Transportation Impact Study

The Ox Fairfax – Block A

City of Fairfax, Virginia

January 26, 2024

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Executive Summary

The following report presents the findings of a Transportation Impact Study (TIS) conducted for the proposed redevelopment of The Ox Fairfax – Block A site in the City of Fairfax, Virginia. This study was developed in accordance with guidelines and recommendations set forth by the City of Fairfax and VDOT.

This study was prepared in accordance with the best professional practices and standards in order to assess the impact of the proposed redevelopment on the surrounding transportation systems and recommend improvements to lessen or negate those impacts. This study involves the evaluation of anticipated roadway conditions with and without the proposed redevelopment and recommends possible transportation improvements and strategies to offset both the impacts of the increase in future traffic demand and the changes in traffic operations and characteristics due to the redevelopment. This study serves to assist public officials and developers to balance interrelations between efficient traffic movements with necessary access.

Site Location and Study Area

The site is located north of Armstrong Street, south of Sager Avenue, west of University Drive, and east of Chain Bridge Road in the City of Fairfax, Virginia. The site is located in the Old Town Fairfax Activity Center within the Old Town Fairfax Historic District.

For the purposes of this study, the analysis presented herein includes 17 existing intersections and two (2) future intersections.

The study intersections are as follows:

- 1. Main Street and West Street/North Street
- 2. Chain Bridge Road and North Street
- 3. University Drive and North Street
- 4. North Street and Blenheim Boulevard
- 5. Main Street and East Street/Blenheim Boulevard
- 6. Main Street and University Drive
- 7. Main Street and Chain Bridge Road
- 8. Chain Bridge Road and Sager Avenue
- 9. Sager Avenue and Site Entrance
- 10. University Drive and Sager Avenue
- 11. Sager Avenue and East Street/South Street
- 12. University Drive and South Street
- 13. University Drive and Parking Lot
- 14. University Drive and Armstrong Street
- 15. Chain Bridge Road and Armstrong Street
- 16. Chain Bridge Road and Judicial Drive
- 17. Chain Bridge Road and Humane Society Entrance
- 18. Chain Bridge Road and South Street (Future)
- 19. South Street and West Street (Future by Others)

Description of Proposed Development

The site is currently developed with 90,421 SF of office and other non-residential uses split between three (3) buildings. The proposed program is a mixed-use development comprised of up to a 4,127-person capacity (2,796 fixed seat) concert hall with a 117 seat black box theater, a 169-room hotel with 26,127 SF of conference space, 6,049 SF of office, two (2) art galleries totaling 8,399 SF, 13,912 SF of restaurant space, and 13,912 SF of retail. Total site build-out is planned for the year 2026.

Principal Findings and Conclusions

Discussions regarding the study assumptions and relevant background information were held with the City of Fairfax and VDOT staff during a scoping meeting on March 30, 2023. A copy of the signed scoping document is included in Appendix A.

The analysis presented in this report supports the following major finding:

• After the addition of redevelopment traffic, one (1) intersection has movements that begin to operate below acceptable levels of service as compared to the Future Conditions without Development (2026), after mitigations.

Additional assumptions, findings, and conclusions are as follows:

TIA Components

- As determined based on discussions with the City, a growth rate of one (1) percent per year was applied to major movements at the study intersections to account for regional growth in background traffic volumes.
- A Mode Split/TDM reduction of fifteen (15) percent was applied to the theater (attendees and employees), hotel, and office uses, as agreed upon with the City.
- The site is expected to generate approximately 259 trips in the AM peak hour, 511 trips in the PM peak hour, and 580 trips in the Saturday peak hour, after reductions.

Infrastructure

- Existing vehicular access is provided via two (2) driveways on Chain Bridge Road, two (2) driveways on Sager Avenue, and two (2) driveways on University Drive.
- With redevelopment of the property, site access will be provided via Sager Avenue and University Drive.
- The proposed South Street Extension will reroute traffic and relieve congestion on Main Street through Old Town and will ultimately provide access to The Ox Fairfax Block A site. The extension is expected to be complete in 2031 and is to be built by others.

Non-SOV Elements

- Sidewalks and curb ramps generally exist along the corridors adjacent to and within the vicinity of the site.
- A dedicated on-street bike lane currently exists on University Drive adjacent to the site.
- Five (5) bus routes provide service in the vicinity of the site, providing regional access to the area.

Analysis Results

- Nine (9) intersections within the study area have movements that operate below acceptable levels of service under the Existing Conditions (2023).
- Ten (10) intersections within the study area have movements that operate below acceptable levels of service under Future Conditions without Development (2026).
- After the addition of redevelopment traffic, one (1) intersection has movements that begin to operate below acceptable levels of service as compared to the Future Conditions without Development (2026), after mitigations.

Proposed Mitigation

- Future with Development (2026) Mitigation measures have been identified to improve traffic operations in the Future with Development (2026) scenario. These improvements include the following:
 - o Intersection 1: Main Street and West Street/North Street
 - Adjust signal timings to improve operations (SAT Peak only).
 - o Intersection 5: Main Street and East Street/Blenheim Boulevard
 - Adjust signal timings to improve operations (PM Peak only).
 - Intersection 7: Main Street and Chain Bridge Road
 - Adjust signal timings to improve operations (PM Peak only).
 - o Intersection 12: University Drive and South Street
 - Install a traffic signal and add exclusive northbound left lane.

Introduction

The following report presents the findings of a Transportation Impact Study (TIS) conducted for the proposed redevelopment of The Ox Fairfax – Block A site in the City of Fairfax, Virginia. This study was developed in accordance with guidelines and recommendations set forth by the City of Fairfax and VDOT.

The proposed program is a mixed-use development comprised of up to a 4,127-person capacity (2,796 fixed seat) concert hall with a 117 seat black box theater, a 169-room hotel with 26,127 SF of conference space, 6,049 SF of office, two (2) art galleries totaling 8,399 SF, 13,912 SF of restaurant space, and 13,912 SF of retail. Total site build-out is planned for the year 2026.

The following tasks were completed as part of this study effort:

- A scoping meeting was held with City of Fairfax and VDOT staff on March 30, 2023, which included discussions about the parameters of the study and relevant background information. A copy of the scoping document is included in Appendix A.
- Existing conditions were observed in the field to verify roadway geometry, pedestrian and bicycle infrastructure, and traffic flow characteristics.
- Turning movement counts were collected at the study area intersections on Tuesday, April 25, 2023 during the morning and afternoon peak periods. Turning movement counts were also collected at the study intersections on Saturday, April 29, 2023 and Saturday, May 6, 2023 from 10:00 AM to 2:00 PM.
- Vehicular traffic analysis for the study intersections was performed using *Synchro 11* based on Highway Capacity Manual (HCM) 6th Edition methodology. HCM 2000 methodology was used for all intersections in lieu of HCM 6th Edition if HCM 6th Edition methodology was not applicable.
- Intersection capacity analyses were performed for existing conditions (2023), interim future conditions (2026), ultimate future conditions (2031), and the design year (2032).
- Future traffic volumes were developed by accounting for regional growth in the area and background developments and roadway improvements. A growth rate of one (1) percent per year was applied to the existing volumes to account for a regional increase in background traffic.
- Proposed site traffic volumes were generated based on the methodology outlined in ITE *Trip Generation*, 11th Edition.
- An assessment of the previous crashes has been conducted at existing study intersections.

Sources of data for this study include Institute of Transportation Engineers (ITE), Virginia Department of Transportation (VDOT), City of Fairfax, and the office files and field reconnaissance efforts by Gorove Slade.

Background Information: Proposed Development (Site and Nearby)

Description of the Existing Site

Site Location

The site is located north of Armstrong Street, south of Sager Avenue, west of University Drive, and east of Chain Bridge Road in the City of Fairfax, Virginia. The site is located in the Old Town Fairfax Activity Center within the Old Town Fairfax Historic District.

The geographic scope of the study area was developed in accordance with City of Fairfax guidance. Figure 1 shows the location of existing and future study intersections.



Figure 1: Study Intersections

Description of the Parcel

The Ox Fairfax – Block A site is approximately 3.589 acres and is comprised of three (3) parcels, which are identified on the City of Fairfax Tax Map as Tax Map #57-04-02-044, #57-04-02-045, and #57-04-02-046. The parcel map is shown in Figure 2.



Figure 2: Parcel Map

Location within Jurisdiction and Region

The site is located in the Old Town Fairfax Activity Center within the Old Town Fairfax Historic District in the City of Fairfax as shown in Figure 3. Specifically, the site is included within the boundaries of the Old Town Fairfax Small Area Plan.



Figure 3: Jurisdiction Location (Source: City of Fairfax 2035 Comprehensive Plan)

Comprehensive Plan Recommendations

According to the City of Fairfax 2035 Comprehensive Plan, this site is planned for the Activity Center Place Type. The Activity Center Place Type applies to locations in the City where pedestrian-oriented, mixed-use development is strongly encouraged. The Old Town Fairfax Activity Center encompasses a cultural hub for the City, with a concentration of historic buildings, public services, active open space, and commercial buildings. Old Town Fairfax can also capitalize on its proximity to George Mason University to attract university supported businesses and arts and entertainment venues.

Zoning for the Site and Nearby Uses

The existing zoning for the site is CR (Commercial Retail) as shown in Figure 4.



Figure 4: Zoning Map

Description of Geometric Scope and Limits of the Study Area

The geographic scope of the study area was developed in accordance with the City of Fairfax guidance.

Existing Roadways

The site is located north of Armstrong Street, south of Sager Avenue, west of University Drive, and east of Chain Bridge Road in the City of Fairfax, Virginia.

The existing study area includes seventeen (17) intersections along Chain Bridge Road, Sager Avenue and University Drive.

Detailed roadway descriptions are provided in the Existing Conditions (2023) section of this study.

The vehicular study area includes the following existing intersections:

- 1. Main Street and West Street/North Street
- 2. Chain Bridge Road and North Street
- 3. University Drive and North Street
- 4. North Street and Blenheim Boulevard
- 5. Main Street and East Street/Blenheim Boulevard
- 6. Main Street and University Drive
- 7. Main Street and Chain Bridge Road
- 8. Chain Bridge Road and Sager Avenue
- 9. Sager Avenue and Site Entrance
- 10. University Drive and Sager Avenue
- 11. Sager Avenue and East Street/South Street
- 12. University Drive and South Street
- 13. University Drive and Parking Lot
- 14. University Drive and Armstrong Street
- 15. Chain Bridge Road and Armstrong Street
- 16. Chain Bridge Road and Judicial Drive
- 17. Chain Bridge Road and Humane Society Entrance

Planned Future Transportation Improvements

South Street Extension

The City's recommended extension of South Street to West Street between University Drive and Chain Bridge Road will reroute traffic and relieve congestion on Main Street through Old Town. The extension will also permit the continuation of pedestrian facilities through Old Town. The proposed South Street Extension is expected to be complete in 2031 and is to be built by others. The extension will ultimately provide access to The Ox Fairfax – Block A site. For the purposes of this study, an ultimate scenario was evaluated with the South Street Extension in place through The Ox Fairfax – Block A site.

Old Town Streetscape Plan & Standards and Main Street Streetscape Design

The Main Street Streetscape Design is part of an overall effort to prepare an Old Town Streetscape Plan and Standards that would improve the appearance and experience of Old Town Fairfax as a destination. These improvements are to be completed by others.

University Drive Road Diet

This is a project to modify the roadway configuration of University Drive between South Street and Layton Hall Drive. The project will extend the redesign that previously converted University Drive from four lanes to three travel lanes with on-street bike lanes between Armstrong Street and South Street. The proposed project will install on-street bike lanes and sharrows between North Street and Layton Hall Drive to help improve pedestrian and bicycle safety. These improvements are to be completed by others.

Transit Improvements

According to the City of Fairfax 2035 Comprehensive Plan, bus improvement and bus transfer improvements are proposed in the vicinity of the site. Figure 5 shows the City's proposed transit network enhancements.



Figure 5: Proposed Transit Network Enhancements (Source: City of Fairfax 2035 Comprehensive Plan)

Bicycle and Pedestrian Improvements

According to the City of Fairfax 2035 Comprehensive Plan, on-street bike facilities and concentrated bicycle supportive infrastructure are proposed along and near Main Street and University Drive in the vicinity of the site. Figure 6 shows the City's proposed network for bicycle travel.



Figure 6: Proposed Network for Bicycle Travel (Source: City of Fairfax 2035 Comprehensive Plan)

Existing Conditions (2023)

In order to project future traffic conditions, it was necessary to create an existing scenario.

A site visit was conducted in order to capture existing conditions along the site perimeter and written descriptions of the conditions observed, noting any deficiencies and substandard conditions of the multimodal facilities present or lacking.

The site visit, which involved documenting elements within up to a half-mile radius in the vicinity of the site, occurred on Thursday, July 13, 2023.

The site visit indicates locations with which to improve the multimodal connectivity aspects in and around the site and provide suggestions with which to improve upon any discontinuous facility segments, lack of a crosswalks, or other places with low levels of comfort in the vicinity of the site.

Existing Transit Service

Five (5) bus routes currently serve the site area on Main Street, Chain Bridge Road (Route 123), and University Drive. Bus service is provided by City of Fairfax CUE Gold and Green Routes, Metrobus Routes 29K and 17G, and Fairfax Connector Route 306. The existing bus routes are shown in Figure 7 through Figure 10.



Figure 7: Existing CUE Bus Routes (Source: City of Fairfax)



Figure 8: Existing Metrobus Route 29K (Source: WMATA)



Figure 9: Existing Metrobus Route 17G (Source: WMATA)



Figure 10: Existing Fairfax Connector Route 306 (Source: Fairfax Connector)

Existing Bicycle Facilities

The roadways adjacent to the site are considered comfortable bicycling routes per the Fairfax County Bicycle Map (which includes the City of Fairfax). Sager Avenue is considered "Most Comfortable", University Drive is considered "Somewhat Comfortable", and Chain Bridge Road is considered "Less Comfortable". Main Street is considered a "Use Caution" bicycling route. A dedicated on-street bike lane currently exists on University Drive adjacent to the site.

The 10-minute, 20-minute, and 30-minute bicycle travel shed for the proposed development is shown in Figure 11. Within a 10minute bicycle ride, the proposed development has access to several destinations including George Mason University, public transportation stops, residential neighborhoods, retail zones, and community amenities. Within a 20-minute bicycle ride, the proposed development has access to destinations in Fairfax County including residential neighborhoods and retail zones. Within a 30-minute bicycle ride, the proposed development has access to the Town of Vienna, the Mosaic District, and is accessible to the Vienna/Fairfax-GMU Metro station served by the Orange line and the Burke Centre Amtrak/VRE Station.



Figure 11: Approximate Bicycle Travel Times

Existing Pedestrian Facilities

Sidewalks and curb ramps generally exist along the corridors adjacent to and within the vicinity of the site. Sidewalks exist on one side of Chain Bridge Road and both sides of University Drive and Sager Avenue along the perimeter of the site, but most driveways lack crosswalks. The signalized intersection of University Drive and Sager Avenue has marked crosswalks with pedestrian signal heads and call buttons in place. The unsignalized intersections of Chain Bridge Road and Sager Avenue and University Drive & Fairfax Volunteer Fire Department Entrance have marked crosswalks in place.

The existing pedestrian infrastructure facilities, including curb ramps, marked striped crossings, and any observed deficiencies are depicted in Figure 12. Of note, this graphic includes these items within a quarter-mile radius walkshed of the site.

The 10-minute, 20-minute, and 30-minute walk travel shed for the proposed development is shown in Figure 13. Within a 10minute walk, the proposed development has access to several destinations including the Fairfax County Judicial Center, the City of Fairfax Regional Library, public transportation stops, nearby residential neighborhoods, and retail zones. Within a 20minute walk, the proposed development has access to destinations including City Hall, residential neighborhoods, and retail zones. Within a 30-minute walk, the proposed development has access to destinations including the Stacy C. Sherwood Community Center, United States Postal Service, and residential neighborhoods.



Figure 12: Existing Pedestrian Facilities



Figure 13: Approximate Pedestrian Travel Times

Existing Roadway Network

A description of the major roadways within the study area is presented in Table 1. The existing lane configurations and traffic control devices at the study intersections are shown in Figure 14.

