking Occupancy

Property	Average Max Parking	Total Units	Bus Stop	Total
Cloverleaf 2011	12	60	Adjacent Bus Stop	0.19
Cloverleaf 2020	6			0.10
Crescent Square 2020	15	80	No Adjacent Bus Stop	0.18
Crescent Square 2021	28			0.36
Crossings 2020	13	60	No Adjacent Bus Stop	0.22
Church Street Stations 2021	20	80	Adjacent Bus Stop	0.26
Gosnold Apartments 2011	13	- 60	Adjacent Bus Stop	0.22
Gosnold Apartments 2021	27			0.45
South Bay 2011	13	60	Adjacent Bus Stop	0.22
Average				0.24

<u>Summary.</u> Based on the information provided above for the proposed residential type, it is anticipated that the proposed parking supply will adequately accommodate the parking demand of future residents and office users. The 18 proposed parking spaces and short-term on-street parking spaces will serve the site users based on the reduced AMI level of residents, serving many older adult residents, and commitment to TDM measures to reduce the need to own a vehicle.

#### TRANSPORTATION DEMAND MANAGEMENT (TDM)

The proposed permanent supportive housing and office uses are anticipated to utilize the nearby pedestrian, bicycle, and transit options surrounding the site. Further, the interior building amenities are designed to support the residents who are likely to utilize transit and not own a vehicle.

#### **TDM Strategies**

TDM is a general term for strategies that result in more efficient use of transportation resources. There are many different TDM strategies with a variety of impacts. Some improve the transportation options available to consumers, while others provide an incentive to choose more efficient travel patterns. Some reduce the need for physical travel through mobility substitutes or more efficient land use. TDM strategies can change travel timing, route, destination, or mode.

Such measures which may be appropriate to The Lamb Center development could include the following:

- A. Designate a Transportation Management Coordinator (TMC) to implement the TDM program and advise residents and employees of the availability and location of the TDM coordinator and program. The position may be part of other duties assigned to the individual. Duties of the Transportation Management Coordinator would include the following:
  - 1. Assist residents and employees in making effective and efficient commuting choices.
  - 2. Disseminate Metrorail, Metrobus, ridesharing, and other relevant transit options to new residents and employees.
  - 3. Solicit support from the Metropolitan Washington Council of Governments (MWCOG) Commuter Connections program, the Washington Metropolitan Area Transit Authority (WMATA), the Fairfax County government, and others.
  - 4. Provide on-site assistance to residents and employees in forming and maintaining carpools and vanpools.
  - 5. Encourage residents and employees to ride bikes or walk to work. Let residents know that they are welcome to use the bike racks that located in the bike room and on the exterior of the building.
  - 6. Market and promote the TDM Program among residents and employees through printed materials and web sites (if available). Display transit, vanpool, carpool, GRH, etc. brochures and flyers in lobby areas. Other promotions include, potentially, telework and transportation fairs.
- B. Incentives to use transit, including:
  - 1. Provide information on Metrorail, Metrobus, and other public transportation facilities, services, routes, schedules, and fares.
  - 2. Disseminate information to transit users regarding free guaranteed rides home in cases of emergency.

- 3. Provide safe, convenient, and attractive pedestrian connections on and off-site.
- 4. Provide ample bicycle parking/storage facilities. Bike racks are being incorporated into the design in a bike room and exterior to the building.

These strategies in addition to the transit subsidy program are anticipated to reduce the number of vehicle trips generated by the proposed uses.

#### **Other TDM Strategies**

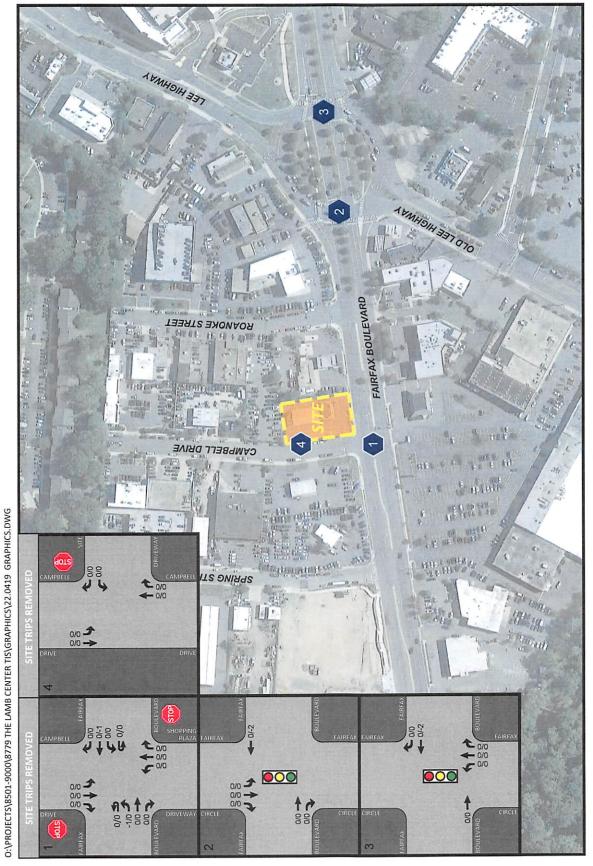
In response to staff comments the applicant has reviewed additional TDM strategies including:

Provide plan for Transportation Demand Management (TDM) and affordable transportation options. Potential elements could include (but are not limited to):

- a. Active parking management Resident and employee vehicles will require registration and parking pass.
- b. Transportation information center Bus schedules and assistance with directions will be available to residents and their guests from a 24-7 staffed front desk.
- c. The Lamb Center partners with the Fairfax City-based Shepherd's Heart Anglican Church Bike Ministry to provide its clients with refurbished bikes and bike repair services.
- d. The Lamb Center will support and promote regional resources, including the Commuter Connections Guaranteed Ride Home program.
- e. Transit subsidies Bus tokens will be provided free of charge to residents obtaining healthcare, employment, and other services. Most residents will be Medicaid recipients and thus entitled to use Medicaid Transportation Services for any form of healthcare or other Medicaid-approved services.

These proposed Transportation Demand Management (TDM) strategies will reinforce the parking reduction that is requested by the applicant given the projects proximity to transit, bicycle, and other multimodal facilities near the site which minimizes the projects vehicle traffic impacts and need for parking.





Site Trips Removed Figure 6-1

Represents One Travel Lane Signalized Intersection Study Intersection

**◎ ∞** √ **()** 

Stop Sign



Figure 6-2 Site Generated Trips

Study Intersection

Signalized Intersection

Represents One Travel Lane
Stop Sign

NORTH
The Lamb Center
City of Fairfax, Virginia

## SECTION 7 ANALYSIS OF FUTURE CONDITIONS WITH SITE DEVELOPMENT

#### **Total Future Traffic Forecasts**

Site trips removed as shown on Figure 6-1 and trip assignments as shown on Figure 6-2 were added to the background traffic forecasts to yield 2026 total future traffic forecasts, as shown on Figure 7-1.

#### **Proposed Improvements**

Additionally, access to the existing Motel is currently provided at three (3) locations, two (2) along Campbell Drive and one (1) right-in/right-out driveway along Fairfax Boulevard. The Applicant intends to consolidate these access drives to a single location providing enhanced access management along Campbell Drive.

Lane use and traffic control at each of the study intersections for 2026 total future conditions is shown on Figure 7-2.

#### **Total Future Levels of Service with Proposed Development Plan**

Future levels of service with the proposed development plan were determined at the study intersection based on the future traffic volumes shown on Figures 7-1, future lane use and traffic control shown on Figures 7-2, and the 2000 HCM methodologies for signalized and unsignalized intersections calculated using the Synchro 10 traffic analysis software. The results of these analyses are provided in Appendix E and summarized in Table 7-1.

As shown in Table 7-1, levels of service under future site development conditions would remain generally consistent with future background conditions (i.e., without site development).

The analyses show that the signalized intersections at Fairfax Boulevard/Fairfax Circle will continue to operate at level of service "C" (LOS "C") or better during the AM and PM peak commuter periods. The approaches to the signalized intersections will continue to operate at LOS "C" or better with the exception of the northbound approach at the intersection of Fairfax Boulevard/Fairfax Circle (East) which will continue to operate at LOS "E" during the AM peak hour.

All approaches at the unsignalized intersections of Fairfax Boulevard/Campbell Drive and Campbell Drive/Site Driveway will operate at LOS "D" or better during the AM and PM peak periods.

#### **Total Future Queue Analysis**

The results are presented in Appendix E and summarized in Table 7-2. With the addition of the proposed development the projected 50th and 95th percentile queues would remain generally consistent to future conditions without development. When compared to future conditions without development estimate per hour queuing would increase by one (1) vehicle or less. The eastbound through at the intersection of Fairfax Boulevard/Fairfax Circle (West) would continue to block the southbound left movement at the intersection of Fairfax Boulevard/Fairfax Circle (West) during the AM peak hour. Residents, employees, and visitors of the site will have to utilize gaps in traffic to perform the southbound left turn onto Fairfax Boulevard from Campbell Drive. Field observations indicate that gaps in traffic are become several times throughout each adjacent cycle length of 220 seconds. Outbound traffic from the proposed development would be able to access Fairfax Boulevard without an unacceptable amount of delay by utilizing gaps in traffic.



Figure 7-1
Future Conditions with Development
Peak Hour Traffic Volumes

Study Intersection

Signalized Intersection

Represents One Travel Lane
Stop Sign

NORTH
The Lamb Center
City of Fairfax, Virginia



Figure 7-2
Future Conditions with Development
Lane Use and Traffic Control

Study Intersection
Signalized Intersection
Represents One Travel Lane
Stop Sign

NORTH
The Lamb Center
City of Fairfax, Virginia

Table 7-1
The Lamb Center
Future Conditions with Development Intersection Level of Service Summary <sup>1, 2</sup>

		Existing Co	onditions		Future (	Conditions wi		velopment	Future	e Conditions v (202		elopment
Approach/ Lane Group	AM P	eak Hour	PM Pe	eak Hour	AM P	eak Hour	PM P	eak Hour	AM P	eak Hour	PM P	eak Hour
	LOS	Delay (s)	LOS	Delay (s)	LOS	Delay (s)	LOS	Delay (s)	LOS	Delay (s)	LOS	Delay (s)
1. Fairfax Boulevard/Cam	pbell Dr	ive - Unsignal	ized									
EB Approach	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.2
WB Approach	Α	0.2	Α	0.3	Α	0.2	Α	0.3	Α	0.2	Α	0.3
NB Approach	С	24.4	C	17.9	С	23.7	C	18.8	С	24.0	C	19.8
SB Approach	С	20.4	В	14.2	С	20.0	В	14.5	С	23.4	D	26.3
2. Fairfax Boulevard/Fair	fax Circle	e (West) - Sign	nalized									
EB Approach	В	16.5	В	10.7	В	17.8	В	10.9	В	18.0	В	10.9
WB Approach	Α	3.7	Α	3.5	Α	4.0	Α	3.7	Α	4.1	Α	3.7
SB Approach	D	36.2	C	29.4	С	34.8	С	31.2	C	34.6	C	31.1
Overall	В	18.0	В	15.6	В	18.4	В	16.3	В	18.4	В	16.3
3. Fairfax Boulevard/Fair	fax Circle	e (East) - Signa	alized									
EB Approach	Α	2.6	Α	2.9	А	2.5	Α	2.9	Α	2.6	Α	2.9
WB Approach	В	11.5	Α	6.6	В	12.2	Α	6.7	В	12.3	Α	6.7
NB Approach	D	54.9	В	17.6	E	55.3	В	17.9	Ε	55.3	В	18.0
Overall	С	21.7	Α	8.0	С	22.0	Α	7.9	С	22.0	Α	8.0
4. Future Site Driveway/0	Campbel	l Drive - Unsig	gnalized									
WB Approach	-	-	-	-	-	-	-	-	Α	8.9	Α	8.9
NB Approach	-	-	-	-	-	-	-	-	Α	0.0	Α	0.0
SB Approach	-	-	-	-	-		-	-	Α	0.0	Α	0.0

## Note(s):

<sup>1.</sup> Capacity analysis based on Highway Capacity Manual methodology, using Synchro 10.

<sup>2.</sup> Highting denotes that the Level of Service is at or beyond capacity.

Future Conditions with Development Intersection Queuing Summary 1, 2, 3, 4

	Storage		Existing C	Conditions		Future C	onditions with (2026)	Future Conditions without Development (2026)	opment	Future Co	nditions wit	Future Conditions with Development (2026)	ant (2026)
Approach/ Lane	Length	AM Pe	AM Peak Hour	PM Peak Hour	ık Hour	AM Peak Hour	k Hour	PIM Pea	PM Peak Hour	AM Peak Hour	ak Hour	PM Peak Hour	k Hour
Group	(£	50th	95th	50th	95th	50th	95th	50th	95th	50th	95th	50th	95th
		Pecentile	Pecentile	Pecentile	Pecentile	Pecentile	Pecentile	Pecentile	Pecentile	Pecentile	Pecentile	Pecentile	Pecentile
. Fairfax Boulevard/Campbell Drive - Unsignalized	ampbell Dr	rive - Unsign	alized										
EBL	100	a to	1	1	1	1	2	ī	1		2	•	4
EBTR	1	1	0	i	0	,	0	1	0	ī	0		0
WBL	80	a	3	,	2		С	,	9		e	•	9
WBTR	,		0	,	0		0	î	0	ï	0	ï	0
NBLTR	,		П	,	12	٠	1	ï	12	•	1	ř	13
SBLTR		•	10		7		9	ı	7		16		21
Fairfax Boulevard/Fairfax Circle (West) - Signalized	airfax Circl	e (West) - Si	ignalized										
EBT		484	615	190	213	527	999	199	223	533	673	200	224
EBR		169	256	92	122	194	290	100	132	198	296	101	133
WBT	,	47	53	65	99	53	09	29	89	23	09	29	89
SBT	ē	244	285	330	#412	245	284	338	#441	244	284	339	#441
SBR	,	0	0	0	0	0	0	0	0	0	0	0	0
3. Fairfax Boulevard/Fairfax Circle (East) - Signalized	airfax Circl	e (East) - Sig	gnalized										
EBT	3	40	45	30	m32	34	37	30	m31	34	38	30	m31
WBT	•	153	181	142	m211	165	195	146	m220	166	195	147	m221
WBR	200	51	77	51	m76	23	79	52	m77	23	80	25	m77
NBT	ı	541	591	126	m142	561	612	124	m147	564	615	124	m148
NBR		0	0	0	m0	0	0	0	m0	0	0	0	m0
1. Future Site Driveway/Campbell Drive - Unsignalized	y/Campbe	II Drive - Un	signalized										
WBLR					·	· ·		•			1	ı	Н
NBTR		i	i	ı	1	٠		ı			0		0
SBLT	•	•		•				•			0		0
1-1-(-)													

Note(s):

 $1. \sim Volume exceeds capacity, queue is theoretically infinite.$ 

2. # 95th percentile volume exceeds capacity, queue may be longer.

3. m Volume for 95th percentile queue is metered by upstream signal. 4. Highting denotes that the 95th percentile queue exceeds the available storage.

## SECTION 8 CONCLUSIONS

Based on the results of this revised traffic impact study, the following may be concluded:

- The Fairfax Boulevard/Fairfax Circle signalized intersections currently operate at an overall LOS "C" or better during the AM and PM commuter peak periods based on Highway Capacity Manual calculations. The approaches at the unsignalized intersection of Fairfax Boulevard/Campbell Drive currently operate at LOS "C" or better during the peak periods.
  - Estimated queues would generally be accommodated within the available storage areas. The eastbound queue towards the intersection of Fairfax Boulevard would potentially extend back to Campbell Avenue during the peak periods. Commuters would be required to utilize gaps in traffic recorded by field observations to occur several times per cycle length.
- 2. A review of VDOT AADT volumes along Fairfax Boulevard in the vicinity of the site indicates a reduction in traffic volumes from 2016 to 2019. AADT volumes along Fairfax Boulevard east of Draper Drive fell from 42,000 vehicles in 2016 to 35,000 vehicles in 2019.
- 3. The approved pipeline development Wawa site at 9700 Fairfax Boulevard is anticipated to generate a net new 399 AM peak hour trips and 327 PM peak hour trips at full buildout.
- 4. Under future 2026 traffic conditions, minimal increases in delay at the study intersections are expected due to the trips generated by approved pipeline developments in the vicinity of the site and overall levels of service would remain generally consistent with existing conditions with the exception of the northbound approach at the intersection of Fairfax Boulevard/Fairfax Circle (East) which would degrade from LOS "D" to LOS "E" during the AM peak hour.
- 5. The site is currently developed with the 12-room Motel. The Applicant proposes to redevelop the site with 54 residential units and up to 1,400 SF of office.
- 6. The project is estimated to generate 23 AM peak hour trips and 29 PM peak commuter hour trips upon buildout.
- 7. Under future 2026 traffic conditions, with the development of the subject site, intersection levels of service would remain generally consistent with existing and background conditions. The analyses show that the Fairfax Boulevard signalized intersections will continue to operate at LOS "C" or better during the AM and PM peak periods. The site is estimated to have a minimal impact on network queueing. Commuters would continue to be able to utilize gaps in traffic on Fairfax Boulevard during the peak periods.

- 8. All unsignalized intersection and access drive approaches will operate at LOS "D" or better during each of the studied peak periods.
- 9. Access to the site and parking garage will be via one (1) full access driveway along Campbell Drive.
- 10. Access to the existing Motel is currently provided at two locations along Campbell Drive and one right-in/right-out driveway along Fairfax Boulevard. The Applicant intends to consolidate these access drives to a single location providing enhanced access management.