Table 1: Existing Road Network

Roadway	From	То	VDOT Classification	Lanes	Speed (mph)	On-Street Parking	2019 AADT (vpd)*	2022 AADT (vpd)*
Main Street	Fairfax Boulevard	West Street	Other Principal Arterial	4	25-35	No	38,000	33,075
Main Street	West Street	Old Lee Highway	Other Principal Arterial	2	25	Yes	11,000	9,419
Chain Bridge Road	SCL Fairfax	Judicial Drive	Other Principal Arterial	4	25-30	No	28,000	14,602
Chain Bridge Road	Judicial Drive	Main Street	Other Principal Arterial	4	25	No	22,000	12,517
Sager Avenue	Chain Bridge Road	Dwight Avenue	Local Road	2	25	Yes	2,700	5,752
Judicial Drive	Main Street	Page Avenue	Major Collector	4	25	No	11,000	7,784
Judicial Drive	Page Avenue	Chain Bridge Road	Major Collector	2	25	No	9,000	9,840

* VDOT Annual Average Daily Traffic (AADT) Data

Historical Crash Data

Historical crash data at the existing study intersections was obtained from VDOT for 2017 through 2022. Crash data for the most recent six (6) years (January 2017 to December 2022) is summarized in Table 2.

Number of Total Number of Property Number of Fatal Intersection Crash Rate Number of Intersection Crashes **Damage Crashes** Crash (Crashes/million entering veichles) Crashes **Resulting in Injury** Main Street and West Street/North Street 0.43 29 24 5 0 Chain Bridge Road and North Street 8 0 0.13 2 11 3 University Drive and North Street 14 13 1 0 0.21 0 4 North Street and Blenheim Boulevard 12 9 3 0.18 Main Street and East Street/Blenheim Boulevard 0 0 0.09 7 7 12 0 6 Main Street and University Drive 13 1 0.33 Main Street and Chain Bridge Road 0.08 5 3 2 0 8 Chain Bridge Road and Sager Avenue 0 0 0 0 0.00 Sager Avenue and Site Entrance 0 0 0 0 0.00 10 University Drive and Sager Avenue 15 8 7 0 0.70 11 Sager Avenue and East Street/South Street 1 1 0 0 0.06 0 12 University Drive and South Street 2 1 1 0.10 13 University Drive and Parking Lot 0 0 0 0 0.00 0 14 University Drive and Armstrong Street 3 2 1 0.13 15 Chain Bridge Road and Armstrong Street 0 0.03 2 16 Chain Bridge Road and Judicial Drive 5 5 0 0 0.09 Chain Bridge Road and Humane Society Entrance 17 0 0 0 0 0.00 Total 119 94 25 0

Table 2: Crash Data (January 2017 – December 2022)

* There were one and three pedestrian injury crashes included in number of crashes resulting in injury at Intersection 3 and Intersection 10 respectively

As shown in Table 2, the intersection of Main Street and West Street had the highest number of reported crashes (29) during the six-year study period. The most common crash type was rear end.

The intersection of University Drive and Sager Avenue had the second highest number of reported crashes (15). The most common crash type was angle.

All existing intersections have a crash rate of less than 1.0 crashes per Million Entering Vehicles (MEV) and are not considered high crash intersections.

The crash rates shown for each intersection are calculated as crashes per one million entering vehicles (MEV), and were calculated based on the following formula:

$$Rate_{intersection} = \frac{1,000,000 * \# of Crashes}{\# of Years * 365 \left(\frac{days}{year}\right) * ADT_{approach}}$$

The approach ADT for an intersection is calculated as the sum of each leg, divided by 2.

The crash data provided by VDOT is included in Appendix B.

Existing Traffic Volumes

Turning movement counts were collected at the study area intersections on Tuesday, April 25, 2023, Saturday, April 29, 2023, and Saturday, May 6, 2023. Analysis of the traffic data found the following system peak hours:

- Weekday Morning (AM) Peak Hour: 7:45 AM to 8:45 AM
- Weekday Afternoon (PM) Peak Hour: 4:45 AM to 5:45 PM
- Weekend (Saturday) Peak Hour:12:45 PM to 1:45 PM

The existing peak hour traffic volumes for the study area intersections are presented in Figure 15. The existing turning movement counts are included in Appendix C.



Figure 14: Existing (2023) – Lane Configuration



Figure 15: Existing (2023) – Peak Hour Traffic Volumes

Existing (2023) Intersection Capacity Analysis

Intersection capacity analysis was performed at the intersections within the study area during the weekday AM, weekday PM, and Saturday peak hours under Existing Conditions (2023). *Synchro Version 11* was used to analyze the study intersections based on the *Highway Capacity Manual* (HCM) methodology and includes level of service (LOS), delay, and queue length for the turning movements analyzed.

Existing signal timings were provided by the City and used as a base for the existing analysis. Existing peak hour factors found in the field were used, except where the field peak hour factor was lower than 0.85 in which case a minimum value of 0.85 was used, consistent with VDOT analysis guidelines. Heavy vehicle percentages determined by existing traffic counts were used for analysis. Pedestrian volumes were also collected during the intersection traffic counts and included in the analysis.

SimTraffic was used to run the simulations for the analysis in order to determine the queue. The same network files that were used for the Synchro analysis were used as input for the SimTraffic analysis. As noted in the scoping document, SimTraffic was calibrated for the traffic volumes entering the roadway network. Consistent with the guidelines set forth in VDOT's *Traffic Operations and Safety Analysis Manual* (TOSAM), Table 3 below shows the parameters that were used for the simulation. Per Section 7.6 of the TOSAM all other parameters not addressed in the table should not be modified from the default value.

Table 3: SimTraffic Analysis Input Parameters

	SimTraffic (Version 11) Analysis Input Parameters
Number of Intervals	One seeding interval and four 15 minute recording intervals
Seeding Interval Duration	15 minutes
PHF Adjust	 Selected "Yes" for one of the four, 15 minute recording intervals Selected "No" for all other intervals
Anti-PHF Adjust	 Selected "Yes" for three, 15 minute recording intervals where PHF Adjust is set to "No" Selected "No" for the recording interval where PHF Adjust is set to "Yes"
Number of Runs	10

A seeding period is necessary to ensure that the results obtained are not skewed because the network is void of any vehicles prior to seeding. Foregoing seeding would lead to lower travel times and delays for the traffic at the beginning of the simulation. The network reaches a normal state during the seeding period without affecting the results of the simulation.

The results of the intersection capacity analysis are presented in Table 4 and are expressed in LOS and delay (seconds per vehicle) per lane group. The average and maximum queue results from SimTraffic are presented in Table 4 and are expressed in feet. Level of service results are also presented in Figure 16. The detailed analysis worksheets are included in Appendix D.

Table 4: Existing (2023) – Intersection Capacity Analysis

				AM Peak H	lour		PM Peak	Hour	SAT Peak Hour		
No.	Intersection (Movement)	Effective Storage Length (ft.)	LOS	Delay (sec/veh)	Ave. Max Queue (ft.)	LOS	Delay (sec/veh)	Ave. Max Queue (ft.)	LOS	Delay (sec/veh)	Ave. Max Queue (ft.)
		19	Sy	nchro	SimTraffic	Syr	nchro	SimTraffic	Sy	nchro	SimTraffic
1	Main Street (E/W) and West										
	Street/North Street (N/S)										
	Overall Intersection (Signalized)	005	C	30.0	205	C	31.3	242	C	29.8	074
	Eastbound Leit	665	C	29.0	325	C	30.1	312	C	32.8	271
	Easthound Right	125	Ċ	32.0	400	C	26.6	400	C	24.9	493
	Westbound Left	125	D	39.9	123	D	50.2	92	c	34.6	32
	Westbound Thru/Right		c	33.5	266	F	94.9	308	D	44.3	290
	Northbound Left/Thru Thru/Right		E	65.9	107	E	64.1	144	E	57.5	89
	Southbound Left/Thru		D	45.9	202	F	81.0	70	D	38.2	80
2	Southbound Right		С	24.7	315	В	11.9	21	В	18.8	315
-	Street (E/W)										
	Overall Intersection (Signalized)		в	17.1		D	45.2		в	15.1	
	Eastbound Thru Thru/Right		A	5.6	258	В	11.7	186	В	19.2	228
	Westbound Thru Thru/Right		В	14.0	264	D	50.9	343	A	6.5	140
	Northbound Thru Thru/Right	100	С	21.0	288	F	84.8	292	В	15.5	269
	Southbound Lett	160	C	30.5	144	C	29.6	160	C	23.3	156
3	University Drive (N/S) and North Street		C	23.0	452	0	51.2	020	0	20.2	231
	(E/W)										
1	Overall Intersection (Signalized)	_	В	17.1		С	22.1		В	18.9	
1	Eastbound Lett	215	A	4.5	179	C	21.4	191	В	11.1	120
1	Westhound Left	150	A 	1.9	208		∠1.1 6.1	2//	B	14.1	134
1	Westbound Thru Thru/Right		A	2.6	223	В	16.2	386	c	22.4	288
	Northbound Left/Thru Thru/Right		E	71.9	200	В	16.0	99	Α	9.6	87
<u> </u>	Southbound Left/Thru Thru/Right		E	69.1	230	D	41.2	365	с	30.5	218
4	North Street (E/W) and Blenheim										
	Overall Intersection (Signalized)		c	30.0		c	31.0		c	26.8	
	Eastbound Left/Thru		F	90.9	140	F	97.7	140	D	45.8	140
	Eastbound Right		A	2.0	323	A	2.8	295	В	15.5	297
	Westbound Right		А	0.0	0	A	0.1	0	А	0.1	0
	Northbound Left		В	14.3	318	В	14.3	405	В	18.7	376
	Northbound Left/Thru/Right		В	14.1	367	В	14.2	381	В	18.7	374
	Southbound Left/Thru Thru		E	73.0	237	F	83.4	634	D	52.8	219
5	Main Street (F/W) and Fast	750	E	57.4	164	D	51.1	632		45.5	160
-	Street/Blenheim Boulevard (N/S)										
	Overall Intersection (Signalized)		с	23.7		с	27.3		с	24.7	
	Eastbound Thru Thru/Right		С	34.6	282	D	51.1	315	D	47.9	310
	Westbound Thru		D	42.2	224	D	52.7	225	С	32.5	224
	Westbound Right		A	3.4	309	A	8.4	602	В	15.3	423
	Northbound Left	105	D	51.4	92	D	51.3	104	D	48.9	71
	Southbound Left		B	17.6	201	B	12.0	275		53.3 7.6	290
	Southbound Thru/Right		D	41.8	243	D	40.9	292	c	20.7	266
6	Main Street (E/W) and University Drive										
	(N/S) Overall Intersection (Signalized)		<u> </u>	20.4			46.2			49.0	
	Easthound Left	00		20.4	40	В	16.3	60	в	18.0	60
	Eastbound Thru/Right	30	Ā	3.0	120	B	15.4	197	Ā	5.6	185
	Westbound Left	125	A	0.1	83	A	8.0	90	A	2.3	81
	Westbound Thru/Right		A	0.7	43	В	10.3	128	А	3.3	124
	Northbound Left/Thru Thru/Right		E	62.3	102	С	28.4	103	D	53.9	103
	Southbound Left/Thru Thru/Right		С	30.4	160	В	12.3	132	D	36.1	197
	Road (N/S)										
	Overall Intersection (Signalized)		с	28.5		D	37.4		с	20.5	
	Eastbound Left	125	A	9.4	53	A	9.7	90	Α	5.1	89
	Eastbound Thru/Right		В	13.3	259	В	17.2	301	В	14.9	296
	Westbound Left	200	E	74.2	167	F	87.2	196	E	73.0	165
	Westbound Thru/Right		В	14.8	190	В	10.2	264	В	10.0	140
	Southbound Left/Thru Thru/Right		B	44.3	322	D	43.9	288	Δ	27.2	292
8	Chain Bridge Road (N/S) and Sager										
	Avenue (E/W)										
	Overall Intersection (Unsignalized)		_					100			=0
	Southbound Left/Thru Thru		В	12.4	87	в	11.7	128	В	11.6	79
9	Sager Avenue (E/W) and Site Entrance		~	2.1	104	~	2.2	215	A	2.0	90
	(N/S)										
	Overall Intersection (Unsignalized)										
1	Westbound Left/Thru		A	1.2	24	A	0.7	21	A	0.9	15
10	Induction Lettickight		A	9.0	24	A	9.3	52	A	9.2	41
10	Avenue (E/W)										
1	Overall Intersection (Signalized)		в	11.2		в	10.2		А	10.0	
1	Eastbound Left/Thru/Right		С	33.1	119	D	36.2	182	С	24.9	104
1	Westbound Left	130	С	29.5	102	С	27.7	102	С	24.5	49
1	Westbound Thru/Right		С	30.1	135	С	27.6	131	С	25.5	117
1	Northbound Left/Thru Thru/Right		A	2.8	84	A	3.7	97	A	3.2	80
L	Southbound Lett/Thru Thru/Right		A	0.7	81	A	1.1	111	A	1.8	80

				AM Pea <u>k H</u>	our			our		SAT Peak Hour		
No.	Intersection (Movement)	Effective Storage Length (ft.) [1]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) ^[4]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) ^[4]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) [4]	
			Syı	nchro	SimTraffic	Syr	nchro	SimTraffic	Syr	ichro	SimTraffic	
11	Sager Avenue (E/W) and South Street/East Street (N/S)											
	Overall Intersection (Unsignalized)	00					0.7	0.0	•	7.0	45	
	Eastbound Thru/Right	90	A	0.2	55	B	9.7	251	A	7.9	45	
	Westbound Left	80	Δ	7.8	63	Δ	9.1	201	A 4	7.8	40	
	Westbound Thru/Right	00	Å	9.7	104	B	11.4	397	A	8.8	40	
	Northbound Left/Thru/Right		Δ	89	74	B	11.3	380	Δ	8.4	64	
	Southbound Left/Thru/Right		A	9.3	147	B	13.2	252	A	9.1	120	
12	University Drive (N/S) and South Street			0.0			10.2	202	~	0.1	120	
	(E/W)											
	Overall Intersection (Unsignalized)											
	Westbound Left/Right		В	13.9	72	С	17.3	92	В	11.7	64	
	Southbound Left		A	8.0	25	A	8.3	21	A	7.8	9	
13	University Drive (N/S) and Parking Lot/Fire Station #3 Entrance (E/W)											
	Overall Intersection (Unsignalized)											
	Eastbound Left/Thru/Right		В	11.2	33	В	12.7	46	В	10.2	33	
	Southbound Left	85	A	7.9	2	A	8.3	5	A	9.0	10	
14	Boulevard (N/S) and Armstrong Street (E/W)											
	Overall Intersection (Signalized)		в	11.8		в	12.5		в	11.4		
	Eastbound Left/Thru/Right		В	16.7	130	С	20.1	128	В	19.5	114	
	Westbound Left/Thru/Right		В	15.6	91	В	19.0	118	В	16.5	87	
	Northbound Left	160	A	7.8	31	A	7.4	65	A	6.8	30	
	Northbound Thru/Right		В	10.2	31	A	9.6	177	A	8.2	120	
	Southbound Left	230	A	7.8	37	A	7.2	79	A	6.8	120	
45	Southbound Thru/Right		В	10.5	194	В	11.3	232	A	8.5	141	
15	Chain Bridge Road (N/S) and											
	Armstrong Street (E/W) Overall Intersection (Signalized)		^	73		в	17.0		•	8.1		
	Westbound Left/Right		2	65.0	182	Ē	60.5	283	ĥ	36.0	114	
	Northbound Thru Thru/Right		Ā	5.0	227	Ċ	22.0	376	A	9.0	236	
	Southbound Left	80	A	9.3	79	A	2.5	73	Δ	3.7	76	
	Southbound Thru	00	A	2,1	215	A	2.3	252	A	3.4	186	
16	Chain Bridge Road (N/S) and Judicial Drive (E/W)				210		2.0	202		0.1		
	Overall Intersection (Signalized)		в	15.0		в	15.4		в	10.2		
	Eastbound Left	410	E	70.6	153	E	66.3	170	E	59.7	90	
	Eastbound Right		D	50.4	85	D	45.6	117	D	48.0	82	
	Northbound Left	160	А	5.4	160	В	15.8	161	Α	1.8	135	
	Northbound Thru		А	5.7	289	A	1.4	282	A	1.8	162	
	Southbound Thru		В	19.7	252	A	8.3	174	A	5.2	182	
	Southbound Right	240	D	37.1	117	A	5.9	47	A	4.3	54	
17	Chain Bridge Road (N/S) and Humane											
	Society Driveway (E/W)											
1	Overall Intersection (Unsignalized)					L .			I .			
	Westbound Left/Right		A	0.0	0	A	0.0	0	С	15.0	40	
L	ουμιρουπα μεπ	90	A	0.0	0	A	0.0	0	A	9.3	21	

[1] Effective storage length is based on the storage length plus one-half of the taper length per TOSAM guidelines.
 [2] #: 95th percentile queues (reported from Synchro) exceed capacity; actual queues may be longer. Queues shown are based on the maximum after two cycles.

[3] m: 95th percentile volume and queues (reported from Synchro) are metered by upstream signal.
 [4] Max queues are based on results from SimTraffic. Per TOSAM guidelines, the queues are based on the average to 10 simulations.
 [5] 50th Percentile Queues are not reported for TWSC intersections under HCM Methodology.

For the purpose of this analysis, it is desirable to achieve a level of service (LOS) of D or better for each lane group at the study intersections.

The intersection capacity analysis results show that the following nine (9) intersections have movements that operate below the acceptable levels of service during one or more peak hours under Existing Conditions (2023):

- Intersection 1: Main Street and West Street/North Street
 - Westbound Thru/Right (PM Peak)
 - Northbound Left/Thru, Thru/Right (AM, PM, and SAT Peaks)
 - Southbound Left/Thru (PM Peak)
- Intersection 2: Main Street and North Street
 - Northbound Thru, Thru/Right (PM Peak)
- Intersection 3: University Drive and North Street
 - Northbound Left/Thru, Thru/Right (AM Peak)
 - Southbound Left/Thru, Thru/Right (AM Peak)
- Intersection 4: North Street and Blenheim Boulevard
 - o Eastbound Left/Thru (AM and PM Peaks)
 - Southbound Left/Thru, Thru (AM and PM Peaks)
 - Southbound Right (AM Peak)
- Intersection 5: Main Street and East Street/Blenheim Boulevard
 - Northbound Thru, Thru/Right (AM and PM Peaks)
- Intersection 6: Main Street and University Drive
 - Northbound Left/Thru, Thru/Right (AM Peak)
- Intersection 7: Main Street and Chain Bridge Road
 - Westbound Left (AM, PM, and SAT Peaks)
- Intersection 15: Chain Bridge Road and Armstrong Street
 - Westbound Left/Right (AM and PM Peaks)
- Intersection 16: Chain Bridge Road and Judicial Drive
 - Eastbound Left (AM, PM, and SAT Peaks)



Figure 16: Existing (2023) – Levels of Service

Future Conditions without Development (2026)

Future without Development (2026) Traffic Volumes

The proposed The Ox Fairfax – Block A redevelopment is anticipated to be complete in 2026. The future background traffic volumes were projected by increasing the existing volumes to 2026 using an inherent growth rate. Historical ADT data is shown in Table 5. As determined based on discussions with the City, a growth rate of one (1) percent per year was applied to existing traffic volumes on major movements as shown in Figure 17.

Table 5: Historical Growth Rate

				AADT						Annual % Change
Route	From	То	2015	2016	2017	2018	2019	2021	(2015-2019)	(2015-2021)
Main Street	Fairfax Boulevard	West Street	35,000	35,000	36,000	38,000	38,000	32,000	2.1%	-1.8%
Main Street	West Street	Old Lee Highway	13,000	13,000	11,000	11,000	11,000	8,900	-4.1%	-7.3%
Chain Bridge Road	Judicial Drive	Main Street	20,000	21,000	21,000	21,000	22,000	14,000	2.4%	-7.8%
University Drive	Armstrong Street	South Street	14,000	14,000	14,000	14,000	15,000	12,000	1.7%	-3.0%
University Drive	South Street	Main Street	12,000	12,000	11,000	11,000	11,000	5,700	-2.2%	-13.8%
Sager Avenue	Chain Bridge Road	Dwight Avenue	2,700	2,700	2,700	2,700	2,700	2,200	0.0%	-4.0%

Source: VDOT Traffic Data (http://www.virginiadot.org/info/ct-trafficcounts.asp)

In addition to the regional background growth, four (4) planned yet currently unbuilt developments in the vicinity of the site were taken into consideration. As discussed at the scoping meeting, the following developments were included in the 2026 analysis:

- One University One University is a 10.8-acre site is located adjacent to the George Mason University campus in Fairfax County. For the purposes of this analysis, the One University site was anticipated to include 120 affordable senior independent dwelling units, 120 affordable multifamily dwelling units, and 360 student housing dwelling units.
- West Drive Homes West Drive Homes is a 0.55-acre site located at 10642 West Drive in the City of Fairfax. For the purposes of this analysis, the West Drive Homes site was anticipated to include two (2) single-family homes and four (4) duplexes.
- Fairfax County Judicial Complex The Judicial Complex is a 47.8-acre portion of Fairfax County surrounded by the City of Fairfax. A new Master Plan for the complex was completed in January 2021. Building One of the redevelopment project was assumed to be in place by 2025. For the purposes of this analysis, Building One was anticipated to included 60,317 SF of government related uses (records and evidence storage) and 89,683 SF of office uses. It should be noted that the entitlement for Phase 1 of the Fairfax County Judicial Complex is not yet approved.
- City Centre West The City Centre West is a 2.12-acre site located adjacent to the Fairfax County Judicial Complex in the City of Fairfax. For the purposes of this analysis, the City Centre West site was anticipated to include 79 multifamily units, 27,793 SF of general office, 8,584 SF of medical office, 3,866 SF of retail, a 3,510 SF bank, and a 3,865 SF restaurant.

The background growth is shown in Figure 17 and the background development volumes are show in Figure 18.

The trips generated by background growth and background development were added to the existing volumes in order to generate the Future Conditions without Development (2025) traffic volumes presented in Figure 19.



Figure 17: Background Growth (2026)



Figure 18: Background Development (2026) – Peak Hour Traffic Volumes

Note: The volumes above reflect a total of the rerouting of existing trips, addition of pass-by trips of the background developments, and addition of the background development site trips, consistent with methodology of the respective traffic studies; therefore, volumes presented above may be shown as negative.
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208/301/164

4

9/15/13 🅕

18/6/9

44/105/47 ->

5/17/14

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15 16 17 18 19 480/832/577 0/0/4 527/1,112/725 460/885/592 63/127/50 €_ 0/0/11 ▲ 22/13/15 79/78/83 47/52/24 85/13/11 hain B 21/4/8 0/0/0 13/16/1 ← 0/0/0 ← 289/72/47 hain 0/0/7 39/95/50 • Ý 4 Intersection ↓ ↓ ¥ 4 ↓ ∢ ∢ ∢ 4 4 ial Drive th Stree th Stree Removed 4 1 1 1 1 ۶ 17/45/7 226/72/51 -927/704/670 -0/0/5 -4/22/6 🔶 58/47/54 66/95/28 -1,352/823/833 1,093/670/700 320/284/186 **beo**s 0/0/0 -> 30/101/25 149/307/211 0/0/0 ridoe

Figure 19: Future without Development (2026) – Peak Hour Traffic Volumes

0/0/0

1 4

0/0/0

Future without Development (2026) Intersection Capacity Analysis

Intersection capacity analysis was performed at the intersections within the study area during the weekday AM, weekday PM, and Saturday peak hours under Future Conditions without Development (2026). *Synchro Version 11* was used to analyze the study intersections based on the *Highway Capacity Manual* (HCM) methodology and includes level of service (LOS), delay, and queue length for the turning movements analyzed.

Signal phasing and timings were not changed from the existing conditions. A peak hour factor of 0.92 was used, unless the peak hour collected in the field was higher.

The results of the intersection capacity analysis are presented in Table 6 and are expressed in LOS and delay (seconds per vehicle) per lane group. The average and maximum queue results from SimTraffic are presented in Table 6 and are expressed in feet. Level of service results are also presented in Figure 20. The detailed analysis worksheets are included in Appendix E.

				AM Peak H	our		PM Peak H	our		SAT Peak H	lour
No.	Intersection (Movement)	Effective Storage Length (ft.) [1]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) ^[4]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) [4]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) [4]
			Sy	nchro	SimTraffic	Syr	nchro	SimTraffic	Syı	nchro	SimTraffic
1	Main Street (E/W) and West Street/North										
	Street (N/S)										
	Overall Intersection (Signalized)		с	33.7		С	34.9		D	39.4	
	Eastbound Left	665	С	31.0	350	С	33.9	378	D	36.9	654
	Eastbound Thru		D	35.3	510	D	40.5	511	D	40.4	756
	Eastbound Right	125	С	23.8	125	С	28.9	124	С	26.9	115
	Westbound Left	125	D	40.6	125	D	51.0	125	С	31.9	124
	Westbound Inru/Right		D	35.3	248	E	96.2	298	D	44.4	297
	Northbound Left/Thru Thru/Right		E	64.1	120	E	63.9	194	D	54.2	108
	Southbound Left/Thru		D	52.7	249	E	78.6	128	С	23.1	217
-	Chain Bridge Bood (N/S) and North Street		C	27.6	325	В	10.8	76	D	39.7	325
-	(F/W)										
	Overall Intersection (Signalized)		в	16.9		р	45.0		в	15.2	
	Eastbound Thru Thru/Right			7.8	258	В	13.9	256	B	16.2	249
	Westbound Thru Thru/Right		В	15.8	296	D	50.8	338	A	7.3	194
	Northbound Thru Thru/Right		в	16.8	287	F	82.6	300	В	16.9	274
	Southbound Left	160	c	30.0	136	C	30.7	160	c	23.9	159
	Southbound Thru Thru/Right		č	29.1	513	c	32.2	621	č	23.7	356
3	University Drive (N/S) and North Street					-	-				
	(E/W)										
	Overall Intersection (Signalized)		В	16.4		c	24.6		В	18.1	
	Eastbound Left	215	A	6.0	187	D	36.9	215	В	11.6	134
	Eastbound Thru Thru/Right		A	2.0	226	С	23.9	314	В	10.7	169
	Westbound Left	150	A	2.7	99	A	6.9	149	A	9.5	133
	Westbound Thru Thru/Right		A	3.2	236	В	17.9	388	С	22.9	334
	Northbound Left/Thru Thru/Right		E	59.8	197	В	16.5	218	В	10.1	89
	Southbound Left/Thru Thru/Right		E	68.5	228	D	42.4	766	С	31.0	233
4	North Street (E/W) and Blenheim										
	Overall Intersection (Signalized)		6	20.7			27.2		C	29.5	
	Easthound Left/Thru		6	02.0	140		155.2	140	C C	49.0	140
	Eastbound Bight		Δ.	2.5	326	Δ	2.6	359	в	40.0	302
	Westbound Right		A .	2.0	0	A .	0.1	11	Δ	0.1	0
	Northbound Left		B	14.1	343	B	14.8	385	ĉ	20.2	373
	Northbound Left/Thru/Right		в	13.0	367	в	14.0	386	C C	20.2	378
	Southbound Left/Thru Thru		F	71 7	235	F	89.0	636	D	52.1	227
	Southbound Right	750	F	57.6	164	D	51.3	624	D	45.2	190
5	Main Street (E/W) and East					_			_		
	Street/Blenheim Boulevard (N/S)										
	Overall Intersection (Signalized)		с	24.2		С	29.0		С	24.3	
	Eastbound Thru Thru/Right		D	37.2	304	E	55.9	299	D	46.5	317
	Westbound Thru		D	44.1	223	E	59.7	225	С	33.8	225
	Westbound Right		A	3.4	390	A	8.4	602	В	15.0	516
1	Northbound Left	105	D	51.4	90	D	51.3	104	D	49.1	78
	Northbound Thru Thru/Right		E	58.3	181	E	62.3	280	D	53.4	146
	Southbound Left		В	17.1	273	В	12.1	311	A	6.9	285
-	Southbound Thru/Right		D	41.3	250	D	42.1	285	В	18.8	263
0	(N/S)										
	Overall Intersection (Signalized)		c	21 0		R	16.4		R	17.6	
1	Eastbound Left	90	Ă	2.7	49	В	13.8	80	A	3.5	84
	Eastbound Thru/Right		A	4.3	160	В	17.3	198	A	6.1	231
1	Westbound Left	125	A	0.2	86	A	7.7	96	A	2.4	80
1	Westbound Thru/Right	.20	A	0.8	69	В	10.1	191	A	3.5	151
	Northbound Left/Thru Thru/Right		F	62.9	105	c	28.4	114	D	54.0	105
	Southbound Left/Thru Thru/Right		D	36.1	178	в	11.8	158	D	36.0	186
7	Main Street (E/W) and Chain Bridge Road										
	(N/S)										
	Overall Intersection (Signalized)		c	28.1		D	39.2		с	21.3	
1	Eastbound Left	125	A	9.8	63	В	12.6	119	A	6.2	124
1	Eastbound Thru/Right		В	13.1	249	C	23.2	307	В	15.2	311
	Westbound Left	200	E	75.2	167	F	91.1	198	E	76.1	190
1	Westbound Thru/Right		В	16.6	242	В	10.5	276	A	9.9	207
	Northbound Thru Thru/Right		D	42.0	323	D	44.1	322	С	27.9	298
-	Southbound Left/Thru Thru/Right		В	11.3	181	D	43.9	306	A	8.9	185
8	Chain Bridge Road (N/S) and Sager										
	Overall Intersection (Unsignalized)										
	Westbound Right		R	117	82	R	11 4	150	R	10.6	84
	Southbound Left/Thru Thru		Δ	25	166	۵ ۵	21	286	Δ	24	109
L				2.0				200		-	

 Table 6: Future without Development (2026) – Intersection Capacity Analysis

						PM Peak Hour					
		Effective Storage	LOS	Delav	Ave. Max	LOS	Delav	Ave. Max	LOS	Delav	Ave. Max
No.	Intersection (Movement)	Length (ft.)		(sec/veh)	Queue (ft.)		(sec/veh)	Queue (ft.)		(sec/veh)	Queue (ft.)
		[1]			[4]			[4]			[4]
			Syı	nchro	SimTraffic	Syn	nchro	SimTraffic	Syı	nchro	SimTraffic
9	Sager Avenue (E/W) and Site Entrance (N/S)										
	Overall Intersection (Unsignalized)										
	Westbound Left/Thru		A	1.3	33	А	0.8	54	A	0.9	26
40	Northbound Left/Right		A	9.0	30	A	9.2	80	A	9.1	58
10	(E/W)										
	Overall Intersection (Signalized)		в	10.9		в	10.1		Α	9.8	
	Eastbound Left/Thru/Right		С	31.4	117	D	36.2	187	С	24.7	96
	Westbound Left	130	C	29.5	80	C	27.7	104	C	24.5	51
	Northbound Left/Thru Thru/Right		A	2.8	81	<u>د</u>	3.7	135	A	25.5	86
	Southbound Left/Thru Thru/Right		A	0.8	91	A	1.1	117	A	1.7	80
11	Sager Avenue (E/W) and South										
	Overall Intersection (Unsignalized)										
	Eastbound Left	90	А	8.0	44	А	9.5	89	А	7.9	42
	Eastbound Thru/Right		А	7.7	72	A	10.4	251	А	7.8	65
	Westbound Left	80	A	7.7	59	A	9.0	80	A	7.7	46
	Westbound Inru/Right		A	9.2	99	A	11.0	400	A	8.7	116
	Southbound Left/Thru/Right		A	8.7 9.0	164	A	12.8	251	A	8.3 9.0	126
12	University Drive (N/S) and South Street										
	(E/W)										
	Westbound Left/Right		в	13.8	77	C	18.7	03	в	11.5	75
	Southbound Left		A	8.0	28	Ă	8.3	21	Ā	7.7	12
13	University Drive (N/S) and Parking										
	Lot/Fire Station #3 Entrance (E/W)										
	Eastbound Left/Thru/Right		в	11.0	30	в	12.3	53	в	10.0	38
	Southbound Left	85	А	7.9	2	А	8.2	9	А	8.9	5
14	University Drive/George Mason										
	Boulevard (N/S) and Armstrong Street (E/W)										
	Overall Intersection (Signalized)		в	11.7		в	13.0		в	11.4	
	Eastbound Left/Thru/Right		В	16.6	96	С	22.3	143	В	19.6	122
	Westbound Left/Thru/Right		В	15.7	96	С	20.5	124	В	16.5	87
	Northbound Left	160	A B	7.9	41	A	6.7	93 180	A	6.8	31
	Southbound Left	230	A	7.8	37	Â	7.5	54	A	6.8	43
	Southbound Thru/Right		В	10.4	184	В	11.7	259	А	8.6	162
15	Chain Bridge Road (N/S) and Armstrong										
	Street (E/W) Overall Intersection (Signalized)		Δ	83		в	17 1		۵	89	
	Westbound Left/Right		E	65.0	184	E	69.7	356	D	35.3	161
	Northbound Thru Thru/Right		A	6.0	362	С	24.0	485	В	10.3	294
	Southbound Left	80	В	19.9	77	A	2.4	78	A	4.5	79
16	Chain Bridge Boad (N/S) and Judicial		A	1.9	169	A	2.2	259	A	4.0	223
	Drive (E/W)										
	Overall Intersection (Signalized)		в	14.5		В	17.4		A	9.6	
	Eastbound Left	410	E	70.1	136	E	66.3	363	E	59.7 47 P	98
	Northbound Left	160	A	6.0	160	D	35.2	160	A	2.0	149
	Northbound Thru		A	6.2	292	A	1.5	312	A	1.9	196
	Southbound Thru		В	19.7	260	В	10.3	231	A	5.5	218
17	Southbound Right	240	С	34.5	136	A	7.3	69	A	4.4	78
	Society Driveway (E/W)										
	INTERSECTION REMOVED										
18	Chain Bridge Road (N/S) and South										
	Overall Intersection (Signalized)		А	3.4		A	9.3		A	7.4	
	Eastbound Left/Thru		E	66.5	66	E	65.0	129	E	62.4	41
	Eastbound Right		E	64.3	58	E	61.5	100	E	60.9	47
	Westbound Left/Thru/Right	222	A	0.0	0	A	0.0	0	A	0.0	60
	Northbound Thru Thru/Right	230	A	1.5	196	A	1.8	23U 412	A	∠.8 4.9	ວບ 155
	Southbound Left		A	0.0	0	A	0.0	0	A	0.0	30
L	Southbound Thru Thru/Right		А	1.2	97	А	6.9	248	А	6.0	288
19	West Street (N/S) and South Street (E/W)										
	Eastbound Left/Thru/Right		۸	0 0	15		1.2	34		10	6
	Southbound Left/Thru/Right		В	10.4	42	В	10.3	38	A	9.0	43
HOTEO											