## APPENDIX A City of Fairfax Scoping Agreement



## SCOPE OF WORK MEETING FORM

## Information on the Project Traffic Impact Analysis Base Assumptions

## THE LAMB CENTER PROPERTY CITY OF FAIRFAX, VIRGINIA June 16, 2022

<b>Contact Information</b>						ounty: City of Fairfax  d (Route 50) and east of  lan Subd Plat Ses and redevelop with a 54- office space served by 18 provide additional space or location. The office space reducing the potential rigure 2.  Other Ses Sec						
Consultant Name: Tele:	703.676.3627			lls +	Associates, Inc.							
E-mail:	gpvaughan@wellsan	dassociate	es.com									
Developer/Owner Name:	Judith Cabelli											
Tele:	703.642.3830 x242											
E-mail:	jcabelli@whdc.org			-								
<b>Project Information</b>				rfax Boulevard (Route 50) and east of location.  Site Plan Subd Plat string motel uses and redevelop with a 54 and a 1,400 office space served by 18 senvisioned to provide additional space at Lamb Center location. The office space the building, reducing the potential sprovided as Figure 2.								
Project Name:	The Lamb Center TIS	5		Lo	ocality/County:	City of Fairfax						
Project Location: (Attach regional and site specific location map)						50) and east of						
Submission Type	Comp Plan	Rezoni	g to raze the existing motel uses and redevelop with a 54-housing project and a 1,400 office space served by 18 e office space is envisioned to provide additional space led by the current Lamb Center location. The office space is to residents of the building, reducing the potential in the Site Layout is provided as Figure 2.  Mixed Use  Other  Other									
Project Description: (Including details on the land use, acreage, phasing, access location, etc. Attach additional sheet if necessary)	unit permanent suppo on-site parking space for similar services pr will likely provide se	ortive hours. The off rovided by rvices to i	oraze the existing motel uses and redevelop with a 54- busing project and a 1,400 office space served by 18 office space is envisioned to provide additional space by the current Lamb Center location. The office space oresidents of the building, reducing the potential Site Layout is provided as Figure 2.  Mixed Use  Other  Other  ITE LU Code(s):									
Proposed Use(s): (Check all that apply; attach additional pages as necessary)	unit permanent supportive housing project and a 1,400 office space served by 1 on-site parking spaces. The office space is envisioned to provide additional spat for similar services provided by the current Lamb Center location. The office s will likely provide services to residents of the building, reducing the potential vehicle trips generated. The Site Layout is provided as Figure 2.  Residential  Commercial  Mixed Use  Other  Commercial  Other  Commercial											
(See Table 1)	Existing Lodging Us Number of Units: ITE LU Code(s):	ses(s)	12 320									
	Proposed Uses(s) Number of Housing U ITE LU Code(s):	Units:	<u>54</u> <u>223</u>	of the building, reducing the potential is provided as Figure 2.  Mixed Use  Other  Other Use(s)  ITE LU Code(s):								
	SF of Office Space: ITE LU Code(s):		1,400 712									
	Parking Spaces:		10	Locality/County: City of Fairfax  Fax Boulevard (Route 50) and east of ocation.  Site Plan Subd Plat Sing motel uses and redevelop with a 54-and a 1,400 office space served by 18 envisioned to provide additional space Lamb Center location. The office space he building, reducing the potential provided as Figure 2.  Mixed Use Other Other Single County								

Total Peak Hour Trip Projection:	Less than 100 🖂	1	00 – 499		500 – 9	999 [		1,000 or	r more 🗌
Traffic Impact Analysis	Assumptions								
Study Period	Existing Year: 2022		Build-out	Year: 2	026		Desig	gn Year:	n/a
Chile Anna Davidacias	North: Northern Site	Bou	ındary	South: I	Fairfax I	Boulev	ard (U	S Route	50)
Study Area Boundaries	East: Roanoke Street	t		West: C	Campbel	l Drive	Į.		
External Factors That Could Affect Project (Planned road improvements, other nearby developments)	N/A								
Consistency With Comprehensive Plan (Land use, transportation plan)	The proposed develor identifies the site alo CR (Commercial Re	ng E	Fairfax Bo	ılevard a	as "Com	mercia	l Corr		
Available Traffic Data (Historical, forecasts)	VDOT historical tra 2020 VDOT Averag Fairfax Boulevard (1) 2019 VDOT Averag Fairfax Boulevard (1) 2018 VDOT Averag Fairfax Boulevard (1) 2017 VDOT Averag Fairfax Boulevard (1) 2016 VDOT Averag Fairfax Boulevard (1) 2016 VDOT Averag Fairfax Boulevard (1)	ge Ar US F ge Ar US F ge Ar US F ge Ar	nnual Daily Route 50): Innual Daily Route 50): Innual Daily Route 50): Innual Daily Route 50): Innual Daily Route 50):	y Traffic 28,000 y Traffic 35,000 y Traffic 35,000 y Traffic 35,000 y Traffic	TGAA):	<u>C):</u> C):			
Trip Distribution (AM/PM) (Pending data	From the West: 35%/6	65%		Fron	n the Sou	ıth: 10%	%/5%		
from existing traffic counts) (See Figure 1)	From the North: 20%/	10%		Fron	n the Eas	st: 35%/	′20%		
Annual Vehicle Trip			Period for all that apply			⊠ A	M [	⊠ PM	SAT
Growth Rate:		Peak	Hour of th	e Genera	ator	N/A			
Study Intersections and/or Road Segments (See Figure 1)	1. Fairfax Boulevard/	Camp	obell Drive	3. Fa	airfax Bo	ulevard	l/Fairfa	ıx Circle	(East)
	2. Fairfax Boulevard/ (West)	Fairfa	ax Circle	4. Pı	roposed S	Site Dri	veway	/Campbe	ll Drive
Trip Adjustment Factors	Internal allowance: Reduction:% tr		es 🛭 No		-by allov uction:			es 🛛 N	0
Software Methodology	Synchro HCS	(v.2	000/+)	aaSIDR	A 🗌 C	ORSIM	_ o	ther Synd	chro Version 10

Traffic Signal Proposed or Affected (Analysis software to be used, progression speed, cycle length)	None
Improvement(s) Assumed or to be Considered	None
Background Traffic Studies Considered	Wawa – 9700 Fairfax Boulevard – 6,049 SF Grocery Store, 12 Fueling Stations
Plan Submission	Master Development Plan (MDP)
Additional Issues to be Addressed	☐ Queuing analysis       ☐ Actuation/Coordination       ☐ Weaving analysis         ☐ Merge analysis       ☐ Bike/Ped Accommodations       ☐ Intersection(s)         ☐ TDM Measures       ☐ Other Parking Reduction Study

#### **NOTES on ASSUMPTIONS:**

- Synchro 10 will be used to conduct capacity analysis with peak hour factors measured in the field for existing conditions (0.85<PHF). Under background and total future conditions, a minimum PHF of 0.92 will be used for all movements.
- 2. Existing Synchro (signal timing) files to be provided by the city.
- 3. Additional information supporting the parking reduction request is attached to this scope. This information will summarize the reduced parking demand of the proposes residential type and justify the proposed supply.
- 4. Similar permanent supportive housing locations will be counted during peak hours to compare to the proposed trip generation for the Lamb Center site.
- 5. Traffic counts collected in 2022 will be reviewed and balanced with turning movement or VDOT data provided prior to the COVID-19 pandemic.

## **SCOPE OF WORK MEETING**

## ADDITIONS TO THE REQUIRED ELEMENTS, CHANGES TO THE METHODOLOGY OR STANDARD ASSUMPTIONS, AND SIGNATURE PAGE

Any additions to the Required Elements or changes to the Methodology or Standard Assumptions due to special circumstances that are approved by the City of Fairfax:

AGREED:Consultant	DATE:
PRINT NAME: <u>Grady P. Vaughan, PE, l</u> Consultant	PTOE, PTP
SIGNED:	DATE:
PRINT NAME:	<del></del>
Attachments: Figure 1 – Site Location, Study Intersections, Figure 2 – Site Layout Table 1 – Trip Generation	, and Directional Distributions



Site, Intersection Location, and Directional Distributuions Figure 1

Study Intersection







Figure 2 Site Plan



Table 1
The Lamb Center
Trip Generation <sup>1</sup>

Land Use	ITE Land Use Code	Size	Units/SF	<u>A</u> In	M Peak Hour Out	Total	<u>Pi</u> In	<u>M Peak Hour</u> Out	Total	Weekday ADT
Existing Program										
Motel - General Urban/Suburban	320	12	Rooms	1	3	4	2	2	4	40
Proposed Program										ı
Affordable Housing - General Urban/Suburban	223	54	DU	6	13	19	15	10	25	260
Small Office Building - General Urban/Suburban	712	1,400	SF	2	2	4	2	2	4	20
Difference: Proposed minus Existing				7	12	19	15	10	25	240

#### Notes:

<sup>1.</sup> Trip Generation based on the Institute of Transportation Engineers' (ITE) <u>Trip Generation Manual</u>, 11th Edition.

## WELLS + ASSOCIATES

#### MEMORANDUM

To: Tara Ruszkowski

The Lamb Center

Copy: Aaron Vinson, P.E.

Walter L. Phillips

From: Michael J. Workosky, PTP, TOPS, TSOS

Evan S. Gittelman

Date: May 3, 2022

Re: The Lamb Center

Parking Reduction Study Approach

Fairfax, Virginia



1420 Spring Hill Road, Suite 610, Tysons, VA 22102 703-917-6620 WellsandAssociates.com

#### INTRODUCTION

This memorandum summarizes the approach to prepare a parking reduction study for the proposed redevelopment of the 9640 Fairfax Boulevard property, located in the City of Fairfax, Virginia. The subject site is bounded by Campbell Drive to the west, Fairfax Boulevard to the south, and Roanoke Street to the east. The Applicant proposes to demolish the existing 12-room Hy-Way Motel building and redevelop the site with 54 affordable residential dwelling units and 1,400 SF of office uses. Nearly all of the units (51 units) are studio or one-bedroom units. The proposed uses would be served a ground-level parking garage with a total of 18 parking spaces.

The building would require a total of 77 parking spaces based on the City of Fairfax Zoning Ordinance. Thus, based on the proposed parking supply, a parking reduction of 59 spaces would be required.

#### PARKING REDUCTION STUDY APPROACH

The building is planned to primarily serve as permanent supportive housing for those that fall into the 60% of the Average Median Income (AMI) category. Many of the residents do not own a car or create the need for a parking space. The office space will serve administrative staff and provide space for job interviews or other training services for residents of the building. Therefore, the parking supply is likely to be primarily used by office visitors and staff, with only limited use by residents.

The proposed parking reduction will be evaluated through a review of the anticipated operations of the proposed building, a survey or review of comparable buildings if available, or through other published sources such as the Institute of Transportation Engineers (ITE) or surrounding jurisdictions. The study will also document the multiple bus lines that provide direct access to the nearby Vienna/Fairfax George Mason University metro station and other services.

## **WELLS + ASSOCIATES**

## **MEMORANDUM**

The results of the study will be documented in a summary report for review by the City.

Questions regarding this document should be directed to Wells + Associates.

O:\Projects\8501-9000\8779 The Lamb Center TIS\Documents\The Lamb Center Parking Reduction Approach Memo (5.3.22).docx

# APPENDIX B Existing Count Data

### Tysons, Virginia

### **Turning Movement Count - Total Vehicles**

PROJECT: The Lamb Center TIS

DATE: 4/12/2022

SOUTHBOUND ROAD: Campbell Drive

WHA JOB NO: p8779

DAY: Tuesday

NORTHBOUND ROAD: Campbell Drive

INTERSECTION: Campbell Dr. & Site Entr.- North

WEATHER: clear

WESTBOUND ROAD: Site Entrance - North

LOCATION: City of Fairfax.VA

COUNTED BY: Agan

EASTBOUND ROAD: 0

LOCATION:	City o	f Fairfax	,VA					- 1	COUN	TED I	BY:	Agan				E	ASTE	BOUN	ID RO	<b>AD</b> : (	0						- 1
									INPU	TED I	BY:	agan															_
			Southbo						Westb						Northbo						Eastbo	und				East	!
Time			ampbell						Entranc						ampbell			-: :-			0			51.15	. &		Total
Period	Right	Thru	Left J-	Turn	Total	PHF	Right	Thru	Left J.	Turn	Total	PHF	Right	Thru	Left J-	Turn	Total	PHF	Right	Thru	Left J-	Turn	otal	PHF	South V	vest	-
15 Minute Volumes						Transportation (C. C.												\$154.0 TE				_		A Company			4
6:00 AM - 6:15 AM	0	1_	0	0			0	0	0	0	0		0	3	0	0	3	Angest -	0	0	0	0	0	97	<u> </u>	0	
6:15 AM - 6:30 AM	0	!_	0	0			0	0	0	0	0		0	4	0_	0	4		0	0	0	0	0		- 6	0	-6
6:30 AM - 6:45 AM	0	0	0	0	0		0	0	0	0	0		0	6	0	0	6 8		0	0	0	0	0		-8	0	- 8
6:45 AM - 7:00 AM	0	0	0	0	0		0	0	0	0	0		0	8	0	0			-	-0	0	0	尚		7	0	+
7:00 AM - 7:15 AM	0	4	0	0	4		0	0	0	0	0		0	3			3		<del> </del>	-0	-	-	-	50.000 C	<del>- /</del>	-	<del>- 11</del>
7:15 AM - 7:30 AM	0	5	0	0	5		0	0	0	0_	0		0	6	0	0	6 5		0	-	0	-	ö	-0.0	9	- 6	<del>''</del>
7:30 AM - 7:45 AM	0	4	0	0	4		0	0	0	0_	0		0	<u> 5</u> 8	0	0	8		0	0	0	0	ŏ		17	0	17
7:45 AM - 8:00 AM	0	9	0	0	9		0	0	0	0	0		-0		0	-0-	- 6	49.0	0	- 0	<del>-</del> 0	-	-		- 17	0	<del>'</del> 9
8:00 AM - 8:15 AM	0	3	0	0			0	0	0	0	0	150 Au 170 Au	_ <u>`</u>	13	-0	0	14		0	- 0	<del>-</del>	-	0		21	0	21
8:15 AM - 8:30 AM	0		0	0	7		0	0	0	0	_0		-	3	-0	0	3		0	-	-0	0	-	A CONTRACTOR	7	0	7
8:30 AM - 8:45 AM	0	4		0	4	Table Co.	0	0	0	0	0		- 0	10	0	-	10		0	0	-0	0	히		13	0	13
8:45 AM - 9:00 AM	0		0	0	3		0	0	0	0	-		-0	2	-	-	2	10 4 5 4 5	0	0	- 0	0	ő		3	0	3
4:00 PM - 4:15 PM	0		0	0_			0	0	-0	-0	-		0	9	-	<del>-</del>	- 9	2020	0	0	-	0	-	10261/	20	0	20
4:15 PM - 4:30 PM	0	- 11	0	0	11		0	0	0	0	-		0	2	-	-	2		0	0	-	0	Ö	= + .	10	0	10
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4:30 AM - 4:45 AM	0		- 0	-0	-		0	0	-	-	0	5.4.35	0		0	0	0		0	0	0	0	0		0	0	0
4:45 AM - 5:00 AM	1 0		<del>-</del>	-	- 0		0	<del>-</del>	- 0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0
5:00 AM - 5:15 AM	1 0	_	0	-	0	FE-52	0	-	0	-	ō		0	0	0	0	0	10.0	0	0	0	0	0	7.5	0	0	0
5:15 AM - 5:30 AM	1 0		0	0	-	ARTICIPATION.	-	0	0	0	0	<b>7</b>	0	0	0	0	0	91,000	0	0	0	0	0		0	0	0
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One Hour Volumes	_																										
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6:30 AM - 7:30 AM	1 0		0	0	9	_	0	0	0	0	0		0	23	0	0	23	0.72	0	0	0	0	0		32	0	32
6:45 AM - 7:45 AM	1 6		0	0	13	0.65	0	0	0	0	0	1	0	22	0	0	22	0.69	0	0	0	0	0		35	0	35
7:00 AM - 8:00 AM	1 0		0	0	22	0.61	0	0	0	0	0		0	22	0	0	22	0.69	0	0	0	0	0		44	0	44
7:15 AM - 8:15 AM	10		0	0	21	0.58	0	0	0	0	0		0	25	0	0	25	0.78	0	0	0	0	0		46	0	46
7:30 AM + 8:30 AM	0	23	0	0	23	0.64	0	0	0	0	0		I	32	0	0	33	0.59	0	0	0	0	0		\$6	0	56
7:45 AM - 8:45 AM	0	23	0	0	23	0.64	0	0	0	0	0			30	0	0	31	0.55	0	0	0	0	0	L	54	0	54
8:00 AM - 9:00 AM	0	17	0	0	17	0.61	0	0	0	0	0			32	0	0	33	0.59	0	0	0	0	0	<u> </u>	50	0	50
4:00 PM - 5:00 PM	0	26	0	0	26	0.59	0	0	0	0	0		0	20	0	0	20	0.56	0	0	0	0	0	<u> </u>	46	0	46
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4:30 PM - 5:30 PM	0	28	0	0	28	0.64	0	0	0	0	0		0	16	0	0	16	0.57	0	0	0	0	0	<u> </u>	44	0	44
4:45 PM - 5:45 PM	0	23	0	0	23	0.52	0	0	0	0	0		0	14	0	0	14	0.5	0	0	0	0	0	L	37	0	37
5:00 PM - 6:00 PM	10		0	0	21	0.48	0	0	0	0	0		0	10	0	0	10	0.5	0	0	0	0	0	L	31	0	31
5:15 PM - 6:15 PM	0	14	0	0	14	0.88	0	0	0	0	0		0	6	0	0	6	0.5	0	0	0	0	0	<u> </u>	20	0	20
5:30 PM - 6:30 PM	0	13	0	0	13	0.81	0	0	0	0	0		0	4	0	0	4	0.33	0	0	0	0	0	<u> </u>	17	0	17
5:45 PM - 6:45 PM	0	13	0	0	13	0.81	0	0	0	0	0		0	7	0	0	7	0.58	0	0	0	0	0	<u> </u>	20	0	20
6:00 PM - 7:00 PM	0	9	0	0	9	0.56	0	0	0	0	0		0	4	0	0	4	0.33	0	0	0	0	0	<u> </u>	13	0	13