[2] Effective storage length is based on the storage length plus one-half of the taper length per TOSAM guidelines.
[2] #: 95th percentile queues (reported from Synchro) exceed capacity; actual queues may be longer. Queues shown are based on the maximum after two cycles.
[3] m: 95th percentile volume and queues (reported from Synchro) are metered by upstream signal.
[4] Max queues are based on results from SimTraffic. Per TOSAM guidelines, the queues are based on the average to 10 simulations.
[5] 50th Percentile Queues are not reported for TWSC intersections under HCM Methodology.
[6] ~: Volume exceeds capacity, queue is theoretically infinite.

For the purpose of this analysis, it is desirable to achieve a level of service (LOS) of D or better for each lane group at the study intersections.

The intersection capacity analysis results show that the following ten (10) intersections have movements that operate below the acceptable levels of service during one or more peak hours under Future Conditions without Development (2026). Movements in **bold** operate below acceptable levels of service under Existing Conditions (2023):

- Intersection 1: Main Street and West Street/North Street
 - Westbound Thru/Right (PM Peak)
 - Northbound Left/Thru, Thru/Right (AM and PM Peaks)
 - Southbound Left/Thru (PM Peak)
- Intersection 2: Main Street and North Street
 - Northbound Thru, Thru/Right (PM Peak)
- Intersection 3: University Drive and North Street
 - Northbound Left/Thru, Thru/Right (AM Peak)
 - Southbound Left/Thru, Thru/Right (AM Peak)
- Intersection 4: North Street and Blenheim Boulevard
 - Eastbound Left/Thru (AM and PM Peaks)
 - o Southbound Left/Thru, Thru (AM and PM Peaks)
 - Southbound Right (AM Peak)
- Intersection 5: Main Street and East Street/Blenheim Boulevard
 - Eastbound Thru, Thru/Right (PM Peak)
 - Westbound Thru (PM Peak)
 - Northbound Thru, Thru/Right (AM and PM Peaks)
- Intersection 6: Main Street and University Drive
 - Northbound Left/Thru, Thru/Right (AM Peak)
- Intersection 7: Main Street and Chain Bridge Road
 - Westbound Left (AM, PM, and SAT Peaks)
- Intersection 15: Chain Bridge Road and Armstrong Street
 - Westbound Left/Right (AM and PM Peaks)
- Intersection 16: Chain Bridge Road and Judicial Drive
 - Eastbound Left (AM, PM, and SAT Peaks)
- Intersection 18: Chain Bridge Road and South Street/Humane Society Entrance
 - Eastbound Left/Thru (AM, PM, and SAT Peaks)
 - Eastbound Right (AM, PM, and SAT Peaks)



Figure 20: Future without Development (2026) - Levels of Service

Future Conditions with Development (2026) – Interim Condition

Site Description

The proposed program is a mixed-use development comprised of up to a 4,127-person capacity (2,796 fixed seat) concert hall with a 117 seat black box theater, a 169-room hotel with 26,127 SF of conference space, 6,049 SF of office, two (2) art galleries totaling 8,399 SF, 13,912 SF of restaurant space, and 13,912 SF of retail. Total site build-out is planned for the year 2026.

Site Access

Existing vehicular access is provided via two (2) driveways on Chain Bridge Road, two (2) driveways on Sager Avenue, and two (2) driveways on University Drive.

With redevelopment of the property, site access will be provided via Sager Avenue and University Drive.

Transportation Demand Management

The redevelopment of The Ox Fairfax – Block A site provides an opportunity to create a high-density, mixed-use environment that increases pedestrian accessibility and promotes recreational and educational use in the Old Town Fairfax Activity Center. Encouragement of non-singular occupancy vehicle (SOV) use, non-peak hour SOV trips, and non-automobile modes of transportation will result in less vehicular impacts on the surrounding roadway network.

Transportation Demand Management involves many components that are tailored to accommodate a given facility with a goal of reducing peak hour automobile trips by encouraging alternative forms of transportation. The following strategies are anticipated to be considered:

- Provide incentives to use transit, potentially including:
 - Providing information on the City of Fairfax CUE Bus, Fairfax Connector, and Metrobus routes, schedules, and fares
 - o Providing safe pedestrian connections on and off site
 - Providing bicycle parking
- Parking Management, potentially including:
 - On-site parking spaces for carshare agencies

A full Transportation Demand Management (TDM) Plan will be submitted with the application.

Preliminary Event Operations Plan for The Ox Fairfax – Block A

This section of the report reviews the transportation operations plan for The Ox Fairfax – Block A, which includes a concert hall that will serve as a venue for concerts by musicians, bands, and other preforming artists, with a total patron-capacity of 4,127 people.

This section provides an outline of a preliminary operation plan for The Ox Fairfax – Block A, including discussing traffic and parking demand loads that the concert hall will generate. Sellout concerts will be the largest event the concert hall will be able to accommodate. They will have the highest parking demand of any event, and the highest traffic demand for all modes. When they occur on weeknights, they have the greatest chance of traffic impacts when some concertgoers start arriving at the end of the PM peak commuter hour. Based on this overview, this preliminary event operation plan is based on the worst case scenario to provide a conservative transportation analysis. The worst case scenario analyzed within this report is a sellout crowd of 4,127 people. Events of this size may occur both on weekends and weeknights.

The projected timeline of arrivals and departures for sellout concerts, and initial considerations on traffic capacity are below:

- Employees of the theater working the show will arrive by 6:00 PM. It is assumed that a portion of the employees arriving will overlap with the PM peak commuter hour.
- Doors to the show will open at 7:00 PM. It is anticipated that arriving theater traffic will peak between 7:00 PM and 8:00 PM, which is off-peak for commuter traffic. With proper dispersal of drivers along different routes and between different times, roadway congestion better than commuter peak times is achievable.
- The concert will begin at 8:00 PM. It is anticipated that some concertgoers will continue to arrive after the show has begun.
- The headliner will come on at 9:00 PM. It is anticipated that all concert attendees have arrived by this time.
- The concert will end at approximately 11:15 PM. It is anticipated that departing traffic will peak between 11:00 PM and 12:00 AM. This time is off-peak with a relatively small number of vehicles on the road. Intersections will be congested from pedestrians walking to off-site parking, and once they are cleared out, vehicles exiting the parking garage and surface lot and heading home should not overwhelm the roadways and create congestion spots.

The parking and transportation demand characteristics of such a concert are summarized in Table 8, and are based on the following assumptions:

- Mode Split: A Mode Split/TDM reduction of 15 percent was applied to the theater (attendees and employees), hotel, and office uses, based on guidance from City staff. It was assumed that 35% of inbound auto trips during the PM peak commuter hour and Saturday peak hour are using some sort of rideshare, and therefore, are not parking on site.
- **Car Occupancy**: This report assumed an event car occupancy of three (3) persons per vehicle, per *Parking* (Robert A. Weant and Herbert S. Levinson, Eno Foundation for Transportation, 1990).
- Attendee Peak Flows: Not all concert attendees will arrive and depart at the same time. This report assumes that 12% of people will arrive during the PM peak commuter hour and Saturday peak hour. The AM peak commuter hour trips were based on 25% of the PM peak hour trips.
- Employee Peak Flows: Based on information provided by the Applicant, approximately 171 staff members will work sellout concerts. This report assumes that 10% of theater employees will generate trips during the AM peak commuter hour and 75% of employees will generate trips during the PM peak commuter hour. It is assumed that the employees arriving during the AM peak commuter hour work typical office hours and will therefore be outbound trips during the PM peak commuter hour.
- **Theater Capacity**: The Ox Fairfax Block A will consist of a 4,127-person capacity concert hall with a 117 seat black box theater. The black box theater will host smaller performances, but performances will not overlap with the larger concert hall. Therefore, a maximum theater capacity of 4,127 people was analyzed in this study.
- **Conference Center Use**: The 26,127 SF of conference center space will be a part of the hotel and is accounted for in the hotel trip generation calculation.

These demand estimates form the basis for the recommendations in the following sections.

Parking

The City of Fairfax has nine (9) public parking lots in Old Town Fairfax. Public parking lots would be available to visitors of The Ox Fairfax – Block A site who opt to park off-site, as an alternative to paying for parking on-site. Thus, a portion of site trips have been routed to public parking lots throughout the City. The Fairfax County Judicial Center also has a paid public parking garage that would be available to visitors of The Ox Fairfax – Block A site. The City's public parking lots and their hours of operation are

summarized in Table 7. The public parking lots and associated pedestrian routes and walksheds relative to The Ox Fairfax – Block A site are shown in Figure 21 and Figure 22, respectively.

Table 7: Public Parking Lots

Parking Facility	Туре	Hours	Rate	Notes
1. Truro Anglican Church (10480 Main Street)	Surface Lot	24/7	Free	None
2. Bank of America (10440 Main Street)	Surface Lot	M-Sun 6a-6p	Free	No overnight parking allowed
3. Old Town Plaza (3955 Chain Bridge Road)	Garage	24/7	Free	2 hour parking restriction
4. City of Fairfax Regional Library (10360 North Street)	Garage	M-T 1p-9p; W-Sun 10a-6p	Free	None
5. Webb Parking Lot (3990 University Drive)	Surface Lot	M-F after 6p, Sat after 1p, Sunday	Free	None
6. Old Town Hall Parking Lot (3999 University Drive)	Surface Lot	24/7	Free	2 hour parking restriction M-F 9a-5p, all other times no restrictions
7. Upper Old Town Hall Parking Lot (10413 North Street)	Surface Lot	24/7	Free	3 hour parking restriction M-F 9a-5p, all other times no restrictions
8. Main Street Parking Lot (10367 Main Street)	Surface Lot	24/7	Free	None
9. Sager Avenue Parking Lot (10412 Sager Avenue)	Surface Lot	M-Sun 6a-2a	Free	Free parking except for reserved spaces
10. Fairfax County Court Parking Garage (10550 Page Avenue)	Garage	24/7	\$2.50/hr, \$12.50 max	Parking is paid M-F 7a-7p



Figure 21: Public Parking Lots – Pedestrian Routes



Figure 22: Public Parking Lots – Walksheds

Site Generated Traffic

The Institute of Transportation Engineers (ITE) *Trip Generation*, 11th Edition along with the assumptions described above were used to determine the future trips generated by the proposed development as shown in Table 8. In order to account for the existing office buildings being removed, the associated existing office trips were removed from the street network. The removed trips are shown in Figure 24.

As stated previously, the 26,127 SF of conference center space will be a part of the hotel and is accounted for in the hotel trip generation calculation. The Ox Fairfax – Block A will consist of a 4,127-person capacity concert hall with a 117 seat black box theater. The black box theater will host smaller performances, but performances will not overlap with the larger concert hall. Therefore, a maximum theater capacity of 4,127 people was analyzed in this study.

Table 8: Trip Generation (2026)

			Weekday						Saturday				
Land Use	ITE Code	Size	AM	Peak H	lour	PM	Peak H	lour	Daily		Peak Ho	ur	Dailv
Trip Generation, 11th Ed.			In	Out	Total	In	Out	Total	Total	In	Out	Total	Total
Proposed Development													
Shopping Center ⁵	822 13.9	9 kSF of GFA	20	13	33	46	46	92	817	46	45	91	910
	Internal Capture Retail - Office	4 (2)	0	-1	-1	-1	0	-1	-5	0	0	0	-1
	Internal Capture Retail - Hotel	4 (3)	-2	-1	-3	-5	-5	-10	-82	-5	-5	-10	-91
	Ret	ail With Internal Reductions	18	11	29	40	41	81	730	41	40	81	818
	Pass-By Reduction	³ 25%/34%/26%	-5	-3	-8	-14	-14	-28	-183	-11	-10	-21	-205
		Retail Subtotal	13	8	21	26	27	53	547	30	30	60	613
High-Turnover (Sit-Down) Restaurant	932 13.9	9 kSF of GFA	73	60	133	77	49	126	1,491	80	76	156	1,703
	Pass-By Reduction	3 25%/43%/25%	-18	-15	-33	-33	-21	-54	-373	-20	-19	-39	-426
		Restaurant Subtotal	55	45	100	44	28	72	1,118	60	57	117	1,277
Theater ¹	4,12	7 People	28	28	56	165	58	223	2,751	165	58	223	2,751
	Mode Split/TDM Reduction	15%	-4	-4	-8	-25	-9	-34	-413	-25	-9	-34	-413
		Entertainment Subtotal	24	24	48	140	49	189	2,338	140	49	189	2,338
Theater ¹	17	1 Employees	15	2	17	113	15	128	342	113	15	128	342
	Mode Split/TDM Reduction	15%	-2	0	-2	-17	-2	-19	-51	-17	-2	-19	-51
			13	2	15	96	13	109	291	96	13	109	291
Hotel ²	310 169	9 Rooms	43	34	77	49	48	97	1,408	69	54	123	1,311
	Internal Capture Hotel - Office	^₄ (1)	0	-1	-1	-1	0	-1	-10	0	0	0	-1
	Internal Capture Hotel - Retail	4 (3)	-1	-2	-3	-5	-5	-10	-82	-5	-5	-10	-91
	Ho	tel With Internal Reductions	42	31	73	43	43	86	1,316	64	49	113	1,219
	Mode Split/TDM Reduction	15%	-6	-5	-11	-6	-6	-12	-197	-10	-7	-17	-183
		Hotel Subtotal	36	26	62	37	37	74	1,119	54	42	96	1,036
General Office Building	710 6.0) kSF of GFA	13	2	15	3	13	16	101	2	1	3	13
	Internal Capture Office - Hotel	4 (1)	-1	0	-1	0	-1	-1	-10	0	0	0	-1
	Internal Capture Office - Retail	4 (2)	-1	0	-1	0	-1	-1	-5	0	0	0	-1
	Offi	ce With Internal Reductions	11	2	13	3	11	14	86	2	1	3	11
	Mode Split/TDM Reduction	15%	-2	0	-2	0	-2	-2	-13	0	0	0	-2
		Office Subtotal	9	2	11	3	9	12	73	2	1	3	9
Museum ⁶	580 8.4	1 kSF of GFA	2	0	2	0	2	2	20	4	2	6	60
	Proposed Development S	Site Trips With Reductions	152	106	259	346	165	511	5,506	386	194	580	5,624

¹ Since doors to shows will not open until 7 PM and shows will not start until 8 PM, it was assumed 12% of people would arrive during the PM and SAT peak hour with three (3) people per car. AM peak hour trips were based on 25% of the PM peak hour trips. It was assumed that 10% of employees and 75% of employees would arrive during the AM and PM peak hours, respectively. Note: City of Fairfax Zoning Ordinance defines a theater as any building, structure or other indoor place, or any portion thereof, having fixed seating arranged in rows to allow spectators an unobstructed view and devoted to the performance arts for which an admission charge is made. The parking ratio requirement is 1 space per 4 seats.

² Hotel includes 26,127 SF of conference center space.

³ The pass by reduction for Shopping Center is based on the ITE Trip Generation methodology, as provided in the Trip Generation Handbook, 3rd Edition. The average rate for Shopping Centers is 34% for the PM Peak and 26% for the Saturday Peak. The average rate for High-Turnover (Sit-Down) Restaurants is 43% for the PM Peak. For all other time periods, the default pass by rate is 25%.

⁴ Internal capture rates consider site trips "captured" within a mixed use development, recognizing that trips from one land use can access another land use within a site development without having to access the adjacent street system. Internal capture allows reduction of site trips from adjacent intersections and roadways.

The internal reduction is based on the VDOT Updated Administrative Guidelines for the Traffic Impact Analysis Regulations:

(1) office / hotel - smaller of 10% of office trips or 10% of hotel trips

(2) office / retail - smaller of 5% of office trips or 5% of retail trips

(3) retail / hotel - smaller of 10% of retail trips or 10% of hotel trips

⁵ Saturday rates are not provided for the Shopping Center (822) land use; therefore, daily trips were estimated as 10 times the Saturday peak hour trips.

⁶ Weekday and Saturday daily rates are not provided for the Museum (580) land use; therefore, daily trips were estimated as 10 times the PM and Saturday peak hour trips, respectively.

Site Trip Distribution

The distribution of site trips was based on the 2045 Metropolitan Council of Governments (MWCOG) model as well as existing and anticipated traffic patterns with guidance and input from the City staff. The directional distribution percentages are shown in Figure 23. The site generated traffic volumes at the study intersections are shown in Figure 25 and the pass-by trips are shown in Figure 26.



Figure 23: Directional Distribution

Future with Development (2026) Traffic Volumes

In order to determine the Future Conditions with Development (2026) traffic volumes, the trips associated with the existing office buildings were removed and the site generated traffic volumes and pass-by trips were added to the Future without Development (2026) traffic volumes. The Future with Development (2026) peak hour traffic volumes are presented in Figure 27.



Figure 24: Removed Trips (2026)



Figure 25: Site Trips (2026)



Figure 26: Pass-By Trips (2026)



Figure 27: Future with Development (2026) - Peak Hour Traffic Volumes

Future with Development (2026) Intersection Capacity Analysis

Intersection capacity analysis was performed in a manner consistent with the methodology used for the Future Conditions without Development (2026) analysis.

The results of the intersection analysis are presented in Table 9, and are expressed in LOS and delay (seconds per vehicle) per lane group. The average and maximum queue results from SimTraffic are presented in Table 9 and are expressed in feet. The detailed analysis worksheets are included in Appendix F.

Table 9: Future with Development (2	026) – Intersection Capacity Analysis
-------------------------------------	---------------------------------------

				AM Peak H	our		PM Peak H	our		SAT Peak I	Hour
No.	Intersection (Movement)	Effective Storage Length (ft.) ^[1]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) ^[4]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) [4]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) ^[4]
			Sy	nchro	SimTraffic	Syr	nchro	SimTraffic	Sy	nchro	SimTraffic
1	Main Street (E/W) and West Street/North										
	Street (N/S)					_			-		
	Easthound Left	CCE	D C	35.0	202	D	40.3	760	D	43.6	044
	Eastbound Thru	000		37.0	547		34.1 45.0	700	D	37.4 48.2	858
	Eastbound Right	125	c	23.8	125	c	29.0	112	c	26.9	92
	Westbound Left	125	D	44.7	125	D	47.5	125	c	32.3	125
	Westbound Thru/Right		D	43.9	301	F	136.8	306	E	62.3	312
	Northbound Left/Thru Thru/Right		E	62.3	106	E	63.8	217	D	54.1	109
	Southbound Left/Thru		D	52.1	249	E	78.0	149	С	23.8	211
	Southbound Right		С	28.7	328	В	10.3	65	D	39.9	316
	(F/W)										
	Overall Intersection (Signalized)		в	17.3		D	44.9		в	15.7	
	Eastbound Thru Thru/Right		A	8.4	264	В	14.1	268	В	16.0	235
	Westbound Thru Thru/Right		В	16.8	313	D	50.3	338	A	7.8	186
	Northbound Thru Thru/Right		В	16.5	295	F	82.8	295	В	17.0	276
	Southbound Left	160	С	30.4	144	с	34.5	160	с	25.9	159
3	University Drive (N/S) and North Street		C	28.7	512	C	32.6	620	C	24.0	4/5
l i	(E/W)										
	Overall Intersection (Signalized)		в	18.6		с	24.9		в	18.8	
	Eastbound Left	215	A	6.0	172	D	38.9	214	В	13.0	134
	Eastbound Thru Thru/Right		A	2.6	211	С	23.9	322	В	12.7	196
	Westbound Left	150	A	2.6	120	A	7.1	149	A	9.7	140
	Westbound I nru I nru/Right		A	3.3	279	В	17.9	384	C	23.3	325
	Southbound Left/Thru Thru/Right			74.0 69.8	252	D	10.9	709	C	31.2	284
4	North Street (E/W) and Blenheim			03.0	252	0	40.4	105	0	01.2	204
	Boulevard (N/S)										
	Overall Intersection (Signalized)		С	31.0		D	38.4		С	29.3	
	Eastbound Left/Thru		F	94.6	140	F	152.9	140	D	47.8	140
	Eastbound Right		A	2.5	331	A	2.7	352	B	18.4	314
	Northbound Left		A	0.0	220	A	0.1	202	A	0.1	260
	Northbound Left/Thru/Right		B	14.1	367	B	14.0	380	c	21.9	382
	Southbound Left/Thru Thru		E	72.3	234	F	95.3	635	D	52.2	295
	Southbound Right	750	E	57.6	206	D	51.3	625	D	44.9	193
5	Main Street (E/W) and East										
	Street/Blenheim Boulevard (N/S)								•		
	Easthound Thru Thru/Right			25.3	200		34.0	216		26.2	214
	Westbound Thru		D	46.1	235	Ē	89.9	225	D	41.9	225
	Westbound Right		A	3.4	376	A	8.1	604	В	15.6	586
	Northbound Left	105	D	51.4	101	D	51.3	104	D	48.1	75
	Northbound Thru Thru/Right		E	59.3	202	E	64.1	278	D	53.4	173
	Southbound Left		В	16.8	275	В	11.8	310	A	6.8	286
-	Southbound Thru/Right		D	41.4	247	D	43.2	295	В	19.5	262
6	(N/S)										
	Overall Intersection (Signalized)		в	19.9		в	17.6		в	19.3	
	Eastbound Left	90	А	3.6	60	В	13.6	89	А	6.1	88
	Eastbound Thru/Right		A	5.3	191	В	17.5	275	А	8.9	272
	Westbound Left	125	A	0.3	108	В	10.1	118	A	3.2	118
	weswound I nru/Right		A	0.9	84	A	8.0	235	A	4.1	214
	Southbound Left/Thru Thru/Right		B	18.6	198	В	15.6	218	C	40.0	210
7	Main Street (E/W) and Chain Bridge Road		0	10.0	130	0	10.0	210	0	52.0	210
	(N/S)										
	Overall Intersection (Signalized)		С	29.5		D	39.1		С	21.8	
	Eastbound Left	125	В	11.8	83	В	11.0	119	A	5.5	100
	Eastbound I hru/Right	200	B	15.4	266	C	25.2	316	B	18.1	316
	Westbound Thru/Right	200		7U.3 20.2	1/2		84.3 12.0	198		12.1	195
	Northbound Thru Thru/Right		n	44.4	324	D	45.2	325	c	28.9	301
	Southbound Left/Thru Thru/Right		В	11.7	219	D	44.2	294	Ā	8.9	208
8	Chain Bridge Road (N/S) and Sager										
	Avenue (E/W)										
	Overall Intersection (Unsignalized)		_	40.0	400	-	44.7	470		<u> </u>	<u>.</u>
	weswound Right		В	12.0	106	B	11.7	1/6	A	9.5	85
9	Sager Avenue (E/W) and Site Entrance		A	5.0	204	A	4.8	016	A	4./	100
Ē	(N/S)										
	Overall Intersection (Unsignalized)										
	Westbound Left/Thru		A	5.1	63	A	4.7	116	A	4.4	81
L	Northbound Lett/Right		A	9.8	82	В	10.7	129	В	10.6	96

				AM Peak H	our		PM Peak F	lour		SAT Peak	Hour
No.	Intersection (Movement)	Effective Storage Length (ft.)	LOS	Delay (sec/veh)	Ave. Max Queue (ft.)	LOS	Delay (sec/veh)	Ave. Max Queue (ft.)	LOS	Delay (sec/veh)	Ave. Max Queue (ft.)
			Sv	nchro	SimTraffic	Svr	nchro	SimTraffic	Sv	nchro	SimTraffic
10	University Drive (N/S) and Sager										
	Avenue (E/W)		_			_					
	Overall Intersection (Signalized)		B	10.7	150	B	10.3	194	A	9.5	147
	Westbound Left	130	c	25.3	100	c	23.7	96	в	19.8	63
	Westbound Thru/Right		С	25.3	128	С	23.5	211	с	20.3	161
	Northbound Left/Thru Thru/Right		Α	1.8	88	A	3.5	209	A	4.3	120
- 11	Southbound Left/Thru Thru/Right		A	1.0	113	A	3.1	128	A	2.6	120
11	Sager Avenue (E/W) and South Street/East Street (N/S)										
	Overall Intersection (Unsignalized)										
	Eastbound Left	90	Α	8.2	60	A	9.9	89	A	8.2	48
	Eastbound Thru/Right	00	A	7.7	66	B	10.6	261	A	7.8	65
	Westbound Thru/Right	80	A	7.8 9.3	58 98	B	9.2	393	A	7.8	30 114
	Northbound Left/Thru/Right		A	8.8	65	В	11.4	387	A	8.5	58
	Southbound Left/Thru/Right		A	9.1	154	В	13.3	249	A	9.2	127
12	University Drive (N/S) and Entrance										
	Overall Intersection (Signalized)		А	6.0		А	9.6		A	7.1	
	Eastbound Left/Thru/Right		А	0.0	0	С	26.7	113	с	27.9	97
	Westbound Left/Thru/Right		С	29.7	118	С	33.6	140	С	28.4	87
	Northbound Left/Thru/Right		A	4.2	129	A	6.5	143	A	3.6	116
	Southbound Thru/Right		A 4	1.8	25	A	2.9	28	A A	1.8	25
13	University Drive (N/S) and Parking		A	2.4	104		7.2	102		2.0	100
	Lot/Fire Station #3 Entrance (E/W)										
	Overall Intersection (Unsignalized)		Р	44.5	54	P	12.5	405		44.0	<u></u>
	Southbound Left	85	ь А	79	54 4	ь А	84	105	A	9.1	0
14	University Drive/George Mason	00		1.0	•		0.1	0		0.1	0
	Boulevard (N/S) and Armstrong Street										
	(E/W) Overall Intersection (Signalized)		в	11 7		в	14 7		в	12.0	
	Eastbound Left/Thru/Right		В	17.2	114	c	20.2	164	В	16.6	141
	Westbound Left/Thru/Right		В	16.1	107	В	18.4	202	в	14.5	127
	Northbound Left	160	Α	7.8	39	A	8.4	75	A	8.2	40
	Northbound Thru/Right	220	A	9.9	136	B	11.2	262	В	10.0	170
	Southbound Thru/Right	230	B	10.4	30 187	B	9.1	32 241	B	0.3 10.6	160
15	Chain Bridge Road (N/S) and					-					
	Armstrong Street (E/W)					_					
	Verall Intersection (Signalized)		A	9.6	204	B	18.9	342	В	10.6 36.4	170
	Northbound Thru Thru/Right		A	6.7	355	C	26.1	545	В	11.9	315
	Southbound Left	80	в	18.9	79	A	2.7	76	А	5.6	80
	Southbound Thru		A	2.6	208	A	2.4	240	A	4.7	193
16	Chain Bridge Road (N/S) and Judicial Drive (E/W)										
	Overall Intersection (Signalized)		в	14.4		в	17.0		A	9.5	
	Eastbound Left	410	E	70.1	146	E	66.3	360	E	59.7	90
	Eastbound Right	160	D	48.0	83	D	42.8	737	D	47.8	79
	Northbound Thru	UOT	A	5.8 6.1	295	Δ	33.2 15	308	A	∠.∪ 1.9	147
	Southbound Thru		В	19.3	231	В	10.2	218	A	5.5	195
	Southbound Right	240	С	34.1	96	Α	6.5	82	A	4.4	40
17	Chain Bridge Road (N/S) and Humane										
	INTERSECTION REMOVED										
18	Chain Bridge Road (N/S) and South										
	Street/Humane Society Entrance (E/W)										
	overall intersection (Signalized)		A	3.5	67	A	9.0	149	B	10.7 23.0	36
	Eastbound Right		E	64.3	62	E	61.5	92	c	23.9 22.8	48
	Westbound Left/Thru/Right		A	0.0	0	A	0.0	0	c	23.9	51
	Northbound Left	230	А	1.4	212	A	1.6	230	А	6.2	75
	Northbound Thru Thru/Right		A	2.1	320	A	1.9	407	A	9.7	227
	Southbound Thru Thru/Right		A ^	U.O	U 104	A	0.0	U 207	A	8.6 11.2	30
19	West Street (N/S) and South Street		А	1.4	104	А	0.0	201		11.2	201
	(E/W)										
	Overall Intersection (Unsignalized)			0.0	24		10	60		10	c
	Southbound Left/Thru/Right		R	0.8 10.4	∠1 46	B	1.2	53 36	A	1.9 9.0	0 45
·										2.0	

NOTES:

(1) Effective storage length is based on the storage length plus one-half of the taper length per TOSAM guidelines.
(2) #:95th percentile queues (reported from Synchro) exceed capacity; actual queues may be longer. Queues shown are based on the maximum after two cycles.
(3) m: 95th percentile volume and queues (reported from Synchro) are metered by upstream signal.
(4) Max queues are based on results from Sim Taffic. Per TOSAM guidelines, the queues are based on the average to 10 simulations.

[5] 50th Percentile Queues are not reported for TWSC intersections under HCM Methodology.
 [6] ~: Volume exceeds capacity, queue is theoretically infinite.

For the purpose of this analysis, it is desirable to achieve a level of service (LOS) of D or better for each lane group at the study intersections.

The intersection capacity analysis results show that the following ten (10) intersections have movements that operate below the acceptable levels of service during one or more peak hours under Future Conditions with Development (2026). Movements in **bold** operate below acceptable levels of service under Future Conditions without Development (2026).

- Intersection 1: Main Street and West Street/North Street
 - Westbound Thru/Right (PM and SAT Peaks)
 - Northbound Left/Thru, Thru/Right (AM and PM Peaks)
 - Southbound Left/Thru (PM Peak)
- Intersection 2: Main Street and North Street
 - Northbound Thru, Thru/Right (PM Peak)
- Intersection 3: University Drive and North Street
 - Northbound Left/Thru, Thru/Right (AM Peak)
 - Southbound Left/Thru, Thru/Right (AM Peak)
- Intersection 4: North Street and Blenheim Boulevard
 - Eastbound Left/Thru (AM and PM Peaks)
 - o Southbound Left/Thru, Thru (AM and PM Peaks)
 - Southbound Right (AM Peak)
- Intersection 5: Main Street and East Street/Blenheim Boulevard
 - Eastbound Thru, Thru/Right (PM Peak)
 - Westbound Thru (PM Peak)
 - Northbound Thru, Thru/Right (AM and PM Peaks)
- Intersection 6: Main Street and University Drive
 - Northbound Left/Thru, Thru/Right (AM Peak)
- Intersection 7: Main Street and Chain Bridge Road
 - Westbound Left (AM, PM, and SAT Peaks)
- Intersection 15: Chain Bridge Road and Armstrong Street
 - Westbound Left/Right (AM and PM Peaks)
- Intersection 16: Chain Bridge Road and Judicial Drive
 - Eastbound Left (AM, PM, and SAT Peaks)
- Intersection 18: Chain Bridge Road and South Street/Humane Society Entrance
 - Eastbound Left/Thru (AM and PM Peaks)
 - Eastbound Right (AM and PM Peaks)

In order to achieve acceptable levels of service or maintain similar traffic operation conditions as compared to the Future without Development (2026) scenario, the following roadway improvements are recommended (by intersection):

- Intersection 1: Main Street and West Street/North Street
 - Adjust signal timings to improve operations (SAT Peak only).
- o Intersection 5: Main Street and East Street/Blenheim Boulevard
 - Adjust signal timings to improve operations (PM Peak only).
- o Intersection 7: Main Street and Chain Bridge Road
 - Adjust signal timings to improve operations (PM Peak only).
- Intersection 12: University Drive and South Street
 - Install a traffic signal and add exclusive northbound left lane.