## Tysons, Virginia

## **Turning Movement Count - Bicycles**

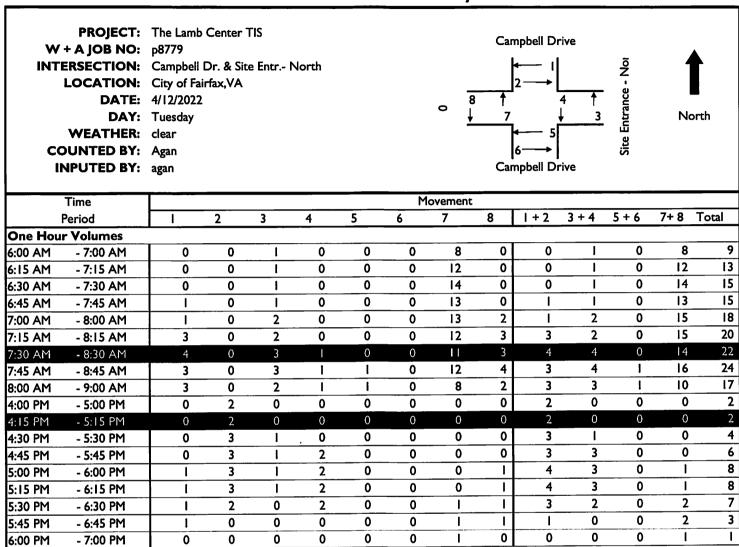
PROJECT: The Lamb Center TIS DATE: 4/12/2022 UTHBOUND ROAD: Campbell Drive W+A JOB NO: p8779 DAY: Tuesday RTHBOUND ROAD: Campbell Drive INTERSECTION: Campbell Dr. & Site EntWEATHER: clear /ESTBOUND ROAD: Site Entrance - North

LOCATION: City of Fairfax, VA COUNTED BY: Agan :ASTBOUND ROAD: 0

				INP	UTED	BY: a	agan												
		Southb	ound			Westb	ound			Northb	ound			Eastbo	ound		North	East	
Time		ampbe	ll Driv	е	Site	Entrand	:e - No	orth		ampbel	ll Driv	е		0			&	&	Total
Period	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	South	West	
One Hour Volumes																			
6:00 AM - 7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0_	0	0	0	0	0
6:15 AM - 7:15 AM	0	0	0	0	0	0	0_	0	0	1	0	- 1	0	0	0	0		0	
6:30 AM - 7:30 AM	0	0	0	0	0	0	0	0	0		0		0	0	0	0		0	
6:45 AM - 7:45 AM	0	0	0	0	0	0	0	0	0	1	0		0	0	0	0		0	<u> </u>
7:00 AM - 8:00 AM	0	0	0	0	0	0	0	0	0	2	0	2	0	0_	0	0	2	0	2
7:15 AM - 8:15 AM	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1	0	- 1
7:30 AM - 8:30 AM	0	0	0	0	0	0	0	0	0	1	0		0	0	0	0		0	I
7:45 AM - 8:45 AM	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	2	0	2
8:00 AM - 9:00 AM	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1	0	
4:00 PM - 5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM - 5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM - 5:30 PM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	
4:45 PM - 5:45 PM	0	1	0	- 1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
5:00 PM - 6:00 PM	0	i	0	ı	0	0	0	0	0	0	0	0	0	0	0	0		0	l
5:15 PM - 6:15 PM	0	ı	0	T	0	0	0	0	0	0	0	0	0	0	0	0		0	- 1
5:30 PM - 6:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM - 6:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:00 PM - 7:00 PM	0	ı	0	ı	0	0	0	0	0	0	0	0	0	0	0	0		0	1

## Tysons, Virginia

## **Pedestrian Volume Survey**



## Tysons, Virginia

### **Turning Movement Count - Total Vehicles**

DATE: 4/12/2022 SOUTHBOUND ROAD: Campbell Drive PROJECT: The Lamb Center TIS W+A JOB NO: p8779 DAY: Tuesday NORTHBOUND ROAD: Campbell Drive INTERSECTION: Campbell Dr. & Site Entr.- South WEATHER: clear WESTBOUND ROAD: Site Entrance - South

LOCATION:	City of Fairfax,VA	COUNTED BY: Agan INPUTED BY: agan	EASTBOU	ND ROAD: 0	
	Southbound	Westbound	Northbound	Eastbound	North East
Time	Campbell Drive	Site Entrance - South	Campbell Drive	0	& & Total
Period	Right Thru Left-Turn Total PHF	<del></del>	Right Thru Left-Turn Total PHF		
15 Minute Volumes	<del></del>	Right Thru Leit-Turn Total 174	right that Leit-Turn rotal tri	Tright This Edit Total Total Title	
6:00 AM - 6:15 AM	0 1 0 0 1	100000	0 3 0 0 3	100000	4 0 4
6:15 AM - 6:30 AM		1 0 0 0 0	0 4 0 0 4		5 0 5
6:30 AM - 6:45 AM	0 0 0 0	0 0 0 0	0 7 0 0 7	0 0 0 0	7 0 7
6:45 AM - 7:00 AM			0 8 0 0 8	0 0 0 0	8 0 8
7:00 AM - 7:15 AM	0 6 0 0 6	0 0 0 0	0 5 0 0 5	0 0 0 0	11 0 11
7:15 AM - 7:30 AM	0 4 0 0 4	0 0 0 0	0 6 0 0 6	0 0 0 0	10 0 10
7:30 AM - 7:45 AM	0 4 0 0 4	0 0 0 0	0 5 0 0 5	0 0 0 0	9 0 9
7:45 AM - 8:00 AM	0 10 0 0 10	1 0 0 0 0	0 8 0 0 8	0 0 0 0	18 0 18
8:00 AM - 8:15 AM	0 3 0 0 3	0 0 0 0	0 7 0 0 7	0 0 0 0	10 0 10
8:15 AM - 8:30 AM	0 8 0 0 8	1 0 0 0 0 0	0 11 0 0 11	0 0 0 0	19 0 19
8:30 AM - 8:45 AM	0 5 0 0 5	0 0 0 0	0 3 0 0 3	0 0 0 0	8 0 8
8:45 AM - 9:00 AM	0 3 0 0 3	0 0 0 0	0 9 0 0 9	0 0 0 0	12 0 12
4:00 PM - 4:15 PM	0 2 0 0 2		0 3 0 0 3	0 0 0 0	5 0 5
4:15 PM - 4:30 PM	0 11 0 0 11	0 0 0 0	0 7 0 0 7	0 0 0 0	18 0 18
4:30 PM - 4:45 PM	0 8 0 0 8		0 2 0 0 2	0 0 0 0	10 0 10
4:45 PM - 5:00 PM	0 6 0 0 6		0 7 0 0 7	0 0 0 0	13 0 13
5:00 PM - 5:15 PM	0 12 0 0 12		0 6 0 0 6	0 0 0 0	18 0 18
5:15 PM - 5:30 PM	0 3 0 0 3	0 0 0 0	0 2 0 0 2	0 0 0 0	5 0 5
5:30 PM - 5:45 PM	0 3 0 0 3	0 0 0 0	0 0 0 0	0 0 0 0	3 0 3
5:45 PM - 6:00 PM	0 4 0 0 4	1 0 0 0 0	0 3 0 0 3	0 0 0 0	7 0 7
6:00 PM - 6:15 PM	0 4 0 0 4	0 0 0 0	0 1 0 0 1	0 0 0 0	5 0 5
6:15 PM - 6:30 PM	0 2 0 0 2	0 0 0 0	0 0 0 0		2 0 2
6:30 PM - 6:45 PM	0 3 0 0 3		0 3 0 0 3	0 0 0 0	6 0 6
6:45 PM - 7:00 PM	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0
4:00 AM - 4:15 AM	0 0 0 0		0 0 0 0	0 0 0 0	0 0 0
4:15 AM - 4:30 AM	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0 0	0 0 0
4:30 AM - 4:45 AM	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0 0	0 0 0
4:45 AM - 5:00 AM	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0 0	0 0 0
5:00 AM - 5:15 AM	0 0 0 0	0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0
5:15 AM - 5:30 AM	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0 0	0 0 0
5:30 AM - 5:45 AM	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0 0	0 0 0
5:45 AM - 6:00 AM	0 0 0 0	0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0
Total	0 103 0 0 103	0 0 0 0 0	0 110 0 0 110	0 0 0 0 0	213 0 213
One Hour Volume					
6:00 AM - 7:00 AM	0 2 0 0 2 0.5	0 0 0 0 0	0 22 0 0 22 0.7	0 0 0 0 0	24 0 24
6:15 AM - 7:15 AM	0 7 0 0 7 0.3	0 0 0 0	0 24 0 0 24 0.8		31 0 31
6:30 AM - 7:30 AM	0 10 0 0 10 0.4		0 26 0 0 26 0.8		36 0 36
6:45 AM - 7:45 AM	0 14 0 0 14 0.6		0 24 0 0 24 0.8	0 0 0 0 0	38 0 38
7:00 AM - 8:00 AM	0 24 0 0 24 0.6		0 24 0 0 24 0.8	0 0 0 0 0	48 0 48
7:15 AM - 8:15 AM	0 21 0 0 21 0.5	<del></del>	0 26 0 0 26 0.8	0 0 0 0 0	47 0 47
7:30 AM - 8:30 AM	0 25 0 0 25 0.6		0 31 0 0 31 0.7	0 0 0 0	56 0 56
7:45 AM - 8:45 AM	0 26 0 0 26 0.7		0 29 0 0 29 0.7	0 0 0 0 0	55 0 55
8:00 AM - 9:00 AM	0 19 0 0 19 0.6		0 30 0 0 30 0.7	0 0 0 0 0	49 0 49
4:00 PM - 5:00 PM	0 27 0 0 27 0.6		0 19 0 0 19 0.7	0 0 0 0 0	46 0 46
4:15 PM - 5:15 PM	0 37 0 0 37 0.8		0 22 0 0 22 0.8	0 0 0 0	59 0 59
4:30 PM - 5:30 PM	0 29 0 0 29 0.6		0 17 0 0 17 0.6	0 0 0 0 0	46 0 46
4:45 PM - 5:45 PM	0 24 0 0 24 0.5		0 15 0 0 15 0.5	0 0 0 0 0	39 0 39
5:00 PM - 6:00 PM	0 22 0 0 22 0.5		0 11 0 0 11 0.5	0 0 0 0 0	33 0 33
5:15 PM - 6:15 PM	0 14 0 0 14 0.9		0 6 0 0 6 0.5	0 0 0 0 0	20 0 20
5:30 PM - 6:30 PM	0 13 0 0 13 0.8		0 4 0 0 4 0.3	0 0 0 0 0	17 0 17
5:45 PM - 6:45 PM	0 13 0 0 13 0.8		0 7 0 0 7 0.6	0 0 0 0 0	20 0 20
6:00 PM - 7:00 PM	0 9 0 0 9 0.6		0 4 0 0 4 0.3	0 0 0 0 0	13 0 13
			·		

## Tysons, Virginia

#### **Turning Movement Count - Bicycles**

PROJECT: The Lamb Center TIS

DATE: 4/12/2022

DUTHBOUND ROAD: Campbell Drive

P8779

DAY: Tuesday

DAY: Tuesday

DAY: Tuesday

DAY: Tuesday

INTERSECTION: Campbell Dr. & Site Entrace - South

LOCATION: City of Fairfax, VA COUNTED BY: Agan **EASTBOUND ROAD:** 0 INPUTED BY: agan Northbound Eastbound North East Southbound Westbound & Total Campbell Drive Time Campbell Drive Site Entrance - South Left Total Right Thru Left Total Left Total Right Thru South West Period Right Thru Left Total Right Thru One Hour Volumes 6:00 AM - 7:00 AM ī ı - 7:15 AM 6:15 AM I 6:30 AM - 7:30 AM ī ı 6:45 AM - 7:45 AM - 8:00 AM 7:00 AM ī ı 7:15 AM - 8:15 AM 7:30 AM - 8:30 AM 7:45 AM - 8:45 AM ō ī ı 8:00 AM - 9:00 AM ō - 5:00 PM 4:00 PM 4:15 PM - 5:15 PM 4:30 PM - 5:30 PM 4:45 PM - 5:45 PM ı ī 5:00 PM - 6:00 PM ī ı ī 5:15 PM - 6:15 PM ī 5:30 PM - 6:30 PM ō 5:45 PM - 6:45 PM - 7:00 PM ī ō 6:00 PM

## Tysons, Virginia

## **Pedestrian Volume Survey**

								Jui Vey						
INTE	PA JOB NO: ERSECTION: LOCATION: DATE: DAY: WEATHER: UNTED BY:	The Lamb p8779 Campbell City of Fa 4/12/2022 Tuesday clear Agan agan	Dr. & Si irfax,VA	te Entr	South			o <u>₹</u>	7	npbell Di	4 1 3	Site Entrance - Sou	No	orth
	Time	1					м	ovement				-		-
	Period	<del></del>	2	3	4	5	6	7	8	1 + 2	3 + 4	5 + 6	7+ 8	Total
	r Volumes	<u> </u>			•			<u> </u>						
6:00 AM	- 7:00 AM	4	0	3	0	12	2	2	0	4	3	14	2	23
6:15 AM	- 7:15 AM	5	0	4	0	13	2	6	0	5	4	15	6	30
6:30 AM	- 7:30 AM	5	0	4	0	13	l	8	0	5	4	14	8	31
6:45 AM	- 7:45 AM	4	0	4	0	12	ī	10	0	4	4	13	10	31
7:00 AM	- 8:00 AM	2	ı	5	<u>l</u>	5	ī	13	ī	3	6	6	14	29
7:15 AM	- 8:15 AM	1	1	4		3	0	10	2	2	5	3	12	22
7:30 AM	- 8:30 AM	1	I	4	2	4	0	9	2	2	6	4	11	23
7:45 AM	- 8:45 AM	i	ı	5	2	2	0	9	3	2	7	2	12	23
8:00 AM	- 9:00 AM	0	0	2	Ī	2	0	5	2	0	3	2	7	12
4:00 PM	- 5:00 PM	0	0		1	6	4	0	0	0	2	10	0	12
4:15 PM	- 5:15 PM	0	0	!		4	2	0	0	0	2	6	0	8
4:30 PM	- 5:30 PM	0	0	1	ı	5	I	0	0	0	2	6	0	8
4:45 PM	- 5:45 PM	0	0	1	ı	4	1	0	0	0	2	5	0	7
5:00 PM	- 6:00 PM	0	0	Ī	0	2	1	0	Ī	0	l	3		5
5:15 PM	- 6:15 PM	0	0	1	0	2	2	0	1	0	1_	4	1	6
5:30 PM	- 6:30 PM	0	0	0	0	0	2	<u> </u>	I	0	0	2	2	4
5:45 PM	- 6:45 PM	0	0	0	0	1	2	l	1	0	0	3	2	5
6:00 PM	- 7:00 PM	0	0	I	0	4	5	ı	0	0	l l	9	I	

#### Tysons, Virginia

### **Turning Movement Count - Total Vehicles**

PROJECT: The Lamb Center TIS W+A JOB NO: p8779

5:45 PM - 6:45 PM

6:00 PM - 7:00 PM

 DATE: 4/12/2022 DAY: Tuesday

SOUTHBOUND ROAD: Campbell Drive NORTHBOUND ROAD: Campbell Drive

INTERSECTION: Fairfax Blvd. & Campbell Dr.

WEATHER: clear

WESTBOUND ROAD: Fairfax Boulevard

0.9

2505 2547

0.7 

> 0.7  $\overline{\Pi}$

LOCATION: City of Fairfax, VA COUNTED BY: Majda & Ramiz EASTBOUND ROAD: Fairfax Boulevard INPUTED BY: agan North East Southbound Westbound & Campbell Drive Fairfax Boulevard Campbell Drive Fairfax Boulevard Time Total PHF Right Thru Left-Turn Total PHF Right Left-Turn Total PHF West Right Thru Left-Turn Total PHF Right Thru Left-Turn Thru Period 15 Minute Volumes 6:00 AM - 6:15 AM RA 6:15 AM - 6:30 AM 6:30 AM - 6:45 AM ō 6:45 AM - 7:00 AM ō 7:00 AM - 7:15 AM o 7:15 AM - 7:30 AM 7:30 AM - 7:45 AM ā T TI 7:45 AM - 8:00 AM ō a ı 8:00 AM - 8:15 AM ï ō ī ō 8:15 AM - 8:30 AM 8:30 AM - 8:45 AM 8:45 AM - 9:00 AM 4:00 PM - 4:15 PM 4:15 PM - 4:30 PM 4:30 PM - 4:45 PM 4:45 PM - 5:00 PM 5:00 PM - 5:15 PM 5:15 PM - 5:30 PM n П ī 5:30 PM - 5:45 PM 5:45 PM - 6:00 PM -5 ī ō 6:00 PM - 6:15 PM ō 6:15 PM - 6:30 PM Õ ō ō ō 6:30 PM - 6:45 PM Ti 6:45 PM - 7:00 PM 13842 | 14081 15 6553 Total One Hour Volumes 0.7 0.5 6:00 AM - 7:00 AM 0.5 6:15 AM - 7:15 AM 0.3 0.8 0.8 0.5 ı 6:30 AM - 7:30 AM 0.4 0.9 ı 2 1094 0.9 0.6 0.9 0.5 6:45 AM - 7:45 AM 7:00 AM - 8:00 AM 0.6 0.8 0.4 3 1194 1 1206 0.9 0.8 0.5 8 1292 0.9 0.6 ō 7:15 AM - 8:15 AM 11 1345 0 1373 0.9 0.5 0.9 7:30 AM - 8:30 AM 0.6 0,8 0.9 7:45 AM - 8:45 AM 0.7 0 1339 0.9 0.6 0.9 O 0.5 9 1313 8:00 AM - 9:00 AM 0.9 n ō 0.7 7 1100 2 1116 0.9 4:00 PM - 5:00 PM 0.9 ō 0.7 8 1087 1 1106 0.9 0.9 4:15 PM - 5:15 PM 4 1157 0.8 7 1139 4:30 PM - 5:30 PM 0.8 8 1121 0.9 0.6 12 1617 0.9 4:45 PM - 5:45 PM 8 1150 0.8 9 1128 5:00 PM - 6:00 PM ī 0.5 0.9 5:15 PM - 6:15 PM 0.7 0.9 2721 2771 13 1047 0.8 5:30 PM - 6:30 PM ō 0.6 6 1020 0.9 