The results of the mitigated intersection analysis are presented in Table 10, and are expressed in LOS and delay (seconds per vehicle) per lane group. The average and maximum queue results from the mitigated SimTraffic are presented in Table 10 and are expressed in feet. Level of service results are also presented in Figure 28.

Table 10: Future with Development (2026) Mitigated – Intersection Capacity Analysis

		, ,		AM Peak H	lour		PM Peak I	lour		SAT Peak	Hour
No.	Intersection (Movement)	Effective Storage Length (ft.) [1]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) [4]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) ^[4]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) ^[4]
			Syı	nchro	SimTraffic	Syr	nchro	SimTraffic	Sy	nchro	SimTraffic
1	Main Street (E/W) and West										
	Street/North Street (N/S)										
	(MIT: Signal timing modification - SAT										
	Only)		D	35.0		с	34.1		D	40.1	
	Eastbound Left	665	С	31.0	402	D	36.6	848	D	47.5	496
	Eastbound Thru		D	37.0	601	С	24.9	836	С	26.6	612
	Eastbound Right	125	С	23.8	125	В	17.3	114	В	17.4	67
	Westbound Thru/Right	125	D	44.7	265	F	40.0	314	Ċ	22.5	310
	Northbound Left/Thru Thru/Right		E	62.3	122	Ê	64.1	389	D	54.1	116
	Southbound Left/Thru		D	52.1	252	E	78.0	139	D	40.9	247
	Southbound Right		С	28.7	314	В	11.4	108	D	49.0	330
2	Chain Bridge Road (N/S) and North										
	Overall Intersection (Signalized)		в	17 3		- C	31.0		в	12.8	
	Eastbound Thru Thru/Right		A	8.4	269	в	14.1	284	A	2.4	213
	Westbound Thru Thru/Right		В	16.8	287	D	50.3	341	А	7.8	181
	Northbound Thru Thru/Right		В	16.5	293	В	13.9	285	В	16.9	279
	Southbound Left	160	С	30.4	150	С	34.5	160	С	25.9	159
2	Southbound Thru Thru/Right		С	28.7	483	С	32.6	626	С	24.0	508
3	(E/W)										
	Overall Intersection (Signalized)		в	18.6		с	25.6		в	19.1	
1	Eastbound Left	215	Α	6.0	182	D	38.9	215	В	11.5	195
	Eastbound Thru Thru/Right		A	2.6	236	С	25.3	335	В	11.3	288
	Westbound Left	150	A	2.6	91	A	7.0	139	В	10.7	139
	Nethbound Left/Thru Thru/Right		A	3.3	254	В	17.8	377	С	25.5	307
	Southbound Left/Thru Thru/Right		E	74.7 69.8	214		19.8	794	Ċ	31.2	233
4	North Street (E/W) and Blenheim		_	00.0	214		40.4	754	Ŭ	01.2	200
	Boulevard (N/S)										
	Overall Intersection (Signalized)		С	31.0		D	38.3		с	30.5	
	Eastbound Left/Thru		F	94.6	140	F	153.0	140	D	45.5	140
	Eastbound Right		A	2.5	329	A	2.6	376	В	16.3	340
	Northbound Left		B	14.1	356	B	14.8	384	ĉ	26.8	378
	Northbound Left/Thru/Right		В	14.0	364	В	14.7	379	c	26.7	374
	Southbound Left/Thru Thru		E	72.3	237	F	95.3	638	D	52.2	552
	Southbound Right	750	E	57.6	192	D	51.3	622	D	44.9	530
5	Main Street (E/W) and East										
	Overall Intersection (Signalized)										
	(MIT: Signal timing modification - PM										
	Only)		С	25.3		с	29.3		с	25.2	
	Eastbound Thru Thru/Right		D	38.4	294	D	39.5	163	D	40.6	309
	Westbound Thru		D	46.1	224	E	57.1	225	С	33.7	225
	Westbound Right	105	A	3.4	434	A	8.1	609	В	13.6	561
	Northbound Thru Thru/Right	105	F	59.3	65 195	F	52.2	63 277	F	40.9	98
	Southbound Left		В	16.8	275	В	15.0	313	В	10.7	307
	Southbound Thru/Right		D	41.4	254	D	48.3	293	С	26.5	289
6	Main Street (E/W) and University Drive										
1	(N/S) Overall Intersection (Signalized)			10.0			24.0			40.0	
1	Eastbound Left	۵n		3.6	66	_ △	∠1.0 10.0	79		6.1	85
1	Eastbound Thru/Right	30	Â	5.3	156	ĉ	29.1	324	Ā	8.9	278
1	Westbound Left	125	A	0.3	111	в	10.1	106	A	2.2	121
	Westbound Thru/Right		A	0.9	88	В	11.8	137	A	3.2	237
	Northbound Left/Thru Thru/Right		E	62.3	153	С	29.5	152	D	48.6	151
7	Southbound Left/Thru Thru/Right		В	18.6	153	В	15.6	137	С	32.1	194
'	Road (N/S)										
	Overall Intersection (Signalized)										
	(MIT: Signal timing modification - PM										
	Only)	10-	C	29.5	46-	C .	34.1	100	C	24.1	
1	Eastbound Left	125	В	11.8	105	B	18.9	122	В	12.7	115
1	Westbound Left	200	B F	15.4 70.4	283	E C	25.0	322		27.0	318
1	Westbound Thru/Right	200	c	20.3	238	c	31.0	322	В	16.6	269
1	Northbound Thru Thru/Right		D	44.4	313	D	52.9	309	c	28.9	300
	Southbound Left/Thru Thru/Right		В	11.7	208	В	14.1	220	A	8.9	251
8	Chain Bridge Road (N/S) and Sager										
	Avenue (E/W)										
	Overall Intersection (Unsignalized)		Б	12.0	00		11 7	111		0 5	124
	Southbound Left/Thru Thru		A	12.0	90 240	Å	4.8	277	A	9.5 4 7	134
9	Sager Avenue (E/W) and Site Entrance			0.0	2.10		7.0	211		7.1	
	Overall Intersection (Unsignalized)										
1	Westbound Left/Thru		A	5.1	62	A	4.7	75	A	4.4	91
	Northbound Left/Right		A	9.8	70	В	10.7	123	В	10.6	109

				AM Peak Ho	our		PM Peak H	our		SAT Peak	Hour
No.	Intersection (Movement)	Effective Storage Length (ft.) [1]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) [4]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) ^[4]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) [4]
			Syı	nchro	SimTraffic	Syn	ichro	SimTraffic	Syr	nchro	SimTraffic
10	University Drive (N/S) and Sager Avenue (E/W)										
	Overall Intersection (Signalized)		в	10.7		Α	10.0		A	9.9	
	Eastbound Left/Thru/Right	100	С	29.4	150	С	28.8	148	С	24.4	157
	Westbound Left Westbound Thru/Right	130	C C	25.3	89 139	C C	23.7	170	C B	19.8	61 192
	Northbound Left/Thru Thru/Right		A	1.8	80	A	5.6	194	A	4.3	125
	Southbound Left/Thru Thru/Right		А	1.0	74	A	2.7	133	A	3.3	93
11	Sager Avenue (E/W) and South Street/East										
	Overall Intersection (Unsignalized)										
	Eastbound Left	90	А	8.2	57	A	9.9	89	A	8.2	66
	Eastbound Thru/Right	80	A	7.7	61	В	10.6	314	A	7.8	91
	Westbound Thru/Right	00	A	9.3	98	В	9.2	396	A	8.9	212
	Northbound Left/Thru/Right		A	8.8	71	В	11.4	431	A	8.5	73
	Southbound Left/Thru/Right		A	9.1	154	В	13.3	262	A	9.2	158
12	Drive/South Street (E/W)										
	Overall Intersection (Signalized)										
	(MIT: Install signal, add exclusive NBL					_					
	Easthound Left/Thru/Right		A 	6.0	40	в	13.6	120	A	7.0	63
	Westbound Left/Thru/Right		c	29.7	105	D	41.7	165	c	28.4	96
	Northbound Left	50	A	3.0	27	A	6.3	70	A	2.8	48
	Northbound Thru/Right		A	4.1	120	A	8.4	145	A	3.4	115
	Southbound Lett Southbound Thru/Right		A A	1.8 2.4	46 86	A 	7.6 9.4	85 161	A 	1.8	37
13	University Drive (N/S) and Parking Lot/Fire		~	2.4			J. 4	101		2.4	68
	Station #3 Entrance (E/W)										
	Overall Intersection (Unsignalized)		P	11.5	40		12.4	124		11.0	74
	Southbound Left	50	A	7.9	49	A	8.4	4	A	9.1	2
14	University Drive/George Mason Boulevard	00	~	1.0	-		0.1			0.1	-
	(N/S) and Armstrong Street (E/W)		-			_					
	Overall Intersection (Signalized)		B	11.7 17.2	116	В	14.7	201	B	12.0	120
	Westbound Left/Thru/Right		В	16.1	105	в	18.4	201	В	14.5	83
	Northbound Left	160	Ā	7.8	39	A	8.4	66	A	8.2	35
	Northbound Thru/Right		A	9.9	130	В	11.2	364	В	10.0	136
	Southbound Left	230	A	7.7	40	A	9.1	34	A	8.3	34
15	Chain Bridge Road (N/S) and Armstrong		D	10.4	100	B	15.1	233	D D	10.6	130
	Street (E/W)										
	Overall Intersection (Signalized)		A	9.6	000	B	18.9	0.40	В	10.6	170
	Northbound Thru Thru/Right		A	67	209	E C	72.3 26.1	343 577	B	30.4	440
	Southbound Left	80	В	18.9	79	A	2.7	77	A	5.6	79
	Southbound Thru		А	2.6	213	А	2.4	289	A	4.7	236
16	Chain Bridge Road (N/S) and Judicial Drive (F/W)										
	Overall Intersection (Signalized)		в	14.4		в	16.8		A	9.5	
	Eastbound Left	410	E	70.1	139	E	66.3	277	E	59.7	88
	Eastbound Right	100	D	48.0	72	D	42.8	277	D	47.8	89
	Northbound Thru	160	A	ວ.8 6.1	293	A	33.2 1.5	293	A	2.0 1.9	149 255
	Southbound Thru		в	19.3	261	В	10.0	284	A	5.5	222
	Southbound Right	240	С	34.1	170	A	3.6	62	A	4.4	66
17	Chain Bridge Road (N/S) and Humane Society Driveway (E/W) INTERSECTION REMOVED										
18	Chain Bridge Road (N/S) and South		-								
	Street/Humane Society Entrance (E/W) Overall Intersection (Signalized)		۸	35		•	84		R	10.7	
	Eastbound Left/Thru		E	66.5	58	Ê	65.0	147	c	23.9	39
	Eastbound Right		E	64.3	72	E	61.5	79	c	22.8	56
	Westbound Left/Thru/Right		А	0.0	68	A	0.0	0	С	23.9	175
	Northbound Left	230	A	1.4	216	A	1.6	130	A	6.2	148
	Southbound Left		A	∠.1 0.0	346 83	A	0.0	341 30	A	9.7 8.6	კენ 81
	Southbound Thru Thru/Right		A	1.4	280	A	5.3	215	В	11.2	241
19	West Street (N/S) and South Street (E/W)										
	Overall Intersection (Unsignalized)			0.9	14		1.0	70		10	2
	Southbound Left/Thru/Right		A R	0.8 10.4	14	R	1.2	72	A	1.9	3 46
NOTES:			5							0.0	
[1]	Effective storage length is based on the storage	e length plus one-half of th	e taper length	per TOSAM guid	elines.						
[2]	#: 95th percentile queues (reported from Synch	nro) exceed capacity; actu	al queues maj	/ be longer. Queue	es shown are based o	on the maximum	after two cycles.				
[3] [4]	m: som percentile volume and queues (reported)	ea rrom Synchro) are mete ffic. Per TOSAM quidelino	red by upstrea	im signal. are based on the	average to 10 simulat	ions					
[⁴⁴] [5]	50th Percentile Queues are not reported for TV	VSC intersections under H	CM Methodol	ogy.	aronago to ro simulat						
[6]	~: Volume exceeds capacity, queue is theoretic	cally infinite.									



Figure 28: Future with Development (2026) – Levels of Service

Future Conditions with Development (2031) - Ultimate Condition

The City's recommended extension of South Street to West Street between University Drive and Chain Bridge Road will reroute traffic and relieve congestion on Main Street through Old Town. The extension will also permit the continuation of pedestrian facilities through Old Town. The proposed South Street Extension is expected to be complete in 2031 and will be built by others. The extension will ultimately provide access to The Ox Fairfax – Block A site. For the purposes of this study, an ultimate scenario was evaluated with the South Street Extension in place through The Ox Fairfax – Block A site.

The Future with Development (2031) peak hour traffic volumes are presented in Figure 29.



Figure 29: Future with Development (2031) - Peak Hour Traffic Volumes

Future with Development (2031) Intersection Capacity Analysis

Intersection capacity analysis was performed in a manner consistent with the methodology used for the Future Conditions with Development (2026) analysis.

The results of the intersection analysis are presented in Table 11, and are expressed in LOS and delay (seconds per vehicle) per lane group. The average and maximum queue results from SimTraffic are presented in Table 11 and are expressed in feet. The detailed analysis worksheets are included in Appendix G.

Table 11: Future with Development (2031) – Intersection Capacity Analysis

				AM Peak H	lour		PM Peak H	our		SAT Peak	Hour
No.	Intersection (Movement)	Effective Storage Length (ft.) ^[1]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) ^[4]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) [4]	LOS	Delay (sec/veh)	Ave. Max Queue (ft.) [4]
			Sy	nchro	SimTraffic	Syı	nchro	SimTraffic	Syı	nchro	SimTraffic
1	Main Street (E/W) and West										
	Street/North Street (N/S)										
	Overall Intersection (Signalized)										
	(MIT: Signal timing modification - SAT		~	24.0		<u> </u>	20.7				
	Easthound Left	665		34.9	200		32.1	946	5	41.4	502
	Eastbound Thru	005		36.5	613	C	24.8	843	C	45.2	601
	Eastbound Right	125	C	23.7	124	B	24.0	115	B	17.4	03
	Westbound Left	125	D	43.5	125	D	50.2	125	в	19.4	123
	Westbound Thru/Right	120	D	41.1	269	E	76.2	314	c	20.5	316
	Northbound Left/Thru Thru/Right		E	60.2	113	E	64.1	392	D	54.1	145
	Southbound Left/Thru		D	51.0	251	E	79.4	131	D	39.9	254
	Southbound Right		С	30.0	331	В	13.6	72	D	50.9	334
2	Chain Bridge Road (N/S) and North										
	Street (E/W)										
	Overall Intersection (Signalized)		В	17.4		С	32.3		В	12.8	
	Eastbound Ihru Ihru/Right		A	8.7	259	В	15.0	288	A	2.4	218
	Westbound Ihru Ihru/Right		В	17.6	319	D	51.2	336	A	7.2	202
1	Normbound Inru Inru/Right	100	В	16.4	290	В	18.2	291	В	17.2	287
	Southbound Leit	160	C	30.1	159		36.1	159	C	26.5	159
2	University Drive (N/S) and North Street		U U	20.0	000		33.1	027		24.4	506
Ŭ	(E/W)										
1	Overall Intersection (Signalized)		в	18.1		с	26.6		в	19.3	
	Eastbound Left	215	A	8.0	185	D	47.5	215	в	14.0	215
	Eastbound Thru Thru/Right		A	2.5	236	С	26.0	336	в	10.8	314
	Westbound Left	150	A	2.6	83	A	7.3	139	в	10.4	132
	Westbound Thru Thru/Right		A	3.4	283	В	19.0	377	С	26.1	352
	Northbound Left/Thru Thru/Right		E	69.5	194	В	19.2	154	A	9.5	114
	Southbound Left/Thru Thru/Right		E	69.6	211	D	43.7	799	С	31.2	316
4	North Street (E/W) and Blenheim										
	Boulevard (N/S)										
	Easthound Left/Thru		C	32.1	140		40.6	140	C D	31.4	140
	Eastbound Leivinnu			99.3	140		172.6	140	D	47.1	140
	Westbound Right		A .	2.5	320	A .	2.5	0		0.1	0
	Northbound Left		R	15.0	367	R	15.5	303	C C	28.5	380
	Northbound Left/Thru/Right		B	14.8	374	B	15.3	383	C C	20.0	374
	Southbound Left/Thru Thru		F	74.0	249	F	99.7	637		52.4	617
	Southbound Right	750	E	57.5	212	D	51.4	623	D	44.8	613
5	Main Street (E/W) and East										
	Street/Blenheim Boulevard (N/S)										
	Overall Intersection (Signalized)										
	(MIT: Signal timing modification - PM										
	Only)		C	24.8	005	c	29.0	450	c	25.0	040
	Eastbound Thru Thru/Right		D	37.8	295	D	39.3	158	D	40.7	312
	Westbound Tillu		D	46.3	224	Ē	57.3	225		33.7	225
1	Northbound Left	105		5.0	333		52.2	104		14.2	104
	Northbound Thru Thru/Bight	105	F	59.3	195	F	52.2 66.4	278	F	49.0	251
	Southbound Left		В	16.8	289	В	15.5	313	В	10.7	304
	Southbound Thru/Right		D	41.0	264	D	48.1	292	c	25.7	291
6	Main Street (E/W) and University Drive			-							
1	(N/S)										
	Overall Intersection (Signalized)		С	20.5		с	20.1		в	18.5	
	Eastbound Left	90	A	3.4	60	A	9.9	83	A	4.8	88
	Eastbound Thru/Right		A	5.1	192	С	28.2	326	A	7.9	255
	Westbound Left	125	A	0.3	110	В	10.2	107	A	1.6	124
	Westbound Thru/Right		A	0.9	92	В	11.8	136	A	2.5	236
1	Normbound Left/Thru Thru/Right		E	64.0	148	C _	28.5	151	D	53.4	144
	Southbound Left/Thru Thru/Right		С	21.0	174	В	14.4	143	С	34.5	194
· '	Road (N/S)										
1	Overall Intersection (Signalized)										
1	(MIT: Signal timing modification - PM										
1	Only)		с	28.6		с	32.3		с	24.3	
1	Eastbound Left	125	В	12.0	104	С	20.1	125	В	12.3	110
1	Eastbound Thru/Right		В	15.9	288	С	26.5	325	С	26.2	317
	Westbound Left	200	E	71.8	190	E	59.5	200	E	68.8	195
1	Westbound Thru/Right		В	19.6	228	С	34.9	321	В	15.2	286
1	Northbound Thru Thru/Right		D	41.6	314	D	47.5	311	С	29.9	299
L	Southbound Left/Thru Thru/Right	ļ	В	12.3	214	В	11.3	187	A	9.2	246

		AM Peak Hour PM Peak Hour			SAT Peak Hour						
		Effective Storage	LOS	Delay	Ave. Max	LOS	Delay	Ave. Max	LOS	Delay	Ave. Max
No.	Intersection (Movement)	Length (ft.)		(sec/veh)	Queue (ft.)		(sec/veh)	Queue (ft.)		(sec/veh)	Queue (ft.)
		[1]			[4]			[4]			[4]
8	Chain Bridge Road (N/S) and Sager Avenue		Syi	nchro	SimTraffic	Syn	chro	SimTraffic	Syr	ichro	SimTraffic
	(E/W)										
	Overall Intersection (Unsignalized)		_			_					
	Westbound Right		B	10.8	100	B	10.6	89	A	9.6	134
9	Sager Avenue (E/W) and Site Entrance		A	4.0	220		4.5	245		4.5	100
	Overall Intersection (Unsignalized)										
	Westbound Left/Thru		A	5.0	60	A	4.7	60	A	4.4	103
40	Northbound Left/Right		A	9.8	80	В	10.7	111	В	10.5	105
10	(E/W)										
	Overall Intersection (Signalized)		в	11.0		A	9.8		Α	9.7	
	Eastbound Left/Thru/Right		C	31.8	148	С	28.6	148	С	23.9	144
	Westbound Left Westbound Thru/Right	130	C	26.0	86	C C	24.2	91	C C	20.4	56
	Northbound Left/Thru Thru/Right		A	1.7	79	A	5.3	185	A	3.0	125
	Southbound Left/Thru Thru/Right		А	0.9	77	A	2.5	137	А	3.1	106
11	Sager Avenue (E/W) and South Street/East										
	Overall Intersection (Unsignalized)										
	Eastbound Left	90	А	8.2	66	A	9.9	89	А	8.1	50
	Eastbound Thru/Right		A	7.7	65	В	10.6	323	A	7.8	66
	Westbound Left	80	A	7.8	59	A	9.1	80	A	7.8	43
	Northbound Left/Thru/Right		A	9.3	95 71	B	11.3	423	A	8.5	62
	Southbound Left/Thru/Right		A	9.1	163	В	13.3	272	A	9.2	141
12	University Drive (N/S) and South Street										
	(E/W) Overall Intersection (Signalized)										
	(MIT: Install signal, add exclusive NBL										
	lane)		Α	6.1		В	12.9		Α	6.2	
	Eastbound Left/Thru/Right		C	28.8	48	С	29.4	59	C	24.8	52
	Northbound Left	50	۲ ۵	30.7	25	A	40.9	135		28.3	108
	Northbound Thru/Right	50	A	3.6	129	A	8.4	147	A	3.4	118
	Southbound Left		А	1.5	43	A	7.6	102	A	1.8	37
40	Southbound Thru/Right		A	2.0	99	A	9.3	146	A	2.5	100
13	Station #3 Entrance (E/W)										
	Overall Intersection (Unsignalized)										
	Eastbound Left/Thru/Right		В	11.7	47	В	13.7	144	В	11.2	81
	Northbound Left/Thru/Right	50	A	0.1	37	A	0.3	510	A	0.4	150
14	University Drive/George Mason Boulevard	50	A	7.9	5	A	8.4	14	A	9.1	/
	(N/S) and Armstrong Street (E/W)										
	Overall Intersection (Signalized)		в	11.7		в	14.6		В	11.9	
	Eastbound Left/Thru/Right		B	17.2	126	B	20.0	243	B	16.4	133
	Northbound Left	160	A	7.8	55	A	18.3	148	A	14.4	31
	Northbound Thru/Right		A	10.0	153	В	11.4	390	В	10.1	167
	Southbound Left	230	A	7.7	38	A	9.1	57	A	8.3	39
45	Southbound Thru/Right		В	10.4	181	В	14.9	217	В	10.5	166
15	Street (E/W)										
	Overall Intersection (Signalized)		Α	9.3		в	17.6		Α	9.8	
	Westbound Left/Right		E	66.9	226	E	70.2	285	D	35.7	169
	Normbound I hru I hru/Right	80	A	6.7	515	C	25.2	474	B	11.4	413
	Southbound Thru	00	A	1.9	206	Â	2.2	286	Â	4.4	200
16	Chain Bridge Road (N/S) and Judicial Drive			-							-
	(E/W) Overall Intersection (Signalized)		в	10.6		, P	10.4			0.4	
	Eastbound Left	410	E	70.1	150	E	10.4 66.3	212	E	9.4 59.7	134
	Eastbound Right		D	46.5	75	D	42.8	198	D	47.8	110
	Northbound Left	160	A	6.1	160	D	41.5	160	A	2.1	151
	Northbound Thru		A	6.2	300	A	1.5	283	A	1.9	240
	Southbound Right	240	A	5.0 2.2	209 150	A	7.9	314 184	A	0.0 4.4	207
17	Chain Bridge Road (N/S) and Humane	2.10		2.6	.00						
	Society Driveway (E/W)										
18	Chain Bridge Road (N/S) and South Street										
	(E/W)										
	Overall Intersection (Signalized)		В	13.6		В	14.7		В	15.3	
	Eastbound Left/Thru		E	66.7	67	E	65.0	120	C	30.6	56
	Eastbound Right		E	64.1 58.2	48	E	61.0 66.6	90	C	29.5	60 140
	Northbound Left	230	A	4.6	229	Ā	4.4	178	A	8.3	178
	Northbound Thru Thru/Right		A	7.5	394	A	6.3	339	в	13.6	338
	Southbound Left	90	В	17.6	78	A	7.2	85	В	11.2	68
40	Southbound Thru Thru/Right		С	20.9	299	A	9.6	286	В	15.2	239
1.9	Overall Intersection (Unsignalized)										
	Eastbound Left/Thru/Right		А	0.7	18	A	1.2	38	А	1.5	15
L	Southbound Left/Thru/Right		В	10.5	44	В	10.7	40	А	9.2	48
NOTES											

Es:

[1] Effective storage length is based on the storage length plus one-half of the taper length per TOSAM guidelines.
[2] #: 95th percentile queues (reported from Synchro) exceed capacity; actual queues may be longer. Queues shown are based on the maximum after two cycles.
[3] m: 95th percentile volume and queues (reported from Synchro) are metered by upstream signal.
[4] Max queues are based on results from SimTaffic. Per TOSAM guidelines, the queues are based on the average to 10 simulations.
[5] 50th Percentile Queues are not reported for TWSC intersections under HCM Methodology.
[6] ~: Volume exceeds capacity, queue is theoretically infinite.

For the purpose of this analysis, it is desirable to achieve a level of service (LOS) of D or better for each lane group at the study intersections.

The intersection capacity analysis results show that the following nine (9) intersections have movements that operate below the acceptable levels of service during one or more peak hours under Future Conditions with Development (2031). Movements in **bold** operate below acceptable levels of service under Future Conditions with Development (2026).

- Intersection 1: Main Street and West Street/North Street
 - Westbound Thru/Right (PM Peak)
 - Northbound Left/Thru, Thru/Right (AM and PM Peaks)
 - Southbound Left/Thru (PM Peak)
- Intersection 3: University Drive and North Street
 - Northbound Left/Thru, Thru/Right (AM Peak)
 - Southbound Left/Thru, Thru/Right (AM Peak)
- Intersection 4: North Street and Blenheim Boulevard
 - Eastbound Left/Thru (AM and PM Peaks)
 - Southbound Left/Thru, Thru (AM and PM Peaks)
 - Southbound Right (AM Peak)
- Intersection 5: Main Street and East Street/Blenheim Boulevard
 - Westbound Thru (PM Peak)
 - Northbound Thru, Thru/Right (AM, PM, and SAT Peaks)
- Intersection 6: Main Street and University Drive
 - Northbound Left/Thru, Thru/Right (AM Peak)
- Intersection 7: Main Street and Chain Bridge Road
 - Westbound Left (AM, PM, and SAT Peaks)
- Intersection 15: Chain Bridge Road and Armstrong Street
 - Westbound Left/Right (AM and PM Peaks)
- Intersection 16: Chain Bridge Road and Judicial Drive
 - Eastbound Left (AM, PM, and SAT Peaks)
- Intersection 18: Chain Bridge Road and South Street
 - Eastbound Left/Thru (AM and PM Peaks)
 - Eastbound Right (AM and PM Peaks)
 - Westbound Left/Thru/Right (AM and PM Peaks)

The following roadway improvements were recommended in the Future Conditions with Development (2026) and continue to be recommended in the Future with Development (2031) scenario:

- o Intersection 1: Main Street and West Street/North Street
 - Adjust signal timings to improve operations (SAT Peak only).
- o Intersection 5: Main Street and East Street/Blenheim Boulevard
 - Adjust signal timings to improve operations (PM Peak only).
- o Intersection 7: Main Street and Chain Bridge Road
 - Adjust signal timings to improve operations (PM Peak only).
- Intersection 12: University Drive and South Street
 - Install a traffic signal and add exclusive northbound left lane.



Figure 30: Future with Development (2031) – Levels of Service

Homeland Security Emergency Evacuations

The Ox Fairfax – Block A redevelopment is situated with access to all surrounding roadways. The redevelopment has a total of three (3) access points; one (1) to the north onto Sager Avenue and two (2) to the east onto University Drive. The site also has proximity to multi-lane highways and mass transit. Interstate 66 (I-66) is within two (2) miles of the site. Additionally, the site is served by five (5) bus routes. Crosswalks currently exist at the signalized intersections, and sidewalks are located along most of the roads.

Future Conditions with Development (2032) - Planning Scenario

For the purposes of this study, the development is anticipated to be fully constructed by 2026. Based on discussions with VDOT and the City, a planning level analysis was recommended for the year 2032.

Future with Development (2032) Traffic Volumes

Traffic volumes were projected by increasing the Future without Development (2031) traffic volumes to the year 2032 using an inherent growth rate of one (1) percent per year. The removed existing trips, proposed site trips, and pass-by trips were added to the inherent growth volumes to determine the Future with Development (2032) volumes. The Future with Development (2032) traffic volumes along the arterial corridors are shown in Figure 31.



Figure 31: Future with Development (2032) – Traffic Volumes

Future Conditions (2032) Volume-to-Capacity Analysis

As noted previously, the Future with Development (2032) scenario is to be used for planning purposes only. As such, in order to determine the potential traffic demand along the Main, North, Chain Bridge, and Blenheim Corridors, the projected 2032 traffic volumes with the development in place were used to determine the volume-to-capacity (V/C) ratio at eleven (11) intersections along the arterial corridors within the vicinity of the study area.

For the planning scenario, the Synchro model was used to determine the V/C ratios. V/C ratios for the Future with Development (2032) conditions are shown in Table 12.

Table 12: V/C Analysis Results

		V/C Ratios					
	Interposition (Movement)	AM Peak	PM Peak	SAT Peak			
	intersection (movement)	TF 2032	TF 2032	TF 2032			
1	Main Street and West Street/North Street						
	Overall Intersection (Signalized)						
	Eastbound Left	0.53	0.61	0.78			
	Eastbound Thru	0.65	0.50	0.57			
	Eastbound Right	0.03	0.02	0.01			
	Westbound Left	0.15	0.17	0.09			
	Westbound Thru/Right	0.65	0.83	0.59			
	Northbound Left/Thru Thru/Right	0.38	0.58	0.34			
	Southbound Left/Thru	0.52	0.31	0.27			
	Southbound Right	0.53	0.76	0.69			
2	Chain Bridge Road and North Street						
	Overall Intersection (Signalized)						
	Eastbound Thru Thru/Right	0.44	0.46	0.38			
	Westbound Thru Thru/Right	0.62	0.72	0.57			
	Northbound Thru Thru/Right	0.86	0.78	0.74			
	Southbound Left	0.32	0.62	0.54			
	Southbound Thru Thru/Right	0.44	0.54	0.40			
3	North Street and University Drive						
	Overall Intersection (Signalized)						
	Eastbound Left	0.36	0.76	0.44			
	Eastbound Thru Thru/Right	0.31	0.56	0.48			
	Westbound Left	0.08	0.24	0.18			
	Westbound Thru Thru/Right	0.43	0.79	0.59			
	Northbound Left/Thru Thru/Right	0.68	0.29	0.24			
	Southbound Left/Thru Thru/Right	0.79	0.71	0.36			
4	North Street and Blenheim Boulevard						
	Overall Intersection (Signalized)						
	Eastbound Left/Thru	0.97	1.17	0.81			
	Eastbound Right	0.24	0.37	0.26			
	Westbound Right	0.04	0.05	0.07			
	Northbound Left	0.58	0.68	0.55			
	Northbound Left/Thru/Right	0.58	0.67	0.55			
	Southbound Left/Thru Thru	0.81	1.01	0.69			
	Southbound Right	0.16	0.17	0.14			
5	Main Street and Blenheim Boulevard/East Street						
1	Overall Intersection (Signalized)	0.40	0.40	0.40			
1		0.48	0.48	0.40			
	vvestbound I hru	0.62	0.84	0.70			
	vvestbound Right	0.43	0.56	0.56			
	Northbound Left	0.04	0.04	0.10			
		0.54	0.72	0.63			
		0.42	0.60	0.49			
L_	Southbound Ihru/Right	0.34	0.78	0.47			
6	main Street and University Drive						
	Overall Intersection (Signalized)	0.00	0.07	0.04			
1		0.02	0.07	0.04			
		0.38	0.62	0.41			
		0.13	0.42	0.23			
		0.20	0.44	0.30			
		0.80	0.40	0.75			
	Soumbouna Lett/Inru Inru/Right	0.47	0.32	0.57			

		V/C Ratios								
		AM Peak	PM Peak	SAT Peak						
	Intersection (Movement)	TE 2022	TE 2022	TE 2022						
		17 2032	172032	172032						
7	Main Street and Chain Bridge Road									
	Overall Intersection (Signalized)									
	Eastbound Left	0.06	0.08	0.05						
	Eastbound Thru/Right	0.70	0.65	0.80						
	Westbound Left	0.56	0.72	0.62						
	WestboundThru/Right	0.28	0.30	0.41						
	Northbound Thru Thru/Right	0.82	0.83	0.62						
	Southbound Left/Thru Thru/Right	0.34	0.68	0.44						
8	Chain Bridge Road and Sager Avenue									
	Overall Intersection (Unsignalized)									
	Westbound Right	0.09	0.14	0.12						
	Northbound Thru Thru/Right	0.40	0.31	0.31						
	Southbound Left/Thru Thru	0.25	0.38	0.27						
15	Chain Bridge Road and Armstrong Street									
	Overall Intersection (Signalized)									
	Westbound Left/Right	0.59	0.76	0.47						
	Northbound Thru Thru/Right	0.64	0.42	0.52						
	Southbound Left	0.36	0.20	0.25						
	Southbound Thru	0.21	0.44	0.32						
16	Chain Bridge Road and Judicial Drive									
	Overall Intersection (Signalized)									
	Eastbound Left	0.59	0.60	0.42						
	Eastbound Right	0.06	0.12	0.08						
	Northbound Left	0.45	0.57	0.31						
	Northbound Thru	0.43	0.27	0.27						
	Southbound Thru	0.23	0.44	0.26						
	Southbound Right	0.03	0.04	0.02						
18	Chain Bridge Road and South Street									
	Overall Intersection (Signalized)									
	Eastbound Left/Thru	0.28	0.45	0.19						
	Eastbound Right	0.02	0.07	0.02						
	Westbound Left/Thru/Right	0.40	0.56	0.47						
	Northbound Left	0.40	0.20	0.15						
1	Northbound Thru Thru/Right	0.42	0.35	0.52						
	Southbound Left	0.03	0.05	0.08						
	Southbound Thru Thru/Right	0.27	0.41	0.48						

As shown in Table 12, two (2) lane groups at one (1) study intersection are anticipated to operate at a V/C greater than 1.0 during one (1) peak hour.