0.7

8 0.7

ō 

0.9 

0.9 

## Tysons, Virginia

## **Turning Movement Count - Bicycles**

PROJECT: The Lamb Center TIS DATE: 4/12/2022 DUTHBOUND ROAD: Campbell Drive
W+A JOB NO: p8779 DAY: Tuesday DRTHBOUND ROAD: Campbell Drive
INTERSECTION: Fairfax Blvd. & Campbell WEATHER: clear WESTBOUND ROAD: Fairfax Boulevard
LOCATION: City of Fairfax,VA COUNTED BY: Majda EASTBOUND ROAD: Fairfax Boulevard

LOCATION	: City o	f Fairfax	,VA	COU	NTED	BY:	Majda			EAST	BOUN	ID RC	AD:	Fairfax	Boule	/ard			
				INP	UTED	BY: a	agan												
		Southb	ound			Westb	ound			North	oound			Eastbo	ound		North	East	
Time		Campbel	II Driv	e	Fa	irfax Bo	oulevar	ď	C	ampbe	II Drive	2	Fa	irfax Bo	oulevai	·d	&	&	Total
Period	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	South	West	
One Hour Volumes	;																		
6:00 AM - 7:00 AM	0	0	0	0	0	6	0	6	0	0	0	0	0	2	0	2	0	8	8
6:15 AM - 7:15 AM	0	0	0	0	0	6	0	6	0	0	0	0	0_	2		3	0	9	9
6:30 AM - 7:30 AM	0	0	0	0	0	5	0	5	0	0	0	0	0	2	ı	3	0	8	8
6:45 AM - 7:45 AM	0	0	0	0	0	2	0	2	0	0	0	0	0	0		1	0	3	3
7:00 AM - 8:00 AM	0	0	0	0	1	0	0	- 1	0	0	0	0	0	0			0	2	2
7:15 AM - 8:15 AM	0	0	0	0	ī	0	0	ı	0	0	0	0	0	0	0	0	0	<u> </u>	1
7:30 AM - 8:30 AM	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	1	1
7:45 AM - 8:45 AM	0	0	0	0	I	0	0	1	0	0	0	0	0	0			0	2	2
8:00 AM - 9:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0		1
4:00 PM - 5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0		0		0	<u> </u>	1
4:15 PM - 5:15 PM	0	0	0	0	0	ī	0	ī	0	0	0	0	0	1	0		0	2	2
4:30 PM - 5:30 PM	0	0	i		0	i	0	I	0	0	0	0	0	0	0	0		1	2
4:45 PM - 5:45 PM	0	0	ı	ī	0	2	0	2	0	0	0	0	0	3	0	3		5	6
5:00 PM - 6:00 PM	0	0	1	ī	0	2	0	2	0	0	0	0	0	3	0	3	<u> </u>	5	6
5:15 PM - 6:15 PM	0	0	1	1	0	ı	0	1	0	0	0	0	0	3	0	3	1	4	5
5:30 PM - 6:30 PM	0	0	0	0	0	I	0	Ī	0	0	0	0	0	3	0	3	0	4	4
5:45 PM - 6:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:00 PM - 7:00 PM	1	0	0	ī	0	0	0	0	0	0	0	0	0	2	0	2	<u> </u>	2	3

## Tysons, Virginia

## **Pedestrian Volume Survey**

**PROJECT:** The Lamb Center TIS

W + A JOB NO: p8779

INTERSECTION: Fairfax Blvd. & Campbell Dr.

LOCATION: City of Fairfax, VA

**DATE:** 4/12/2022 **DAY:** Tuesday

WEATHER: clear
COUNTED BY: Ramiz

INPUTED BY: agan

Campbell Drive

Fairfax

Boulevard

Campbell Drive

Campbell Drive

1	
North	

	Time	Movement													
	Period	ı	2	3	4	5	6	7	8	1 + 2	3 + 4	5 + 6	7+ 8	Total	
6:00 AM	- 7:00 AM	7	2	0	1	ı	0	2	0	9	I	1	2	13	
6:15 AM	- 7:15 AM	7	<u>I</u>	0	ı	1	0	5	0	8	I	ı	5	15	
6:30 AM	- 7:30 AM	7	0	0	ı	- 1	1	5	0	7	- 1	2	5	15	
6:45 AM	- 7:45 AM	4	0	0	ī	0	1	3	0	4	ı		3	9	
7:00 AM	- 8:00 AM	1	0	0	0	0	Ī	4	0	ı	0	<u> </u>	4	6	
7:15 AM	- 8:15 AM	0	0	0	0	0	2	1	1	0	0	2	2	4	
7:30 AM	- 8:30 AM	2	0	0	0	0	1	I	1	2	0	1	2	5	
7:45 AM	- 8:45 AM	5	0	0	0	0	ı	1	2	5	0		3	9	
8:00 AM	- 9:00 AM	6	0	2	0	0	ı	0	2	6	2		2	11	
4:00 PM	- 5:00 PM	7	7	2	0	2	2	0	0	14	2	4	0	20	
4:15 PM	- 5:15 PM	5	6	l	0	2	2	0	0	11	1	4	0	16	
4:30 PM	- 5:30 PM	5	4	0	0	2	2	0	0	9	0	4	0	13	
4:45 PM	- 5:45 PM	4	3	0	0	2	2	0	0	7	0	4	0		
5:00 PM	- 6:00 PM	3	ı	0	0	2	ī	0	0	4	0	3	0		
5:15 PM	- 6:15 PM	3	3	0	0	2	Ī	0	1	6	0	3	1	10	
5:30 PM	- 6:30 PM	l ı	3	i	1	2	l	0	1	4	2	3	1	10	
5:45 PM	- 6:45 PM	2	3	<u>I</u>		3	ı	0	2	5	2	4	2	13	
6:00 PM	- 7:00 PM	5	5	2		2	ı	0	2	10	3	3	2	18	

Tysons, Virginia

#### **Turning Movement Count - Total Vehicles**

PROJECT: The Lamb Center TIS

DATE: 4/12/2022

SOUTHBOUND ROAD: Site Entrance

WHA JOB NO: p8779

DAY: Tuesday

NORTHBOUND ROAD: 0

INTERSECTION: Fairfax Bivd. & Site Entr.

WEATHER: clear

WESTBOUND ROAD: Fairfax Boulevard

COUNTED BY: Agan

EASTBOUND ROAD: Fairfax Boulevard

LOCATION:	City o	f Fairfa	c,VA							OUNTE! INPUTE!					Į	EASTBO	DUND F	OAD:	Fai	irfax B	oulevard				
Time				nbound ntrance			Westbound Fairfax Boulevard						Northbound 0					Eastbound Fairfax Boulevard					North &	East &	Total
Period	Diahe	Thru		U-Turn	Total		Dight			U-Turn		DUE	Right	Theu	Left LTuri	Total	DLIE Dia				urn Tot	4 DLE	4		lotai
15 Minute Volumes		raru	Fest	O-Turn	Total	FEE	right	Thru	Leit	U-Turn	1012	( FEEF	right	Inru	Leit-Turi	1 TOTALL	rnr į Ng	111 1111	<u> </u>	Leit - I	um lot	aj rnr	Souch	**est	
6:00 AM - 6:15 AM	· -		0	<u>_</u>	0		0	91	0	0	91		0	0	0 0	0	—т-	0 (		0	0 (		0	91	91
6:15 AM - 6:30 AM	1 0		<del>-</del> ö-	0	<del>-</del>	<u> </u>	0	107	0	- 0	107	-							<u>,</u>	0	0 0		1 0	107	107
6:30 AM - 6:45 AM	0	<del>-</del> -		0		├──	- 0	116			116		-	- <del>-</del> -					, )	0	0 0		0	116	116
6:45 AM - 7:00 AM	1 0				-		- 6	195	- 0	0	195	<del> </del>	<del>-</del>	-0	0 0				<u>,</u> —-	0	0 0		1 0	195	195
7:00 AM - 7:15 AM	1 +	<del>-</del>	<del>-</del> ö-	<del></del>	-		<del>                                     </del>	168			168		<del>                                     </del>	<del>-</del> 0	0 0				<u>,</u>	0	0 0		<del>                                     </del>	168	168
7:15 AM - 7:30 AM	1 6		<del>-</del> -	0	- 0	<del></del>	<del>  -</del>	193	<del>-</del>	<del></del>	193		<del>-</del>	<del>-</del> -	0 0				<u> </u>	0	<del></del>		1 0	193	193
7:30 AM - 7:45 AM	<del>                                     </del>	<del>-</del>		<u>-</u>			<del>                                     </del>	209	<del>-</del>	-	209		10	0	0 0				<u></u>	<del>-</del>	0 0		1 0	209	209
7:45 AM - 8:00 AM	0		<del></del> -	0	-	<del></del>	<del>  </del>	281	<del>- 0</del>		281			<del>-</del> -	0 0				<u> </u>	0	0 0		<del>  -</del> -	281	281
8:00 AM - 8:15 AM	1 0	<del></del>	<del></del> -	<del></del>	<del>-</del>		1 0	216	-	0	216	<del>- ,</del>	<del>                                     </del>	<del>-</del> -	0 0			0 (		0	0 (		0	216	216
8:15 AM - 8:30 AM	1 0		<del></del>		-	├		232		<del></del>	232		<del>                                     </del>	<del>-</del>	0 0				· ·	<del>-</del>	0 0		10	232	232
8:30 AM - 8:45 AM	<del>"</del>		<del>-</del> ö-	0				239			239	<del> </del>	<del>  </del>	<del></del>	0 0			<del>-</del>		0	0 0		-	239	239
8:45 AM - 9:00 AM	<del>                                     </del>	<del></del> 6-			- 6	<del> </del>	0	266	0	0	266		<del>   </del>	<del>-</del> -	<del>-0-0</del>			0		0	0 0		<del>  -</del>	266	266
4:00 PM - 4:15 PM			<del>- ö</del> -		-	<del> </del>	<del>                                     </del>	408		0	409	<del> </del>		<del></del>	0 0			0 (		- <del>-</del>	0 0		<del> </del>	409	409
4:15 PM - 4:30 PM	÷	<del></del>		- 0	-	<del> </del>	i	451		0	451	<del>                                     </del>	- <del> </del> -	-	0 0			0 (	<u> </u>	0	0 0		<del>  </del>	451	451
4:30 PM - 4:45 PM	├─ <del>`</del>		<del></del> -	<del>_</del> _	Ť		0	437			437			<del></del> -	0 0			0 (		0	0 0		<del> </del>	437	438
4:45 PM - 5:00 PM	<del>;</del>		<del>-</del> ö-	<del>0</del> _	<u>.</u>		<del> </del>	457		- 0	458	_	-		0 0				<u></u> -	<u> </u>	0 0		0	458	458
5:00 PM - 5:15 PM	1 0		<del></del>		0		0	407		0	407		0	<del>-</del>	0 0			0	<u> </u>	0	0 (	<del></del>	<del>                                     </del>	407	407
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5:30 PM - 5:45 PM	-		-	0	0	<del>                                     </del>		405		0	405	<del> </del>	1 0	<del></del>	0 0			0		0	0 0		10	405	405
5:45 PM - 6:00 PM	1 0		<del></del> -	<del></del>	-0	<del>                                     </del>		444			444			<del></del> -	0 0				5	0	0 0		<del>                                     </del>	444	444
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6:30 PM - 6:45 PM			<del></del> -			<del> </del>	0	387			387			<del>-</del> -	0 0	0		0 (		0	0 (	5	1-0	387	387
6:45 PM - 7:00 PM	0	<del></del>	<del>-</del>	<del></del>	<del>-</del>	<del> </del>	Ö	333	0	0	333	_	-	0	0 0	0		0 (	)	0	0 (	5	0	333	333
Total	Ť	0	ō	- 0	Ť		2	7311	ō	0	7313		1	0	0 0	0		0 (	,	0	0 (			7313	7314
One Hour Volumes	<u> </u>	<u>`</u>																							
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6:15 AM - 7:15 AM	1				-0		-	586	<u>`</u>	0	586	0.8			0 0	0		0 (	<u> </u>	0	0 (	; <del> </del>	0	586	586
6:30 AM - 7:30 AM	1 0		0	0	<del>-</del>	<del> </del>	0	672	0	0	672	0.9	0	0	0 0	0		0 (	,	0	0 (	i <del> </del>	0	672	672
6:45 AM - 7:45 AM	1 0		0	0	0		0	765	0		765	0.9	0	0	0 0	0		0 (	<u> </u>	0	0 (	1	0	765	765
7:00 AM - 8:00 AM	10	<del></del>		0	0		0	851	-	0	851	0.8	0	0	0 0	0		0 (	)	0	0 (	5	0	851	851
7:15 AM - 8:15 AM			0				0	899	0	0	899	0.8	0	0	0 0	0		0 (	)	0	0 (	j	0	899	899
7:30 AM - 8:30 AM	1 -	<del></del>	0	0	<del>-</del>		0	938		0	938	0.8	0	<del></del>	0 0	0		0 (	<del></del> -	0	0 (	<u> </u>	0	938	938
7:45 AM - 8:45 AM	0	0	0	0	0		0	968	0	0	968	0.9	0	0	0 0	0		0 0	)	0	0 (	)	9	968	968
8:00 AM - 9:00 AM	0	0	0	0	0		0	953	0	0	953	0.9	0	0	0 0	0		0 (	)	0	0 (		0	953	953
4:00 PM - 5:00 PM		0	0	0	Ė	0.3	2	1753	0	0	1755		θ	0	0 0	0		0 (	}	0	0 (	)	I	1755	1756
4:15 PM - 5:15 PM		0	0	0	- 1	0.3	Ī	1752	0	0	1753		0	0	0 0	0		0 (	)	0	0 (		I	1753	1754
4:30 PM - 5:30 PM	Ι÷	0		0	÷	0.3	<del>l i</del>	1746	0		1747	<b>—</b>	0	0	0 0	0		0 (	<del></del>	0	0 (	7	1	1747	1748
4:45 PM - 5:45 PM	<del>                                     </del>	<del></del>	-	0	0	<del>                                     </del>	i	1714		0	1715	0.9	0	0	0 0	0		0 (	)	0	0 (	7	0	1715	1715
5:00 PM - 6:00 PM	<del>  </del>	<del></del>	<del></del> -	0	-	<del> </del>	0	1701			1701		0	0	0 0	- 0		0 (	<del></del> -	0	0 (	7	0	1701	1701
5:15 PM - 6:15 PM	1 0		<del>-</del>	0	<del>-</del>	<del>                                     </del>	0	1712		0	1712	<del>                                     </del>	0	0	0 0	0		0 (	)	0	0 (	)	0	1712	1712
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5:45 PM - 6:45 PM	0	0		- 0	<del>-</del>		0	1655		0	1655	0.9	0	<del>-</del>	0 0			0	)	0	0 (	7	0	1655	1655
6:00 PM - 7:00 PM	1 0		<del>-</del> -		-0	<del> </del>	-	1544			1544	0.9	-	<del>-</del> -	0 0				5	0	0 (		0	1544	1544
5.55 FFT - 7.00 FFT					<u>~</u>	<u> </u>	L	1377				_ <del></del> _	L <u>-</u>	<u>-</u> _									<u> </u>		

## Tysons, Virginia

## **Turning Movement Count - Bicycles**

**PROJECT:** The Lamb Center TIS

**DATE:** 4/12/2022

**DUTHBOUND ROAD:** Site Entrance

W+A JOB NO: p8779

DAY: Tuesday

**ORTHBOUND ROAD:** 0

LOCATION: City of Fairfax, VA COUNTED BY: Agan

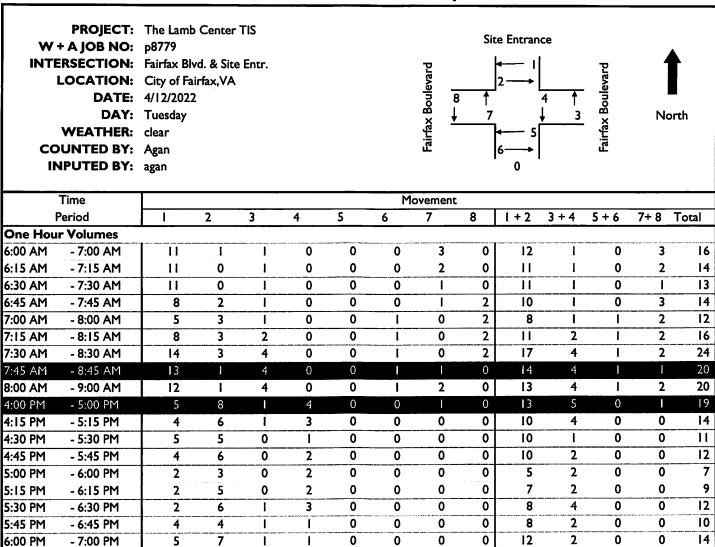
INTERSECTION: Fairfax Blvd. & Site Entr. WEATHER: clear

**WESTBOUND ROAD:** Fairfax Boulevard EASTBOUND ROAD: Fairfax Boulevard

					INP	UTED	BY: a	agan												
			Southb	ound		Westbound					North	ound			Eastbo	ound	North	East		
	Time		Site En	trance		Fairfax Boulevard					0			Fa	irfax Bo	oulevai	&	&	Total	
P	Period	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	South	West	
One Hou	ur Volumes																			
6:00 AM	- 7:00 AM	0	0	0	0	0	4	0	4	0	0	0	0	0	2	0	2	0	6	6
6:15 AM	- 7:15 AM	0	0	0	0	0	4	0	4	0	0	0	0	0	2	0	2	0	6	6
6:30 AM	- 7:30 AM	0	0	0	0	0	3	0	3	0	0	0	0	0	2	0	2	0	5	5
6:45 AM	- 7:45 AM	0	0	0	0	0	ı	0	-	0	0	0	0	0	0	0	0	0	1	- 1
7:00 AM	- 8:00 AM	0	0	0	0	0	Ī	0	-	0	0	0	0	0	0	0	0	0	1 -	- 1
7:15 AM	- 8:15 AM	0	0	0	0	0	I	0		0	0	0	0	0	0	0	0	0	1	1
7:30 AM	- 8:30 AM	0	0	0	0	0	Ī	0	_	0	0	0	0	0	0	0	0	0	1	1
7:45 AM	- 8:45 AM	0	0	0	0	0	1	0	- 1	0	0	0	0	0	0	0	0	0	1	I
8:00 AM	- 9:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:00 PM	- 5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	- 1	0	I	0	1	l
4:15 PM	- 5:15 PM	0	0	0	0	0	- 1	0	1	0	0	0	0	0		0		0	2	2
4:30 PM	- 5:30 PM	0	0	0	0	0	1	0	1	0	0	0	0	0		0	1	0	2	2
4:45 PM	- 5:45 PM	0	0	0	0	0	1	0	ı	0	0	0	0	0	3	0	3	0	4	4
5:00 PM	- 6:00 PM	0	0	0	0	0	1	0	1	0	0	0	0	0	3	0	3	0	4	4
5:15 PM	- 6:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	0	3	3
5:30 PM	- 6:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	0	2	2
5:45 PM	- 6:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:00 PM	- 7:00 PM	0	0	0	0	0		0	1	0	0	0	0	0	2	0	2	0	3	3

## Tysons, Virginia

## **Pedestrian Volume Survey**



## APPENDIX C

**Existing Capacity Analysis Worksheets** 

	٠	-	*	F	•	<b>←</b>	*	4	<b>†</b>	1	1	ļ
Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	N.	ተተ <sub>ጉ</sub>			7	<b>ተ</b> ቀጉ			4			4
Traffic Volume (veh/h)	14	1354	10	18	19	915	21	1	0	2	4	2
Future Volume (Veh/h)	14	1354	10	18	19	915	21	1	0	2	4	2
Sign Control		Free				Free			Stop			Stop
Grade		0%				0%			0%			0%
Peak Hour Factor	0.91	0.91	0.91	0.88	0.88	0.88	0.88	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	15	1488	11	0	22	1040	24	1	0	2	5	2
Pedestrians		3							1			5
Lane Width (ft)		12.0							12.0			12.0
Walking Speed (ft/s)		4.0							4.0			4.0
Percent Blockage		0							0			0
Right turn flare (veh)												
Median type		None			Andrews	None						
Median storage veh)												
Upstream signal (ft)		993				589						
pX, platoon unblocked	0.96			0.00	0.92			0.94	0.94	0.92	0.94	0.94
vC, conflicting volume	1069			0	1500			1943	2638	502	1629	2631
vC1, stage 1 conf vol				Nation?								
vC2, stage 2 conf vol												
vCu, unblocked vol	912			0	1228			1463	2202	141	1128	2195
tC, single (s)	4.1			0.0	4.1			7.5	6.5	6.9	7.5	6.5
tC, 2 stage (s)												
tF (s)	2.2			0.0	2.2			3.5	4.0	3.3	3.5	4.0
p0 queue free %	98			0.0	96			99	100	100	96	95
cM capacity (veh/h)	707			0	516			74	39	808	141	39
25 35 5 5								35.1114.11		00000000	171	00
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	15	595	595	309	22	416	416	232	3	31		
Volume Left	15	0	0	0	22	0	0	0	1	5		
Volume Right	0	0	0	11	0	0	0	24	2	24		
cSH	707	1700	1700	1700	516	1700	1700	1700	188	265		
Volume to Capacity	0.02	0.35	0.35	0.18	0.04	0.24	0.24	0.14	0.02	0.12		
Queue Length 95th (ft)	2	0	0	0	3	0	0	0	1	10		
Control Delay (s)	10.2	0.0	0.0	0.0	12.3	0.0	0.0	0.0	24.4	20.4		
Lane LOS	В				В				C	C		
Approach Delay (s)	0.1				0.2				24.4	20.4		
Approach LOS									C	C		
Intersection Summary	Letter.											
Average Delay			0.4									
Intersection Capacity Utilizati	ion		41.7%	IC	U Level	of Service	)		Α			
Analysis Period (min)			15									



Mayamant	CDD
Movement	SBR
Lan Configurations	00
Traffic Volume (veh/h)	20
Future Volume (Veh/h)	20
Sign Control	
Grade	
Peak Hour Factor	0.85
Hourly flow rate (vph)	24
Pedestrians	
Lane Width (ft)	
Walking Speed (ft/s)	
Percent Blockage	
Right turn flare (veh)	
Median type	
Median storage veh)	
Upstream signal (ft)	
pX, platoon unblocked	0.96
vC, conflicting volume	367
vC1, stage 1 conf vol	
vC2, stage 2 conf vol	
vCu, unblocked vol	178
tC, single (s)	6.9
tC, 2 stage (s)	
tF (s)	3.3
p0 queue free %	97
cM capacity (veh/h)	793
Direction, Lane#	
Direction, Lane #	

	-	*	<b>←</b>	<b>↓</b>	1
Lane Group	EBT	EBR	WBT	SBT	SBR
Lane Group Flow (vph)	1343	317	725	403	307
v/c Ratio	0.55	0.30	0.31	0.48	0.22
Control Delay	18.0	14.3	3.9	64.5	0.4
Queue Delay	0.0	0.0	0.2	0.0	0.0
Total Delay	18.0	14.3	4.1	64.5	0.4
Queue Length 50th (ft)	484	169	47	244	0
Queue Length 95th (ft)	615	256	53	285	0
Internal Link Dist (ft)	509		176	192	
Turn Bay Length (ft)					
Base Capacity (vph)	2446	1063	2354	1014	1411
Starvation Cap Reductn	0	0	850	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.55	0.30	0.48	0.40	0.22
Intersection Summary					

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44	7"		<b>^</b>						414	77
Traffic Volume (vph)	0	1262	298	0	696	0	0	0	0	13	354	279
Future Volume (vph)	0	1262	298	0	696	0	0	0	0	13	354	279
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		3.5	3.5		3.5						4.8	4.0
Lane Util. Factor		0.95	1.00		0.95						0.95	1.00
Frpb, ped/bikes		1.00	1.00		1.00						1.00	0.99
Flpb, ped/bikes		1.00	1.00		1.00						1.00	1.00
Frt		1.00	0.85		1.00						1.00	0.85
Flt Protected		1.00	1.00		1.00						1.00	1.00
Satd. Flow (prot)		3505	1524		3374						3180	1411
Flt Permitted		1.00	1.00		1.00						1.00	1.00
Satd. Flow (perm)		3505	1524		3374						3180	1411
Peak-hour factor, PHF	0.94	0.94	0.94	0.96	0.96	0.96	0.92	0.92	0.92	0.91	0.91	0.91
Adj. Flow (vph)	0	1343	317	0	725	0	0	0	0	14	389	307
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	1343	317	0	725	0	0	0	0	0	403	307
Confl. Peds. (#/hr)	2		3	3		2	2					2
Confl. Bikes (#/hr)												1
Heavy Vehicles (%)	0%	3%	6%	0%	7%	0%	0%	0%	0%	50%	12%	13%
Turn Type		NA	Prot		NA					Split	NA	Free
Protected Phases		2	2		2					4	4	
Permitted Phases								N CONTRACTOR OF THE PARTY				Free
Actuated Green, G (s)		151.9	151.9		151.9						55.5	220.0
Effective Green, g (s)		153.5	153.5		153.5						58.2	220.0
Actuated g/C Ratio		0.70	0.70		0.70						0.26	1.00
Clearance Time (s)		5.1	5.1		5.1						7.5	3.076
Vehicle Extension (s)		3.0	3.0		3.0						3.0	
Lane Grp Cap (vph)		2445	1063		2354						841	1411
v/s Ratio Prot		c0.38	0.21		0.21						c0.13	
v/s Ratio Perm		00.00	0.21		0.21						00110	0.22
v/c Ratio		0.55	0.30		0.31						0.48	0.22
Uniform Delay, d1		16.3	12.7		12.8						68.1	0.0
Progression Factor		1.00	1.00		0.26						0.93	1.00
Incremental Delay, d2		0.9	0.7		0.3						0.4	0.4
Delay (s)		17.2	13.4		3.7						63.6	0.4
Level of Service		В	В		A						E	A
Approach Delay (s)		16.5			3.7			0.0			36.2	
Approach LOS		В			A			A			D	
Intersection Summary											4 65	
HCM 2000 Control Delay			18.0	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capacity	ratio		0.53		0111 2000	2010101						
Actuated Cycle Length (s)	Tullo		220.0	Si	um of lost	t time (s)			8.3			A STATE OF THE PARTY OF
Intersection Capacity Utilization			61.6%			of Service	23		В			
Analysis Period (min)	2519 N		15	10	3 23 101 1				1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			-
c Critical Lane Group												
5 Shidai Earlo Group	- New York			The second second								THE REAL PROPERTY.

Existing AM W+A

	-	•	*	<b>†</b>	1
Lane Group	EBT	WBT	WBR	NBT	NBR
Lane Group Flow (vph)	1411	688	124	739	316
v/c Ratio	0.56	0.29	0.12	0.82	0.20
Control Delay	2.7	12.2	11.5	80.1	0.3
Queue Delay	1.0	0.0	0.0	0.0	0.1
Total Delay	3.7	12.2	11.5	80.1	0.4
Queue Length 50th (ft)	40	153	51	541	0
Queue Length 95th (ft)	45	181	77	591	0
Internal Link Dist (ft)	176	1252		171	
Turn Bay Length (ft)			200		
Base Capacity (vph)	2519	2354	1009	1087	1561
Starvation Cap Reductn	22	0	0	0	0
Spillback Cap Reductn	767	0	0	0	331
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.81	0.29	0.12	0.68	0.26
Intersection Summary					

	۶	-	*	1	<b>—</b>	4	4	†	~	1	ţ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	777	<b>十</b> 个			<b>^</b>	74		414	7			
Traffic Volume (vph)	0	1270	0	0	654	118	42	631	288	0	0	0
Future Volume (vph)	0	1270	0	0	654	118	42	631	288	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		3.5			3.5	3.5		4.8	4.0			
Lane Util. Factor		0.95			0.95	1.00		0.95	1.00			
Frpb, ped/bikes		1.00			1.00	0.98		1.00	0.99			
Flpb, ped/bikes		1.00			1.00	1.00		1.00	1.00			
Frt		1.00			1.00	0.85		1.00	0.85			
Flt Protected		1.00			1.00	1.00		1.00	1.00			
Satd. Flow (prot)		3610			3374	1446		3407	1561			
Flt Permitted		1.00			1.00	1.00		1.00	1.00			
Satd. Flow (perm)		3610			3374	1446		3407	1561			
Peak-hour factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.91	0.91	0.91	0.92	0.92	0.92
Adj. Flow (vph)	0	1411	0	0	688	124	46	693	316	0	0	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	1411	0	0	688	124	0	739	316	0	0	0
Confl. Peds. (#/hr)	1		1	1		1			7			
Confl. Bikes (#/hr)									1			
Heavy Vehicles (%)	0%	0%	0%	0%	7%	10%	15%	5%	2%	0%	0%	0%
Turn Type		NA			NA	Perm	Split	NA	Free			
Protected Phases		2			2		4	4				
Permitted Phases						2			Free			
Actuated Green, G (s)		151.9			151.9	151.9		55.5	220.0			
Effective Green, g (s)		153.5			153.5	153.5		58.2	220.0			
Actuated g/C Ratio		0.70			0.70	0.70		0.26	1.00			
Clearance Time (s)		5.1			5.1	5.1		7.5				
Vehicle Extension (s)		3.0			3.0	3.0		3.0				
Lane Grp Cap (vph)		2518			2354	1008		901	1561			
v/s Ratio Prot		c0.39	1		0.20			c0.22				
v/s Ratio Perm						0.09			0.20			
v/c Ratio		0.56			0.29	0.12		0.82	0.20			
Uniform Delay, d1		16.5			12.6	11.0		76.0	0.0			
Progression Factor		0.11			0.90	0.94		0.95	1.00			
Incremental Delay, d2		0.8			0.3	0.2		6.0	0.3			
Delay (s)		2.6			11.7	10.6		78.3	0.3			
Level of Service		Α			В	В		Е	Α			
Approach Delay (s)		2.6			11.5			54.9			0.0	
Approach LOS		Α			В			D			Α	
Intersection Summary								N.				
HCM 2000 Control Delay			21.7	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capacity	ratio		0.63									
Actuated Cycle Length (s)			220.0	S	um of los	t time (s)			8.3			
Intersection Capacity Utilization			61.6%			of Service			В			
Analysis Period (min)			15	THE REAL PROPERTY AND ADDRESS OF THE PARTY AND		and the second s						
c Critical Lane Group												

Existing AM W+A

	₫	۶	<b>→</b>	*	F	•	<b>—</b>	4	1	†	~	1
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		J.	ተተቡ			7	ተተጉ			4		
Traffic Volume (veh/h)	4	7	1139	7	56	40	1630	14	8	0	32	2
Future Volume (Veh/h)	4	7	1139	7	56	40	1630	14	8	0	32	2
Sign Control			Free				Free			Stop		2525
Grade			0%				0%			0%		
Peak Hour Factor	0.96	0.96	0.96	0.96	0.93	0.93	0.93	0.93	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	0	7	1186	7	0	43	1753	15	9	0	38	2
Pedestrians										4		
Lane Width (ft)							ASSESSMENT PROPERTY			12.0		
Walking Speed (ft/s)										4.0		
Percent Blockage										0	a tanàna dia mandri	
Right turn flare (veh)												
Median type			None				None					
Median storage veh)			Addition									
Upstream signal (ft)			999				583					
pX, platoon unblocked	0.00	0.89	Hallan		0.00	0.95			0.91	0.91	0.95	0.91
vC, conflicting volume	0.00	1777			0.00	1197			1914	3070	403	2303
vC1, stage 1 conf vol												2000
vC2, stage 2 conf vol												
vCu, unblocked vol	0	1432			0	1030			1307	2576	196	1733
tC, single (s)	0.0	4.1			0.0	4.1			7.5	6.5	6.9	7.5
tC, 2 stage (s)	0.0				0.0				7.0	0.0	0.0	7.0
tF (s)	0.0	2.2			0.0	2.2			3.5	4.0	3.3	3.5
p0 queue free %	0.0	98			0.0	93			90	100	95	96
cM capacity (veh/h)	0	414			0	636		Marine State	95	21	771	45
										N311/11:	771	73
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	7	474	474	244	43	701	701	366	47	38		SALE DOMEST
Volume Left	7	0	0	0	43	0	0	0	9	2		
Volume Right	0	0	0	7	0	0	0	15	38	36		
cSH	414	1700	1700	1700	636	1700	1700	1700	326	429		
Volume to Capacity	0.02	0.28	0.28	0.14	0.07	0.41	0.41	0.22	0.14	0.09		THE RESIDENCE OF THE PERSON NAMED IN
Queue Length 95th (ft)	1	0	0	0	5	0	0	0	12	7		
Control Delay (s)	13.8	0.0	0.0	0.0	11.1	0.0	0.0	0.0	17.9	14.2		
Lane LOS	В				В				C	В		
Approach Delay (s)	0.1				0.3				17.9	14.2		
Approach LOS									C	В		
Intersection Summary												
Average Delay			0.6									
Intersection Capacity Utilization	n		50.2%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

	<b>↓</b>	4
Movement	SBT	SBR
Lane Configurations	4	
Traffic Volume (veh/h)	0	31
Future Volume (Veh/h)	0	31
Sign Control	Stop	
Grade	0%	
Peak Hour Factor	0.85	0.85
Hourly flow rate (vph)	0	36
Pedestrians	9	
Lane Width (ft)	12.0	
Walking Speed (ft/s)	4.0	
Percent Blockage	1	
Right turn flare (veh)		
Median type		
Median storage veh)		
Upstream signal (ft)		
pX, platoon unblocked	0.91	0.89
vC, conflicting volume	3066	601
vC1, stage 1 conf vol		
vC2, stage 2 conf vol	0.555	400
vCu, unblocked vol	2571	107
tC, single (s)	6.5	6.9
tC, 2 stage (s)		
tF (s)	4.0	3.3
p0 queue free %	100	96
cM capacity (veh/h)	21	816
Direction, Lane #		

	-	*	<b>←</b>	<b>↓</b>	4
Lane Group	EBT	EBR	WBT	SBT	SBR
Lane Group Flow (vph)	1101	321	1275	940	691
v/c Ratio	0.50	0.34	0.57	0.93	0.44
Control Delay	11.0	9.9	3.5	52.0	0.9
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	11.0	9.9	3.5	52.0	0.9
Queue Length 50th (ft)	190	92	65	330	0
Queue Length 95th (ft)	213	122	66	#412	0
Internal Link Dist (ft)	503		176	192	
Turn Bay Length (ft)					
Base Capacity (vph)	2223	957	2223	1007	1559
Starvation Cap Reductn	0	0	1	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.50	0.34	0.57	0.93	0.44
Intersection Summary					

<sup>95</sup>th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

	۶	<b>→</b>	*	1	<b>—</b>	4	4	†	<i>&gt;</i>	<b>/</b>	Ţ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44	79		<b>^</b>						414	77
Traffic Volume (vph)	0	1013	295	0	1186	0	0	0	0	23	851	643
Future Volume (vph)	0	1013	295	0	1186	0	0	0	0	23	851	643
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		3.4	3.4		3.4						4.8	4.0
Lane Util. Factor		0.95	1.00		0.95						0.95	1.00
Frpb, ped/bikes		1.00	1.00		1.00						1.00	0.98
Flpb, ped/bikes		1.00	1.00		1.00						1.00	1.00
Frt		1.00	0.85		1.00						1.00	0.85
Flt Protected		1.00	1.00		1.00						1.00	1.00
Satd. Flow (prot)		3505	1509		3505						3503	1559
Flt Permitted		1.00	1.00		1.00						1.00	1.00
Satd. Flow (perm)		3505	1509		3505						3503	1559
Peak-hour factor, PHF	0.92	0.92	0.92	0.93	0.93	0.93	0.92	0.92	0.92	0.93	0.93	0.93
Adj. Flow (vph)	0	1101	321	0	1275	0	0	0	0	25	915	691
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	1101	321	0	1275	0	0	0	0	0	940	691
Confl. Peds. (#/hr)	9		9	9		9	10					10
Confl. Bikes (#/hr)												3
Heavy Vehicles (%)	0%	3%	7%	0%	3%	0%	0%	0%	0%	0%	3%	2%
Turn Type		NA	Prot		NA					Split	NA	Free
Protected Phases		2	2		2					4	4	
Permitted Phases												Free
Actuated Green, G (s)		65.0	65.0		65.0						27.5	105.0
Effective Green, g (s)		66.6	66.6		66.6						30.2	105.0
Actuated g/C Ratio		0.63	0.63		0.63						0.29	1.00
Clearance Time (s)		5.0	5.0		5.0						7.5	
Vehicle Extension (s)		3.0	3.0		3.0						3.0	
Lane Grp Cap (vph)		2223	957		2223						1007	1559
v/s Ratio Prot		0.31	0.21		c0.36						c0.27	
v/s Ratio Perm		0.01	0.2.									0.44
v/c Ratio		0.50	0.34		0.57						0.93	0.44
Uniform Delay, d1		10.2	8.9		11.0						36.4	0.0
Progression Factor		0.99	0.99		0.24						0.97	1.00
Incremental Delay, d2		0.8	0.9		0.9						14.9	0.9
Delay (s)		10.9	9.8		3.5						50.3	0.9
Level of Service		В	A		A						D	Α
Approach Delay (s)		10.7			3.5	M. Color		0.0			29.4	
Approach LOS		В			Α			Α			С	
Intersection Summary												
HCM 2000 Control Delay			15.6	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capaci	ity ratio		0.69	MATERIAL PROPERTY.	OW 2000	LOVOI OI	3011100					
Actuated Cycle Length (s)	ity rullo		105.0	S	um of los	t time (s)			8.2			
Intersection Capacity Utilizati	ion		64.3%			of Service			C	446		
Analysis Period (min)	UII		15	10	J LOVOI (	0.0011100						
c Critical Lane Group		WE SEE										
o Official Earle Group	and the second	Design Control					April 1981	State of the last	The street of the street			-

Existing PM W+A

	-	<b>←</b>	*	<b>†</b>	1
Lane Group	EBT	WBT	WBR	NBT	NBR
Lane Group Flow (vph)	1133	1233	232	488	320
v/c Ratio	0.50	0.55	0.23	0.49	0.20
Control Delay	2.9	6.8	5.7	30.5	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	2.9	6.8	5.7	30.5	0.3
Queue Length 50th (ft)	30	142	51	126	0
Queue Length 95th (ft)	m32	m211	m76	m142	m0
Internal Link Dist (ft)	176	1252		171	
Turn Bay Length (ft)			200		
Base Capacity (vph)	2266	2244	1004	996	1592
Starvation Cap Reductn	48	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.51	0.55	0.23	0.49	0.20
Intersection Summary					

m Volume for 95th percentile queue is metered by upstream signal.

	ⅉ	-	*	•	<b>←</b>	*	4	<b>†</b>	1	-	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>	7		414	7			
Traffic Volume (vph)	0	1031	0	0	1134	213	47	368	272	0	0	0
Future Volume (vph)	0	1031	0	0	1134	213	47	368	272	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		3.4			3.4	3.4		4.8	4.0			
Lane Util. Factor		0.95			0.95	1.00		0.95	1.00			
Frpb, ped/bikes		1.00			1.00	1.00		1.00	0.99			
Flpb, ped/bikes		1.00			1.00	1.00		1.00	1.00			
Frt		1.00			1.00	0.85		1.00	0.85			
Flt Protected		1.00			1.00	1.00		0.99	1.00			
Satd. Flow (prot)		3574			3539	1583		3467	1592			
Flt Permitted		1.00			1.00	1.00		0.99	1.00			
Satd. Flow (perm)		3574			3539	1583		3467	1592			
Peak-hour factor, PHF	0.91	0.91	0.91	0.92	0.92	0.92	0.85	0.85	0.85	0.92	0.92	0.92
Adj. Flow (vph)	0	1133	0	0	1233	232	55	433	320	0	0	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	1133	0	0	1233	232	0	488	320	0	0	0
Confl. Peds. (#/hr)									8			
Heavy Vehicles (%)	0%	1%	0%	0%	2%	2%	0%	4%	0%	0%	0%	0%
Turn Type		NA			NA	Perm	Split	NA	Free			
Protected Phases		2			2		4	4				
Permitted Phases		متعدد			وأعدا	2	wari:		Free			
Actuated Green, G (s)		65.0			65.0	65.0		27.5	105.0			
Effective Green, g (s)		66.6			66.6	66.6		30.2	105.0			
Actuated g/C Ratio		0.63			0.63	0.63		0.29	1.00			
Clearance Time (s)		5.0			5.0	5.0		7.5				
Vehicle Extension (s)		3.0			3.0	3.0		3.0				
Lane Grp Cap (vph)		2266			2244	1004		997	1592			
v/s Ratio Prot		0.32			c0.35	1001		c0.14	1002			
v/s Ratio Perm		0.52			00.00	0.15		00.11	0.20	N CAR		
v/c Ratio		0.50			0.55	0.23		0.49	0.20			
Uniform Delay, d1		10.3			10.8	8.2		31.0	0.0			
Progression Factor		0.21			0.59	0.66	The state of the s	0.92	1.00			
Incremental Delay, d2		0.7			0.33	0.2		0.4	0.3			
Delay (s)		2.9			6.7	5.6		29.0	0.3			
Level of Service		2.5 A			Α	Α		C C	A			
Approach Delay (s)		2.9			6.6	- 11		17.6	-		0.0	
Approach LOS		2.9 A			Α			В			A	
Intersection Summary			8.0		CM 2000	Level of S	Sonvico		A			
HCM 2000 Control Delay	ratio		0.53	П	CIVI ZUUU	revel of 3	Del vice		А		TANK MEN	
HCM 2000 Volume to Capacity	iauu		105.0	C	um of lost	time (c)			8.2		STATE OF	
Actuated Cycle Length (s)			64.3%			of Service			C	2000		
Intersection Capacity Utilization Analysis Period (min)			15	IC	O LEVEL	JI SEIVICE				NO COLOR		
ALIGUANS FRIEND HIND			10									**********

Existing PM W+A

## APPENDIX D

2026 Background Future Capacity Analysis Worksheets

	۶	<b>→</b>	*	F	•	<b>—</b>	4	4	†	1	1	<b>↓</b>
Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	ሻ	<b>^</b>			7	<b>^^</b>			4			4
Traffic Volume (veh/h)	14	1420	10	18	19	975	21	1	0	2	4	2
Future Volume (Veh/h)	14	1420	10	18	19	975	21	1	0	2	4	2
Sign Control		Free				Free			Stop			Stop
Grade		0%				0%			0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	15	1543	11	0	21	1060	23	1	0	2	4	2
Pedestrians		3							1			5
Lane Width (ft)		12.0							12.0			12.0
Walking Speed (ft/s)		4.0							4.0			4.0
Percent Blockage		0							0			0
Right turn flare (veh)												
Median type		None				None						
Median storage veh)												
Upstream signal (ft)		993				589						
pX, platoon unblocked	0.95			0.00	0.91			0.93	0.93	0.91	0.93	0.93
vC, conflicting volume	1088			0	1555		The second secon	2001	2710	521	1665	2704
vC1, stage 1 conf vol												1000
vC2, stage 2 conf vol										ar ar a sum on a sum		
vCu, unblocked vol	897			0	1254			1441	2200	115	1081	2193
tC, single (s)	4.1			0.0	4.1			7.5	6.5	6.9	7.5	6.5
tC, 2 stage (s)				THE REAL PROPERTY.								
tF (s)	2.2			0.0	2.2			3.5	4.0	3.3	3.5	4.0
p0 queue free %	98			0	96			99	100	100	97	95
cM capacity (veh/h)	710			0	499			77	39	831	152	39
Direction, Lane #	EB1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB1		
Volume Total	15	617	617	320	21	424	424	235	3	28		
Volume Left	15	0	0	0	21	0	0	0	1	4		
Volume Right	0	0	0	11	0	0	0	23	2	22		
cSH	710	1700	1700	1700	499	1700	1700	1700	195	268		
Volume to Capacity	0.02	0.36	0.36	0.19	0.04	0.25	0.25	0.14	0.02	0.10		
Queue Length 95th (ft)	2	0	0	0	3	0	0	0	1	9		
Control Delay (s)	10.2	0.0	0.0	0.0	12.5	0.0	0.0	0.0	23.7	20.0		
Lane LOS	В	0.0	0.0		В				С	С		
Approach Delay (s)	0.1				0.2				23.7	20.0		
Approach LOS	0.1				0.2				C	C		
Intersection Summary			0.4									
Average Delay	otion		0.4	10	III love	of Consider	11,675,515		Α		Selection.	Armer's
Intersection Capacity Utiliz	auon		41.7%	IC	U Level	of Service			А			
Analysis Period (min)			15									

	•
Movement	SBR
LaneConfigurations	
Traffic Volume (veh/h)	20
Future Volume (Veh/h)	20
Sign Control	
Grade	
Peak Hour Factor	0.92
Hourly flow rate (vph)	22
Pedestrians	
Lane Width (ft)	
Walking Speed (ft/s)	
Percent Blockage	
Right turn flare (veh)	
Median type	
Median storage veh)	
Upstream signal (ft)	
pX, platoon unblocked	0.95
vC, conflicting volume	373
vC1, stage 1 conf vol	
vC2, stage 2 conf vol	
vCu, unblocked vol	142
tC, single (s)	6.9
tC, 2 stage (s)	
tF (s)	3.3
p0 queue free %	97
cM capacity (veh/h)	828
Direction, Lane #	

	-	*	<b>←</b>	ļ	4
Lane Group	EBT	EBR	WBT	SBT	SBR
Lane Group Flow (vph)	1389	345	769	406	325
v/c Ratio	0.57	0.33	0.33	0.47	0.23
Control Delay	19.4	15.5	4.2	63.2	0.4
Queue Delay	0.0	0.0	0.2	0.0	0.0
Total Delay	19.4	15.5	4.4	63.2	0.4
Queue Length 50th (ft)	527	194	53	245	0
Queue Length 95th (ft)	665	290	60	284	0
Internal Link Dist (ft)	509		176	192	
Turn Bay Length (ft)					
Base Capacity (vph)	2416	1050	2325	1014	1411
Starvation Cap Reductn	0	0	758	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.57	0.33	0.49	0.40	0.23
Intersection Summary					

	۶	<b>→</b>	*	•	-	*	1	†	1	1	Ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44	7		44						414	7
Traffic Volume (vph)	0	1306	324	0	738	0	0	0	0	13	361	299
Future Volume (vph)	0	1306	324	0	738	0	0	0	0	13	361	299
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		3.5	3.5		3.5						4.8	4.0
Lane Util. Factor		0.95	1.00		0.95						0.95	1.00
Frpb, ped/bikes		1.00	1.00		1.00						1.00	0.99
Flpb, ped/bikes		1.00	1.00		1.00						1.00	1.00
Frt		1.00	0.85		1.00						1.00	0.85
Flt Protected		1.00	1.00		1.00						1.00	1.00
Satd. Flow (prot)		3505	1524		3374				NAME OF TAXABLE		3180	1411
Flt Permitted		1.00	1.00		1.00						1.00	1.00
Satd. Flow (perm)		3505	1524		3374						3180	1411
Peak-hour factor, PHF	0.94	0.94	0.94	0.96	0.96	0.96	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	1389	345	0	769	0	0	0	0	14	392	325
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	1389	345	0	769	0	0	0	0	0	406	325
Confl. Peds. (#/hr)	2		3	3		2	2					2
Confl. Bikes (#/hr)												1
Heavy Vehicles (%)	0%	3%	6%	0%	7%	0%	0%	0%	0%	50%	12%	13%
Turn Type		NA	Prot		NA					Split	NA	Free
Protected Phases		2	2		2					4	4	
Permitted Phases												Free
Actuated Green, G (s)		150.1	150.1		150.1						57.3	220.0
Effective Green, g (s)		151.7	151.7		151.7						60.0	220.0
Actuated g/C Ratio		0.69	0.69		0.69						0.27	1.00
Clearance Time (s)		5.1	5.1		5.1						7.5	
Vehicle Extension (s)		3.0	3.0	Mill, All	3.0						3.0	
Lane Grp Cap (vph)		2416	1050		2326						867	1411
v/s Ratio Prot		c0.40	0.23		0.23						c0.13	
v/s Ratio Perm												0.23
v/c Ratio		0.57	0.33		0.33						0.47	0.23
Uniform Delay, d1		17.6	13.7		13.7						66.7	0.0
Progression Factor		1.00	1.00		0.27						0.93	1.00
Incremental Delay, d2		1.0	0.8		0.4						0.4	0.4
Delay (s)		18.6	14.5		4.0						62.4	0.4
Level of Service		В	В		Α						E	Α
Approach Delay (s)		17.8			4.0			0.0			34.8	
Approach LOS		В			Α			Α			С	
Intersection Summary												
HCM 2000 Control Delay			18.4	Н	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capacit	y ratio		0.54									
Actuated Cycle Length (s)			220.0	Sı	um of lost	time (s)			8.3			
Intersection Capacity Utilization	n		63.5%		U Level o				В			
Analysis Period (min)			15									
c Critical Lane Group												

	-	•	*	<b>†</b>	1
Lane Group	EBT	WBT	WBR	NBT	NBR
Lane Group Flow (vph)	1428	724	126	767	320
v/c Ratio	0.57	0.31	0.13	0.83	0.20
Control Delay	2.6	12.9	12.1	79.9	0.3
Queue Delay	1.1	0.0	0.0	0.0	0.1
Total Delay	3.7	12.9	12.1	79.9	0.4
Queue Length 50th (ft)	34	165	53	561	0
Queue Length 95th (ft)	37	195	79	612	0
Internal Link Dist (ft)	176	1252		171	
Turn Bay Length (ft)			200		
Base Capacity (vph)	2488	2325	996	1086	1561
Starvation Cap Reductn	40	0	0	0	0
Spillback Cap Reductn	752	0	0	0	325
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.82	0.31	0.13	0.71	0.26
Intersection Summary					

	۶	-	*	•	<b>←</b>	4	4	†	~	1	ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44			<b>^</b>	7		41	7			
Traffic Volume (vph)	0	1314	0	0	688	120	49	657	294	0	0	0
Future Volume (vph)	0	1314	0	0	688	120	49	657	294	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		3.5			3.5	3.5		4.8	4.0			
Lane Util. Factor		0.95			0.95	1.00		0.95	1.00			
Frpb, ped/bikes		1.00			1.00	0.98		1.00	0.99			
Flpb, ped/bikes		1.00			1.00	1.00		1.00	1.00			
Frt		1.00			1.00	0.85		1.00	0.85			
Flt Protected		1.00			1.00	1.00		1.00	1.00			
Satd. Flow (prot)		3610			3374	1446		3404	1561			
Flt Permitted		1.00			1.00	1.00		1.00	1.00			
Satd. Flow (perm)		3610			3374	1446		3404	1561			
Peak-hour factor, PHF	0.92	0.92	0.92	0.95	0.95	0.95	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	1428	0	0	724	126	53	714	320	0	0	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	1428	0	0	724	126	0	767	320	0	0	0
Confl. Peds. (#/hr)	1		1	1		1			7			
Confl. Bikes (#/hr)									1			
Heavy Vehicles (%)	0%	0%	0%	0%	7%	10%	15%	5%	2%	0%	0%	0%
Turn Type		NA			NA	Perm	Split	NA	Free			
Protected Phases		2			2		4	4				
Permitted Phases						2			Free			
Actuated Green, G (s)		150.1			150.1	150.1		57.3	220.0			
Effective Green, g (s)		151.7			151.7	151.7		60.0	220.0			
Actuated g/C Ratio		0.69			0.69	0.69		0.27	1.00			
Clearance Time (s)		5.1			5.1	5.1		7.5				
Vehicle Extension (s)		3.0			3.0	3.0		3.0				
Lane Grp Cap (vph)		2489			2326	997		928	1561			
v/s Ratio Prot		c0.40			0.21			c0.23				
v/s Ratio Perm						0.09			0.21			
v/c Ratio		0.57			0.31	0.13		0.83	0.20			
Uniform Delay, d1		17.5			13.5	11.6		75.1	0.0			
Progression Factor		0.10			0.89	0.94		0.96	1.00			
Incremental Delay, d2		0.8			0.3	0.3		6.1	0.3			
Delay (s)		2.5			12.4	11.1		78.2	0.3			
Level of Service		Α			В	В		Ε	А			
Approach Delay (s)		2.5			12.2			55.3			0.0	
Approach LOS		Α			В			Ε			А	
Intersection Summary												
HCM 2000 Control Delay			22.0	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capacity	y ratio		0.65									
Actuated Cycle Length (s)			220.0	S	um of lost	time (s)			8.3			
Intersection Capacity Utilization	n		63.5%	IC	U Level	of Service			В			
Analysis Period (min)			15									
c Critical Lane Group												

	₫	۶	<b>→</b>	*	<b>F</b>	•	<b>—</b>	4	4	†	~	1
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		7	<b>^^</b>			7	<b>^^</b>			4		
Traffic Volume (veh/h)	4	7	1192	7	56	40	1692	14	8	0	32	2
Future Volume (Veh/h)	4	7	1192	7	56	40	1692	14	8	0	32	2
Sign Control			Free				Free			Stop		
Grade			0%				0%			0%		
Peak Hour Factor	0.96	0.96	0.96	0.96	0.93	0.93	0.93	0.93	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	7	1242	7	0	43	1819	15	9	0	35	2
Pedestrians										4		
Lane Width (ft)										12.0		
Walking Speed (ft/s)										4.0		
Percent Blockage										0		
Right turn flare (veh)												
Median type			None				None					
Median storage veh)												2,240
Upstream signal (ft)			999				583					
pX, platoon unblocked	0.00	0.87			0.00	0.95	RS IN		0.90	0.90	0.95	0.90
vC, conflicting volume	0	1843			0	1253			1990	3192	422	2384
vC1, stage 1 conf vol												
vC2, stage 2 conf vol		A PARTICIPATION OF THE PARTICI							Department of the			
vCu, unblocked vol	0	1460			0	1074			1320	2656	197	1759
tC, single (s)	0.0	4.1			0.0	4.1			7.5	6.5	6.9	7.5
tC, 2 stage (s)					# P V (							
tF (s)	0.0	2.2			0.0	2.2			3.5	4.0	3.3	3.5
p0 queue free %	0	98			0	93			90	100	95	95
cM capacity (veh/h)	0	398			0	609			91	18	766	43
Direction, Lane #	EB 1	EB 2	EB3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		(45±30)
Volume Total	7	497	497	255	43	728	728	379	44	36		
Volume Left	7	0	0	0	43	0	0	0	9	2		
Volume Right	0	0	0	7	0	0	0	15	35	34		
cSH	398	1700	1700	1700	609	1700	1700	1700	305	415		
Volume to Capacity	0.02	0.29	0.29	0.15	0.07	0.43	0.43	0.22	0.14	0.09		
Queue Length 95th (ft)	1	0	0	0	6	0	0	0	12	7		100
Control Delay (s)	14.2	0.0	0.0	0.0	11.4	0.0	0.0	0.0	18.8	14.5		
Lane LOS	В	0.0	0.0	0.0	В				C	В		T. E. I.
Approach Delay (s)	0.1				0.3				18.8	14.5		
Approach LOS	0.1				0.0				C	В		
Intersection Summary												
Average Delay			0.6					A Maria		NU ACTO		
Intersection Capacity Utiliza	ation		51.4%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

	<b>↓</b>	4
Movement	SBT	SBR
Lane Configurations	4	
Traffic Volume (veh/h)	0	31
Future Volume (Veh/h)	0	31
Sign Control	Stop	
Grade	0%	
Peak Hour Factor	0.92	0.92
Hourly flow rate (vph)	0	34
Pedestrians	9	
Lane Width (ft)	12.0	
Walking Speed (ft/s)	4.0	
Percent Blockage	1	
Right turn flare (veh)		
Median type		
Median storage veh)		
Upstream signal (ft)		
pX, platoon unblocked	0.90	0.87
vC, conflicting volume	3188	623
vC1, stage 1 conf vol		
vC2, stage 2 conf vol		
vCu, unblocked vol	2652	65
tC, single (s)	6.5	6.9
tC, 2 stage (s)		
tF (s)	4.0	3.3
p0 queue free %	100	96
cM capacity (veh/h)	18	856
Direction, Lane #		

	<b>→</b>	*	<b>+</b>	ļ	1
Lane Group	EBT	EBR	WBT	SBT	SBR
Lane Group Flow (vph)	1139	343	1320	959	715
v/c Ratio	0.51	0.36	0.59	0.95	0.46
Control Delay	11.2	10.2	3.7	55.1	1.0
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	11.2	10.2	3.7	55.1	1.0
Queue Length 50th (ft)	199	100	67	338	0
Queue Length 95th (ft)	223	132	68	#441	0
Internal Link Dist (ft)	503		176	192	
Turn Bay Length (ft)					
Base Capacity (vph)	2223	957	2223	1007	1559
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.51	0.36	0.59	0.95	0.46
Intersection Summary					
			Carlo Company of the		

<sup>95</sup>th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

	۶	<b>→</b>	*	•	•	*	4	<b>†</b>	~	1	<b>↓</b>	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44	7		44						414	7
Traffic Volume (vph)	0	1048	316	0	1228	0	0	0	0	24	868	665
Future Volume (vph)	0	1048	316	0	1228	0	0	0	0	24	868	665
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		3.4	3.4		3.4						4.8	4.0
Lane Util. Factor		0.95	1.00		0.95						0.95	1.00
Frpb, ped/bikes		1.00	1.00		1.00						1.00	0.98
Flpb, ped/bikes		1.00	1.00		1.00						1.00	1.00
Frt		1.00	0.85		1.00						1.00	0.85
Flt Protected		1.00	1.00		1.00						1.00	1.00
Satd. Flow (prot)		3505	1509		3505						3503	1559
Flt Permitted		1.00	1.00		1.00						1.00	1.00
Satd. Flow (perm)		3505	1509		3505						3503	1559
Peak-hour factor, PHF	0.92	0.92	0.92	0.93	0.93	0.93	0.92	0.92	0.92	0.93	0.93	0.93
Adj. Flow (vph)	0	1139	343	0	1320	0	0	0	0	26	933	715
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	1139	343	0	1320	0	0	0	0	0	959	715
Confl. Peds. (#/hr)	9		9	9		9	10					10
Confl. Bikes (#/hr)												3
Heavy Vehicles (%)	0%	3%	7%	0%	3%	0%	0%	0%	0%	0%	3%	2%
Turn Type		NA	Prot		NA					Split	NA	Free
Protected Phases		2	-2		2					4	4	
Permitted Phases												Free
Actuated Green, G (s)		65.0	65.0		65.0						27.5	105.0
Effective Green, g (s)		66.6	66.6		66.6						30.2	105.0
Actuated g/C Ratio		0.63	0.63		0.63						0.29	1.00
Clearance Time (s)	No.	5.0	5.0		5.0						7.5	
Vehicle Extension (s)		3.0	3.0		3.0						3.0	
Lane Grp Cap (vph)		2223	957		2223						1007	1559
v/s Ratio Prot		0.32	0.23		c0.38						c0.27	
v/s Ratio Perm		0.02	0.120									0.46
v/c Ratio		0.51	0.36		0.59					4159	0.95	0.46
Uniform Delay, d1		10.4	9.1	No. of Lot, House, St. Co.	11.3						36.7	0.0
Progression Factor		0.99	0.99		0.24					4337	0.97	1.00
Incremental Delay, d2		0.8	1.0		1.0						17.9	1.0
Delay (s)		11.1	10:1		3.7						53.7	1.0
Level of Service		В	В		А					Charles and the Ass	D	Α
Approach Delay (s)		10.9			3.7			0.0			31.2	
Approach LOS		В			A			A			С	
				Was to leave the leave							N. FESTER	
Intersection Summary			16.2	Ц	CM 2000	Level of S	Conico		В			
HCM 2000 Control Delay	aitu ratio		16.3	П	CIVI 2000	Level of 3	Service		D			
HCM 2000 Volume to Capac	city ratio		0.71	C.	um of loca	time (c)		a House	8.2	10.00		
Actuated Cycle Length (s)	tion		105.0		um of lost	of Service	atincka <u>i</u>		0.2 C			
Intersection Capacity Utilizat	uOH	MARK AND	66.0%	IC	O Level (	JI SEIVICE			C			
Analysis Period (min)	Sautoronia di		15									
c Critical Lane Group		distribution of the										

	<b>→</b>	<b>←</b>	*	<b>†</b>	1
Lane Group	EBT	WBT	WBR	NBT	NBR
Lane Group Flow (vph)	1160	1273	236	478	302
v/c Ratio	0.51	0.57	0.24	0.48	0.19
Control Delay	2.9	6.9	5.7	30.5	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	2.9	6.9	5.7	30.5	0.3
Queue Length 50th (ft)	30	146	52	124	0
Queue Length 95th (ft)	m31	m220	m77	m147	m0
Internal Link Dist (ft)	176	1252		171	
Turn Bay Length (ft)			200		
Base Capacity (vph)	2266	2244	1004	997	1592
Starvation Cap Reductn	42	0	0	0	0
Spillback Cap Reductn	0	6	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.52	0.57	0.24	0.48	0.19
Intersection Summary					
m Volume for 95th percentil	le queue i	is metered	d by upsti	ream sign	ıal.

	۶	<b>→</b>	*	•	-	4	4	†	~	1	ţ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>	7		414	7			
Traffic Volume (vph)	0	1067	0	0	1171	217	53	386	278	0	0	0
Future Volume (vph)	0	1067	0	0	1171	217	53	386	278	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		3.4			3.4	3.4		4.8	4.0			
Lane Util. Factor		0.95			0.95	1.00		0.95	1.00			
Frpb, ped/bikes		1.00			1.00	1.00		1.00	0.99			
Flpb, ped/bikes		1.00			1.00	1.00		1.00	1.00			
Frt		1.00			1.00	0.85		1.00	0.85			
Flt Protected		1.00			1.00	1.00		0.99	1.00			
Satd. Flow (prot)		3574			3539	1583		3466	1592			
Flt Permitted		1.00			1.00	1.00		0.99	1.00			SER
Satd. Flow (perm)		3574			3539	1583		3466	1592			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	1160	0	0	1273	236	58	420	302	0	0	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	1160	0	0	1273	236	0	478	302	0	0	0
Confl. Peds. (#/hr)									8	PANNE		
Heavy Vehicles (%)	0%	1%	0%	0%	2%	2%	0%	4%	0%	0%	0%	0%
Turn Type	070	NA	0,0	070	NA	Perm	Split	NA	Free	070	0,0	070
Protected Phases		2			2	I GIIII	4	4	1100			
Permitted Phases		_				2			Free			
Actuated Green, G (s)		65.0			65.0	65.0		27.5	105.0			
Effective Green, g (s)		66.6			66.6	66.6		30.2	105.0			
Actuated g/C Ratio		0.63			0.63	0.63		0.29	1.00			
Clearance Time (s)		5.0			5.0	5.0		7.5	1.00			
Vehicle Extension (s)		3.0			3.0	3.0		3.0			SASTA SA	
	2000			- 0.5			PASSION IN		1500	onen.	A COLUMN TO SERVICE	10000
Lane Grp Cap (vph)		2266			2244	1004		996	1592			
v/s Ratio Prot		0.32	Name and Address of the Owner, where the Owner, which is the		c0.36	0.15		c0.14	0.10	************		MARKET MARKET
v/s Ratio Perm						0.15			0.19			
v/c Ratio		0.51			0.57	0.24		0.48	0.19			
Uniform Delay, d1		10.4			11.0	8.3		30.9	0.0			
Progression Factor		0.21			0.59	0.66		0.93	1.00			
Incremental Delay, d2		0.7			0.4	0.2	200	0.4	0.3			
Delay (s)		2.9			6.9	5.7		29.1	0.3			Market Wall
Level of Service		Α			Α	Α		C	Α			
Approach Delay (s)		2.9			6.7			17.9			0.0	
Approach LOS		A			A			В			A	
Intersection Summary										1455		
HCM 2000 Control Delay			7.9	Н	CM 2000	Level of S	Service		A			
HCM 2000 Volume to Capacity	ratio		0.54									
Actuated Cycle Length (s)			105.0	Si	um of lost	time (s)			8.2			
Intersection Capacity Utilization			66.0%	IC	U Level	of Service			С			
Analysis Period (min)			15									
c Critical Lane Group												

## APPENDIX E

2026 Total Future Capacity Analysis Worksheets

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Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	N.	<del>ተ</del> ተጉ			1	<b>^^</b>			4			4
Traffic Volume (veh/h)	16	1420	10	18	19	975	26	1	0	2	14	2
Future Volume (Veh/h)	16	1420	10	18	19	975	26	1	0	2	14	2
Sign Control		Free				Free			Stop			Stop
Grade		0%				0%			0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	17	1543	11	0	21	1060	28	1	0	2	15	2
Pedestrians		3							1			5
Lane Width (ft)		12.0							12.0			12.0
Walking Speed (ft/s)		4.0							4.0			4.0
Percent Blockage		0							0			0
Right turn flare (veh)												
Median type		None				None						
Median storage veh)												
Upstream signal (ft)		993				589						
pX, platoon unblocked	0.95			0.00	0.91			0.93	0.93	0.91	0.93	0.93
vC, conflicting volume	1093			0	1555			2010	2718	521	1671	2710
vC1, stage 1 conf vol												
vC2, stage 2 conf vol			MAN THE STATE	A CONTRACTOR								
vCu, unblocked vol	899			0	1254			1445	2204	115	1083	2195
tC, single (s)	4.1			0.0	4.1		A company	7.5	6.5	6.9	7.5	6.5
tC, 2 stage (s)					45							
tF (s)	2.2			0.0	2.2		Trail Control of the	3.5	4.0	3.3	3.5	4.0
p0 queue free %	98			0	96			99	100	100	90	95
cM capacity (veh/h)	708			0	499			76	38	831	151	39
		ED 0	ED 0			WD 0	MD				101	
Direction, Lane #	EB 1	EB 2	EB 3	320	WB 1	WB 2	WB 3	WB 4 240	NB 1	SB 1		
Volume Total									1	15		
Volume Left	17	0	0	0	21	0	0	0		27		
Volume Right	0	0	1700	1700	0	1700	1700	28	2			
cSH	708	1700	1700	1700	499	1700	1700	1700	192	240		
Volume to Capacity	0.02	0.36	0.36	0.19	0.04	0.25	0.25	0.14	0.02	0.18		
Queue Length 95th (ft)	2	0	0	0	3	0	0	0	1	16		
Control Delay (s)	10.2	0.0	0.0	0.0	12.5	0.0	0.0	0.0	24.0	23.4		Under States
Lane LOS	В				В				C	C		
Approach Delay (s)	0.1				0.2			CONTRACTOR OF THE PARTY OF THE	24.0	23.4		Married W. L.
Approach LOS									C	C		
Intersection Summary												
Average Delay			0.6									
Intersection Capacity Utiliza	ation		41.8%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

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Movement	SBR
LaneConfigurations	
Traffic Volume (veh/h)	25
Future Volume (Veh/h)	25
Sign Control	
Grade	
Peak Hour Factor	0.92
Hourly flow rate (vph)	27
Pedestrians	
Lane Width (ft)	
Walking Speed (ft/s)	
Percent Blockage	
Right turn flare (veh)	
Median type	
Median storage veh)	
Upstream signal (ft)	0.05
pX, platoon unblocked	0.95
vC, conflicting volume	375
vC1, stage 1 conf vol	
vC2, stage 2 conf vol	140
vCu, unblocked vol	140
tC, single (s)	6.9
tC, 2 stage (s)	2.2
tF (s)	3.3
p0 queue free %	97
cM capacity (veh/h)	829
Direction, Lane #	

	<b>→</b>	*	←	<b></b>	1
Lane Group	EBT	EBR	WBT	SBT	SBR
Lane Group Flow (vph)	1395	350	772	406	327
v/c Ratio	0.58	0.33	0.33	0.47	0.23
Control Delay	19.6	15.7	4.2	63.0	0.4
Queue Delay	0.0	0.0	0.2	0.0	0.0
Total Delay	19.6	15.7	4.4	63.0	0.4
Queue Length 50th (ft)	533	198	53	244	0
Queue Length 95th (ft)	673	296	60	284	0
Internal Link Dist (ft)	509		176	192	
Turn Bay Length (ft)					
Base Capacity (vph)	2411	1048	2322	1014	1411
Starvation Cap Reductn	0	0	751	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.58	0.33	0.49	0.40	0.23
Intersection Summary					

	۶	<b>→</b>	*	•	<b>←</b>	4	4	<b>†</b>	~	-	Ţ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44	7		<b>^</b>						414	7
Traffic Volume (vph)	0	1311	329	0	741	0	0	0	0	13	361	301
Future Volume (vph)	0	1311	329	0	741	0	0	0	0	13	361	301
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		3.5	3.5		3.5						4.8	4.0
Lane Util. Factor		0.95	1.00		0.95						0.95	1.00
Frpb, ped/bikes		1.00	1.00		1.00						1.00	0.99
Flpb, ped/bikes		1.00	1.00		1.00						1.00	1.00
Frt		1.00	0.85		1.00						1.00	0.85
Flt Protected		1.00	1.00		1.00						1.00	1.00
Satd. Flow (prot)		3505	1524		3374						3180	1411
Flt Permitted		1.00	1.00		1.00						1.00	1.00
Satd. Flow (perm)		3505	1524		3374						3180	1411
Peak-hour factor, PHF	0.94	0.94	0.94	0.96	0.96	0.96	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	1395	350	0	772	0	0	0	0	14	392	327
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	1395	350	0	772	0	0	0	0	0	406	327
Confl. Peds. (#/hr)	2		3	3		2	2					2
Confl. Bikes (#/hr)												1
Heavy Vehicles (%)	0%	3%	6%	0%	7%	0%	0%	0%	0%	50%	12%	13%
Turn Type		NA	Prot		NA					Split	NA	Free
Protected Phases		2	2		2					4	4	
Permitted Phases												Free
Actuated Green, G (s)		149.8	149.8		149.8						57.6	220.0
Effective Green, g (s)		151.4	151.4		151.4						60.3	220.0
Actuated g/C Ratio		0.69	0.69		0.69						0.27	1.00
Clearance Time (s)		5.1	5.1		5.1						7.5	
Vehicle Extension (s)		3.0	3.0		3.0						3.0	
Lane Grp Cap (vph)		2412	1048	- Allia	2321						871	1411
v/s Ratio Prot		c0.40	0.23		0.23						c0.13	
v/s Ratio Perm	3,000,000	60.40	0.20		0.25						00.10	0.23
v/c Ratio		0.58	0.33		0.33						0.47	0.23
Uniform Delay, d1		17.8	13.9		13.9						66.5	0.0
Progression Factor		1.00	1.00		0.27			TOTAL SERVICE			0.93	1.00
Incremental Delay, d2		1.00	0.9		0.4						0.4	0.4
Delay (s)		18.8	14.7		4.1			e a ve la s			62.2	0.4
Level of Service		В	В		Α.Ι						E	Α
Approach Delay (s)		18.0	D		4.1			0.0			34.6	
Approach LOS		В			Α.1			Α			C	
					^							
Intersection Summary			10.4		014 0000	1 1 - 6 6						
HCM 2000 Control Delay	•		18.4	H	CM 2000	Level of S	service		В			
HCM 2000 Volume to Capac	ity ratio		0.55			tions (-)			0.0			37.23
Actuated Cycle Length (s)	•		220.0		um of lost				8.3			
Intersection Capacity Utilizat	ion		63.7%	IC	U Level (	of Service			В			
Analysis Period (min)			15									
c Critical Lane Group									Mark the			7

	<b>→</b>	<b>←</b>	*	<b>†</b>	1
Lane Group	EBT	WBT	WBR	NBT	NBR
Lane Group Flow (vph)	1434	727	126	771	320
v/c Ratio	0.58	0.31	0.13	0.83	0.20
Control Delay	2.6	13.0	12.1	79.9	0.3
Queue Delay	1.2	0.0	0.0	0.0	0.1
Total Delay	3.8	13.0	12.1	79.9	0.4
Queue Length 50th (ft)	34	166	53	564	0
Queue Length 95th (ft)	38	195	80	615	0
Internal Link Dist (ft)	176	1252		171	
Turn Bay Length (ft)			200		
Base Capacity (vph)	2484	2322	995	1086	1561
Starvation Cap Reductn	40	0	0	0	0
Spillback Cap Reductn	749	0	0	0	324
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.83	0.31	0.13	0.71	0.26
Intersection Summary					

	•	<b>→</b>	*	•	<b>←</b>	*	4	<b>†</b>	~	-	<b>↓</b>	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44			<b>个</b> 个	7		ተቡ	7			
Traffic Volume (vph)	0	1319	0	0	691	120	49	661	294	0	0	0
Future Volume (vph)	0	1319	0	0	691	120	49	661	294	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		3.5			3.5	3.5		4.8	4.0			
Lane Util. Factor		0.95			0.95	1.00		0.95	1.00			
Frpb, ped/bikes		1.00			1.00	0.98		1.00	0.99			
Flpb, ped/bikes		1.00			1.00	1.00		1.00	1.00			
Frt		1.00			1.00	0.85		1.00	0.85			
Flt Protected		1.00			1.00	1.00		1.00	1.00			
Satd. Flow (prot)		3610			3374	1446		3404	1561			
Flt Permitted		1.00			1.00	1.00		1.00	1.00			
Satd. Flow (perm)		3610			3374	1446		3404	1561			
Peak-hour factor, PHF	0.92	0.92	0.92	0.95	0.95	0.95	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	1434	0	0	727	126	53	718	320	0	0	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	1434	0	0	727	126	0	771	320	0	0	0
Confl. Peds. (#/hr)	1		1	1		1			7			
Confl. Bikes (#/hr)									1			
Heavy Vehicles (%)	0%	0%	0%	0%	7%	10%	15%	5%	2%	0%	0%	0%
Turn Type		NA			NA	Perm	Split	NA	Free			
Protected Phases		2			2		4	4				
Permitted Phases						2			Free			
Actuated Green, G (s)		149.8			149.8	149.8		57.6	220.0			
Effective Green, g (s)		151.4			151.4	151.4		60.3	220.0			
Actuated g/C Ratio		0.69			0.69	0.69		0.27	1.00			
Clearance Time (s)		5.1			5.1	5.1		7.5				
Vehicle Extension (s)		3.0			3.0	3.0		3.0				
Lane Grp Cap (vph)		2484			2321	995		933	1561			
v/s Ratio Prot		c0.40			0.22			c0.23				
v/s Ratio Perm						0.09			0.21			
v/c Ratio		0.58			0.31	0.13		0.83	0.20			
Uniform Delay, d1		17.7			13.6	11.7		74.9	0.0			
Progression Factor		0.10			0.89	0.94		0.96	1.00			
Incremental Delay, d2		0.8			0.3	0.3		6.1	0.3			
Delay (s)		2.6			12.5	11.2		78.1	0.3			
Level of Service		Α			В	В		Ε	Α			
Approach Delay (s)		2.6			12.3			55.3			0.0	
Approach LOS		Α			В			Ε			Α	
Intersection Summary												
HCM 2000 Control Delay			22.0	Н	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capacity	ratio		0.65									
Actuated Cycle Length (s)			220.0	Sı	um of lost	time (s)			8.3			
Intersection Capacity Utilization	n		63.7%			of Service			В			
Analysis Period (min)			15									
c Critical Lane Group												

The Lamb Center

	•	•	T		1	₩
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	À		1			4
Traffic Volume (veh/h)	15	0	34	8	0	26
Future Volume (Veh/h)	15	0	34	8	0	26
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	0	37	9	0	28
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage			te avelited		NAME OF TAXABLE	
Right turn flare (veh)						
Median type			None			None
			None			None
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked	70	40			40	DAME
vC, conflicting volume	70	42			46	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol	70	10			40	
vCu, unblocked vol	70	42			46	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	98	100			100	
cM capacity (veh/h)	935	1029			1562	
Direction, Lane #	WB 1	NB1	SB 1			
Volume Total	16	46	28			
Volume Left	16	0	0			
Volume Right	0	9	0			
cSH	935	1700	1562			
Volume to Capacity	0.02	0.03	0.00			
Queue Length 95th (ft)	1	0	0			
Control Delay (s)	8.9	0.0	0.0			
Lane LOS	А					
Approach Delay (s)	8.9	0.0	0.0			A DESCRIPTION OF THE PARTY OF T
Approach LOS	A		3.0			
Intersection Summary						
Average Delay			1.6			THE PARTY OF
Intersection Capacity Utiliza	ntion	Managar.	13.3%	IC	II Lovel s	of Service
	auUII			IC	o revel (	JI SELVICE
Analysis Period (min)			15			

The Lamb Center

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Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		ň	ተተጉ			7	ተተጉ			4		
Traffic Volume (veh/h)	4	18	1192	7	56	40	1692	20	8	0	32	6
Future Volume (Veh/h)	4	18	1192	7	56	40	1692	20	8	0	32	6
Sign Control			Free				Free			Stop		
Grade		******************	0%				0%			0%		
Peak Hour Factor	0.96	0.96	0.96	0.96	0.93	0.93	0.93	0.93	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	19	1242	7	0	43	1819	22	9	0	35	7
Pedestrians										4		
Lane Width (ft)										12.0		
Walking Speed (ft/s)										4.0		
Percent Blockage										0		
Right turn flare (veh)												
Median type			None	of the second			None					
Median storage veh)			110110				110110					
Upstream signal (ft)			999				583					
pX, platoon unblocked	0.00	0.87	300		0.00	0.95			0.90	0.90	0.95	0.90
vC, conflicting volume	0.00	1850			0.00	1253			2022	3224	422	2412
vC1, stage 1 conf vol		1000				1200			2022			
vC2, stage 2 conf vol												
vCu, unblocked vol	0	1463			0	1074			1351	2688	197	1785
tC, single (s)	0.0	4.1			0.0	4.1			7.5	6.5	6.9	7.5
tC, 2 stage (s)	0.0				0.0					0.0	0.0	7.0
tF (s)	0.0	2.2			0.0	2.2		A SHAREST AND A	3.5	4.0	3.3	3.5
p0 queue free %	0.0	95			0.0	93			89	100	95	82
cM capacity (veh/h)	0	396			0	609		media - car	84	17	766	40
							WD 6	1115.4			700	40
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	19	497	497	255	43	728	728	386	44	49		
Volume Left	19	0	0	0	43	0	0	0	9	7		
Volume Right	0	0	0	7	0	0	0	22	35	42		
cSH	396	1700	1700	1700	609	1700	1700	1700	288	217		
Volume to Capacity	0.05	0.29	0.29	0.15	0.07	0.43	0.43	0.23	0.15	0.23		EDECKE METERS
Queue Length 95th (ft)	4	0	0	0	6	0	0	0	13	21		
Control Delay (s)	14.5	0.0	0.0	0.0	11.4	0.0	0.0	0.0	19.8	26.3		
Lane LOS	В				В				С	D		
Approach Delay (s)	0.2				0.3			-	19.8	26.3		
Approach LOS									C	D		
Intersection Summary												
Average Delay			0.9									
Intersection Capacity Utiliza	ation		50.2%	IC	U Level of	of Service	;		Α			
Analysis Period (min)			15									

	ı	1
	*	4
Movement	SBT	SBR
Lane Configurations	4	4.7
Traffic Volume (veh/h)	0	39
Future Volume (Veh/h)	0	39
Sign Control	Stop	
Grade	0%	
Peak Hour Factor	0.92	0.92
Hourly flow rate (vph)	0	42
Pedestrians	9	
Lane Width (ft)	12.0	
Walking Speed (ft/s)	4.0	
Percent Blockage	1	
Right turn flare (veh)		
Median type		
Median storage veh)		
Upstream signal (ft)		
pX, platoon unblocked	0.90	0.87
vC, conflicting volume	3216	626
vC1, stage 1 conf vol		
vC2, stage 2 conf vol		
vCu, unblocked vol	2679	61
tC, single (s)	6.5	6.9
tC, 2 stage (s)		
tF (s)	4.0	3.3
p0 queue free %	100	95
cM capacity (veh/h)	17	858
Direction, Lane #		

	<b>→</b>	*	<b>—</b>	ļ	1
Lane Group	EBT	EBR	WBT	SBT	SBR
Lane Group Flow (vph)	1141	346	1322	959	718
v/c Ratio	0.51	0.36	0.59	0.95	0.46
Control Delay	11.2	10.2	3.7	55.1	1.0
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	11.2	10.2	3.7	55.1	1.0
Queue Length 50th (ft)	200	101	67	339	0
Queue Length 95th (ft)	224	133	68	#441	0
Internal Link Dist (ft)	503		176	192	
Turn Bay Length (ft)					
Base Capacity (vph)	2223	957	2223	1007	1559
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.51	0.36	0.59	0.95	0.46
Intersection Summary					
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<sup>95</sup>th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44	7		44						414	7
Traffic Volume (vph)	0	1050	318	0	1229	0	0	0	0	24	868	668
Future Volume (vph)	0	1050	318	0	1229	0	0	0	0	24	868	668
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		3.4	3.4		3.4						4.8	4.0
Lane Util. Factor		0.95	1.00		0.95						0.95	1.00
Frpb, ped/bikes		1.00	1.00		1.00						1.00	0.98
Flpb, ped/bikes		1.00	1.00		1.00						1.00	1.00
Frt		1.00	0.85		1.00						1.00	0.85
Flt Protected		1.00	1.00		1.00						1.00	1.00
Satd. Flow (prot)		3505	1509		3505						3503	1559
Flt Permitted		1.00	1.00		1.00						1.00	1.00
Satd. Flow (perm)		3505	1509		3505						3503	1559
Peak-hour factor, PHF	0.92	0.92	0.92	0.93	0.93	0.93	0.92	0.92	0.92	0.93	0.93	0.93
Adj. Flow (vph)	0	1141	346	0	1322	0	0	0	0	26	933	718
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	1141	346	0	1322	0	0	0	0	0	959	718
Confl. Peds. (#/hr)	9		9	9		9	10					10
Confl. Bikes (#/hr)												3
Heavy Vehicles (%)	0%	3%	7%	0%	3%	0%	0%	0%	0%	0%	3%	2%
Turn Type		NA	Prot		NA					Split	NA	Free
Protected Phases		2	2		2					4	4	
Permitted Phases	The second second											Free
Actuated Green, G (s)		65.0	65.0		65.0						27.5	105.0
Effective Green, g (s)		66.6	66.6		66.6						30.2	105.0
Actuated g/C Ratio		0.63	0.63		0.63						0.29	1.00
Clearance Time (s)		5.0	5.0		5.0						7.5	
Vehicle Extension (s)		3.0	3.0		3.0						3.0	
Lane Grp Cap (vph)		2223	957		2223						1007	1559
v/s Ratio Prot		0.33	0.23		c0.38						c0.27	
v/s Ratio Perm												0.46
v/c Ratio		0.51	0.36		0.59						0.95	0.46
Uniform Delay, d1		10.4	9.1		11.3						36.7	0.0
Progression Factor		0.99	0.99		0.24						0.97	1.00
Incremental Delay, d2		0.9	1.1		1.0	A Committee Minney					17.9	1.0
Delay (s)		11.1	10.1		3.7						53.7	1.0
Level of Service		В	В		Α						D	Α
Approach Delay (s)		10.9			3.7			0.0			31.1	
Approach LOS		В			Α			Α			С	
Intersection Summary												
HCM 2000 Control Delay			16.3	Н	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capacity	ratio		0.71									
Actuated Cycle Length (s)			105.0	Sı	um of lost	time (s)			8.2			The second second
Intersection Capacity Utilization	n		66.0%			of Service			C			
Analysis Period (min)			15									
c Critical Lane Group												

	<b>→</b>	<b>←</b>	*	<b>†</b>	-	
Lane Group	EBT	WBT	WBR	NBT	NBR	
Lane Group Flow (vph)	1162	1274	236	480	302	
v/c Ratio	0.51	0.57	0.24	0.48	0.19	
Control Delay	2.9	6.9	5.7	30.5	0.3	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	2.9	6.9	5.7	30.5	0.3	
Queue Length 50th (ft)	30	147	52	124	0	
Queue Length 95th (ft)	m31	m221	m77	m148	m0	
Internal Link Dist (ft)	176	1252		171		
Turn Bay Length (ft)			200			
Base Capacity (vph)	2266	2244	1004	996	1592	
Starvation Cap Reductn	42	0	0	0	0	
Spillback Cap Reductn	0	6	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.52	0.57	0.24	0.48	0.19	
Intersection Summary						

m Volume for 95th percentile queue is metered by upstream signal.

	۶	<b>→</b>	*	•	<b>←</b>	•	4	<b>†</b>	~	1	ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			44	7		41	i <sup>r</sup>			
Traffic Volume (vph)	0	1069	0	0	1172	217	53	388	278	0	0	0
Future Volume (vph)	0	1069	0	0	1172	217	53	388	278	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		3.4			3.4	3.4		4.8	4.0			
Lane Util. Factor		0.95			0.95	1.00		0.95	1.00			
Frpb, ped/bikes	Į1	1.00			1.00	1.00		1.00	0.99			
Flpb, ped/bikes		1.00			1.00	1.00		1.00	1.00			
Frt		1.00			1.00	0.85		1.00	0.85			
Flt Protected		1.00			1.00	1.00	4566	0.99	1.00			
Satd. Flow (prot)		3574			3539	1583		3466	1592			
Flt Permitted		1.00			1.00	1.00		0.99	1.00			
Satd. Flow (perm)		3574			3539	1583		3466	1592			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	1162	0	0	1274	236	58	422	302	0	0	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	1162	0	0	1274	236	0	480	302	0	0	0
Confl. Peds. (#/hr)									8			
Heavy Vehicles (%)	0%	1%	0%	0%	2%	2%	0%	4%	0%	0%	0%	0%
Turn Type	Tank on	NA			NA	Perm	Split	NA	Free			
Protected Phases		2			2		4	4				
Permitted Phases						2			Free			
Actuated Green, G (s)		65.0			65.0	65.0		27.5	105.0			
Effective Green, g (s)		66.6			66.6	66.6		30.2	105.0			
Actuated g/C Ratio		0.63			0.63	0.63		0.29	1.00			
Clearance Time (s)		5.0			5.0	5.0		7.5				
Vehicle Extension (s)		3.0			3.0	3.0		3.0				
Lane Grp Cap (vph)		2266		No.	2244	1004		996	1592			
v/s Ratio Prot		0.33			c0.36			c0.14				
v/s Ratio Perm						0.15			0.19			1
v/c Ratio	No. 10 July 18 Miles	0.51			0.57	0.24		0.48	0.19			ALFORD DESCRI
Uniform Delay, d1		10.4			11.0	8.3		30.9	0.0			
Progression Factor		0.21			0.59	0.66		0.93	1.00			
Incremental Delay, d2		0.7	eric		0.4	0.2		0.4	0.3			
Delay (s)		2.9			6.9	5.7		29.1	0.3			
Level of Service		A			А	А		С	Α			
Approach Delay (s)		2.9			6.7			18.0			0.0	Name of Street
Approach LOS		A			A			В			A	
Intersection Summary						SECTION OF			HE THE		ANGE	
HCM 2000 Control Delay		MATERIAL STATE	8.0	Н	CM 2000	Level of S	Service	ERRESTS OF	А		HELLING	
HCM 2000 Volume to Capacity	ratio		0.54		2000	2010101	311100					Selection in the select
Actuated Cycle Length (s)	7000		105.0	Si	um of lost	time (s)			8.2			
Intersection Capacity Utilization	n	Marin May 1	66.0%			of Service			C			THE PARTY NAMED IN
Analysis Period (min)			15		S LOVOI (	, Joi vide						
c Critical Lane Group	P-15/4/200		10									

<u>4:</u>					,		
	1	*	Ī	1	-	¥	
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Y		P			र्स	
Traffic Volume (veh/h)	12	0	21	17	0	33	
Future Volume (Veh/h)	12	0	21	17	0	33	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	13	0	23	18	0	36	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (ft)			- 25				
pX, platoon unblocked							
vC, conflicting volume	68	32			41		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	68	32			41		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)							
tF (s)	3.5	3.3			2.2		
p0 queue free %	99	100			100		
cM capacity (veh/h)	937	1042			1568		
Direction, Lane #	WB 1	NB1	SB 1				
Volume Total	13	41	36	CONTRACTOR DESCRIPTION			THE PARTY OF THE P
Volume Left	13	0	0				
Volume Right	0	18	0				
cSH	937	1700	1568				
Volume to Capacity	0.01	0.02	0.00				
Queue Length 95th (ft)	1	0	0				
Control Delay (s)	8.9	0.0	0.0				
Lane LOS	A						
Approach Delay (s)	8.9	0.0	0.0				
Approach LOS	A						
Intersection Summary					Lange		
Average Delay			1.3				
Intersection Capacity Utiliza	ation		13.3%	IC	U Level c	f Service	А
Analysis Period (min)			15				

Table 1
The Lamb Center
Similar Site Average Parking Occupancy

Property	Average Max Parking	Total Units	Bus Stop	Total	
Cloverleaf 2011	12	60	Adia and Dua Char	0.19	
Cloverleaf 2020	6	60	Adjacent Bus Stop	0.10	
Crescent Square 2020	15	90	No Adiacont Due Stan	0.18	
Crescent Square 2021	28	80	No Adjacent Bus Stop	0.36	
Crossings 2020	13	60	No Adjacent Bus Stop	0.22	
Church Street Stations 2021	20	80	Adjacent Bus Stop	0.26	
Gosnold Apartments 2011	13	<b>CO</b>	Adiacant Bus Chair	0.22	
Gosnold Apartments 2021	27	60	Adjacent Bus Stop	0.45	
South Bay 2011	13	60	Adjacent Bus Stop	0.22	
Average				0.24	