Intersection Analysis Summary

As described in previous sections, vehicular capacity analysis was performed for the following five (5) scenarios:

- Existing (2023) based on existing 2023 turning movement counts.
- Future without Development (2026) assumes existing traffic plus additional traffic due to a one (1) percent annual growth rate plus traffic due to planned yet unbuilt background developments.
- Future with Development (2026) assumes existing traffic plus additional traffic due to a one (1) percent annual growth rate plus traffic due to planned yet unbuilt background developments plus traffic generated by The Ox Fairfax Block A site.
- Future with Development (2026) Mitigated assumes existing traffic plus additional traffic due to a one (1) percent annual growth rate plus traffic due to planned yet unbuilt background developments plus traffic generated by The Ox Fairfax Block A site plus mitigations.
- Future with Development (2031) assumes existing traffic plus additional traffic due to a one (1) percent annual growth rate plus traffic due to planned yet unbuilt background developments plus traffic generated by The Ox Fairfax Block A site plus mitigations plus the South Street Extension in place.

A comparison of the MOE results for all five (5) scenarios are presented in Table 13.

Table 13: MOE Comparison Table

		Level of Service (LOS) (Sec./Veh.)				SAT Book Hour			AM Book Hour				Maximum Queues (ft.) PM Peak Hour					SAT Peak Hour														
No.	Intersection (Movement)	2023	2026 Future	2026 Total	2026 Total Future	e 2031 Total	2023	2026 Future	2026 Total	2026 Total Future	e 2031 Total	2023	2026 Future	2026 Total	2026 Total Future	2031 Total	Effective Storage	2023	2026 Future	2026 Total	2026 Total Future	2031 Total	2023	2026 Future	2026 Total	<u>ur</u> 2026 Total Future	2031 Total	2023	2026 Future	2026 Total	<u>r</u> 2026 Total Future	2031 Total
1	Main Street (E/W) and West Street/North Street (N/S)	Existing	Background	Future	Mitigated	Future	Existing	Background	Future	Mitigated	Future	Existing	Background	Future	Mitigated	Future		Existing	Background	Future	Mitigated	Future	Existing	Background	Future	Mitigated	Future	Existing	Background	Future	Mitigated	Future
	Overall Intersection (Signalized) Eastbound Left	C (30) C (29)	C (33.7) C (31)	D (35) C (31)	D (35) C (31)	C (34.9) C (31.3)	C (31.3) C (30.1)	C (34.9) C (33.9)	D (40.3) C (34.1)	C (34.1) D (36.6)	C (32.7) D (37.4)	C (29.8) C (32.8)	D (39.4) D (36.9)	D (43.6) D (37.4)	D (40.1) D (47.5)	D (41.4) D (49.2)	665	325	350	383	402	389	312	378	760	848	846	271	654	841	496	502
	Eastbound Thru Eastbound Right	C (32.8) C (22.4)	D (35.3) C (23.8)	D (37) C (23.8)	D (37) C (23.8)	D (36.5) C (23.7)	D (35.2) C (26.6)	D (40.5) C (28.9)	D (45) C (29)	C (24.9) B (17.3)	C (24.8) B (17.3)	C (34.9) C (24.5)	D (40.4) C (26.9)	D (48.2) C (26.9)	C (26.6) B (17.4)	C (26.3) B (17.4)	125	485 125	510 125	547 125	601 125	613 124	406 125	511 124	797 112	836 114	843 115	493 89	756 115	858 92	612 67	601 93
	Westbound Left Westbound Thru/Right	D (39.9) C (33.5)	D (40.6) D (35.3)	D (44.7) D (43.9)	D (44.7) D (43.9)	D (43.5) D (41.1)	D (50.2) F (94.9)	D (51) F (96.2)	D (47.5) F (136.8)	D (46) F (95.6)	D (50.2) E (76.2)	C (34.6) D (44.3)	C (31.9) D (44.4)	C (32.3) E (62.3)	B (18.9) C (22.5)	B (19.4) C (20.5)	125	124 266	125 248	125 301	125 265	125 269	92 308	125 298	125 306	117 314	125 314	32 290	124 297	125 312	115 310	123 316
	Northbound Left/Thru Thru/Right Southbound Left/Thru	E (65.9) D (45.9)	E (64.1) D (52.7)	E (62.3) D (52.1)	E (62.3) D (52.1)	E (60.2) D (51)	E (64.1) F (81)	E (63.9) E (78.6)	E (63.8) E (78)	E (64.1) E (78)	E (64.1) E (79.4)	E (57.5) D (38.2)	D (54.2) C (23.1)	D (54.1) C (23.8)	D (54.1) D (40.9)	D (54.1) D (39.9)		107 202	120 249	106 249	122 252	113 251	144 70	194 128	217 149	389 139	392 131	89 80	108 217	109 211	116 247	145 254
2	Southbound Right Chain Bridge Road (N/S) and North Street (E/W)	C (24.7)	C (27.6)	C (28.7)	C (28.7)	C (30)	B (11.9)	B (10.8)	B (10.3)	B (11.4)	B (13.6)	B (18.8)	D (39.7)	D (39.9)	D (49)	D (50.9)		315	325	328	314	331	21	76	65	108	72	315	325	316	330	334
	Overall Intersection (Signalized) Eastbound Thru Thru/Right	B (17.1) A (5.6)	B (16.9) A (7.8)	B (17.3) A (8.4)	B (17.3) A (8.4)	B (17.4) A (8.7)	D (45.2) B (11.7)	D (45) B (13.9)	D (44.9) B (14.1)	C (31) B (14.1)	C (32.3) B (15)	B (15.1) B (19.2)	B (15.2) B (16.2)	B (15.7) B (16)	B (12.8) A (2.4)	B (12.8) A (2.4)		258	258	264	269	259	186	256	268	284	288	228	249	235	213	218
	Westbound Thru Thru/Right	B (14) C (21)	B (15.8) B (16.8)	B (16.8) B (16.5)	B (16.8) B (16.5)	B (17.6) B (16.4)	D (50.9) E (84.8)	D (50.8) F (82.6)	D (50.3) E (82.8)	D (50.3) B (13.9)	D (51.2) B (18.2)	A (6.5) B (15.5)	A (7.3) B (16.9)	A (7.8) B (17)	A (7.8) B (16.9)	A (7.2) B (17.2)		264 288	296 287	313 295	287 293	319 290	343 292	338 300	338 295	341 285	336 291	140	194 274	186 276	181	202 287
	Southbound Thru Thru/Right	C (30.5) C (29.6)	C (30) C (29.1)	C (30.4) C (28.7)	C (30.4) C (28.7)	C (30.1) C (28)	C (29.6) C (31.2)	C (30.7) C (32.2)	C (34.5) C (32.6)	C (34.5) C (32.6)	D (36.1) C (33.1)	C (23.3) C (23.2)	C (23.9) C (23.7)	C (25.9) C (24)	C (25.9)	C (26.5) C (24.4)	160	144	136	144	150	159	160	160	160	160	159	156	159	159	159	159
3	University Drive (N/S) and North Street (E/W) Overall Intersection (Signalized)	B (17.1)	B (16.4)	B (18.6)	B (18.6)	B (18.1)	C (22.1)	C (24.6)	C (24.9)	C (25.6)	C (26.6)	B (18.9)	B (18.1)	B (18.8)	B (19.1)	B (19 3)																
	Eastbound Left	A (4.5)	A (6)	A (6)	A (6)	A (8)	C (21.4)	D (36.9)	D (38.9)	D (38.9)	D (47.5)	B (11.1) B (14.1)	B (11.6) B (10.7)	B (13) B (12 7)	B (11.5) B (11.3)	B (14) B (10.8)	215	179 208	187	172	182	185 236	191 277	215 314	214	215	215	120 166	134	134 196	195 288	215 314
	Westbound Left	A (2.3)	A (2.7)	A (2.6)	A (2.6)	A (2.6)	A (6.1)	A (6.9)	A (7.1)	A (7)	A (7.3)	B (10.2)	A (9.5)	A (9.7)	B (10.7)	B (10.4)	150	64	99	120	91	83	149	149	149	139	139	134	133	140	139	132
	Northbound Left/Thru Thru/Right	E (71.9)	E (59.8)	E (74.8)	E (74.7)	E (69.5)	B (16) B (16)	B (17.9) B (16.5)	B (17.9) B (16.9)	B (17.0) B (19.8)	B (19) B (19.2)	A (9.6)	B (10.1)	B (10.8)	B (10.3)	A (9.5)		200	197	196	174	194	99 95	218	190	172	154	87	334 89	116	116	114
4	North Street (EW) and Blenheim Boulevard (N/S)	C (20)	C (20.7)	C (34)	C (03.0)	C (32.4)	0 (41.2)	D (92.9)	D (40.4)	D (43.4)	D (40.0)	C (30.3)	C (31)	C (30.2)	C (30.E)	C (31.4)		230	220	2.52	214	211	305	700	703	134	100	210	200	204	255	
	Eastbound Left/Thru	F (90.9)	F (93.9)	F (94.6)	F (94.6)	F (99.3)	F (97.7)	F (155.3)	F (152.9)	F (153)	F (172.6)	D (45.8)	D (48)	D (47.8)	D (45.5)	D (47.1)		140	140	140	140	140	140	140	140	140	140	140	140	140	140	140
	Eastbound Right Westbound Right	A (2) A (0)	A (2.5) A (0)	A (2.5) A (0)	A (2.5) A (0)	A (2.5) A (0)	A (2.8) A (0.1)	A (2.6) A (0.1)	A (2.7) A (0.1)	A (2.6) A (0.1)	A (2.5) A (0.1)	A (0.1)	B (18.3) A (0.1)	B (18.4) A (0.1)	A (0.1)	A (0.1)		0	0	0	0	0	0	11	0	0	0	0	0	0	10	348 0
	Northbound Left Northbound Left/Thru/Right	B (14.3) B (14.1)	B (14.1) B (13.9)	B (14.1) B (14)	B (14.1) B (14)	B (15) B (14.8)	B (14.3) B (14.2)	B (14.8) B (14.7)	B (14.8) B (14.8)	B (14.8) B (14.7)	B (15.5) B (15.3)	B (18.7) B (18.7)	C (20.2) C (20.1)	C (21.9) C (21.8)	C (26.8) C (26.7)	C (28.5) C (28.4)		318	343	367	364	367	381	385	393	384	393	376	373	369	378	374
_	Southbound Lett Infu Infu Southbound Right	E (73) E (57.4)	E (71.7) E (57.6)	E (72.3) E (57.6)	E (72.3) E (57.6)	E (74) E (57.5)	P (83.4) D (51.1)	D (51.3)	P (95.3) D (51.3)	F (95.3) D (51.3)	P (99.7) D (51.4)	D (52.8) D (45.5)	D (52.1) D (45.2)	D (52.2) D (44.9)	D (52.2) D (44.9)	D (52.4) D (44.8)	750	164	235 164	234 206	192	249 212	632	624	625	622	623	160	190	295 193	530	613
5	Wain Street (E/W) and East Street/Bienneim Boulevard (N/S) Overall Intersection (Signalized)	C (23.7)	C (24.2)	C (25.3)	C (25.3)	C (24.8)	C (27.3)	C (29)	C (34)	C (29.3)	C (29)	C (24.7)	C (24.3)	C (26.2)	C (25.2)	C (25)																
	Eastbound Thru Thru/Right Westbound Thru	C (34.6) D (42.2)	D (37.2) D (44.1)	D (38.5) D (46.1)	D (38.4) D (46.1)	D (37.8) D (46.3)	D (51.1) D (52.7)	E (55.9) E (59.7)	E (58.7) F (89.9)	D (39.5) E (57.1)	D (39.3) E (57.3)	D (47.9) C (32.5)	D (46.5) C (33.8)	D (44.5) D (41.9)	D (40.6) C (33.7)	D (40.7) C (33.7)		282 224	304 223	299 224	294 224	295 224	315 225	299 225	316 225	163 225	158 225	310 224	317 225	314 225	309 225	312 225
	Westbound Right Northbound Left	A (3.4) D (51.4)	A (3.4) D (51.4)	A (3.4) D (51.4)	A (3.4) D (51.4)	A (3.6) D (51.4)	A (8.4) D (51.3)	A (8.4) D (51.3)	A (8.1) D (51.3)	A (8.1) D (52.2)	A (8.7) D (52.2)	B (15.3) D (48.9)	B (15) D (49.1)	B (15.6) D (48.1)	B (13.6) D (48.9)	B (14.2) D (49)	105	309 92	390 90	376 101	434 85	399 82	602 104	602 104	604 104	609 83	606 104	423 71	516 78	586 75	561 98	592 104
	Northbound Thru Thru/Right Southbound Left	E (58.3) B (17.6)	E (58.3) B (17.1)	E (59.3) B (16.8)	E (59.3) B (16.8)	E (59.3) B (16.8)	E (62.3) B (12)	E (62.3) B (12.1)	E (64.1) B (11.8)	E (66.8) B (15)	E (66.4) B (15.5)	D (53.3) A (7.6)	D (53.4) A (6.9)	D (53.4) A (6.8)	E (55.3) B (10.7)	E (55.4) B (10.7)		201 263	181 273	202 275	195 275	195 289	275 302	280 311	278 310	277 313	278 313	172 290	146 285	173 286	242 307	251 304
6	Southbound Thru/Right Main Street (E/W) and University Drive (N/S)	D (41.8)	D (41.3)	D (41.4)	D (41.4)	D (41)	D (40.9)	D (42.1)	D (43.2)	D (48.3)	D (48.1)	C (20.7)	B (18.8)	B (19.5)	C (26.5)	C (25.7)		243	250	247	254	264	292	285	295	293	292	266	263	262	289	291
	Overall Intersection (Signalized) Eastbound Left	C (20.4) A (2.1)	C (21) A (2.7)	B (19.9) A (3.6)	B (19.9) A (3.6)	C (20.5) A (3.4)	B (16.3) B (12.5)	B (16.4) B (13.8)	B (17.6) B (13.6)	C (21) A (10)	C (20.1) A (9.9)	B (18) A (3.6)	B (17.6) A (3.5)	B (19.3) A (6.1)	B (18.9) A (6.1)	B (18.5) A (4.8)	90	42	49	60	66	60	62	80	89	79	83	69	84	88	85	88
	Eastbound Thru/Right Westbound Left	A (3) A (0.1)	A (4.3) A (0.2)	A (5.3) A (0.3)	A (5.3) A (0.3)	A (5.1) A (0.3)	B (15.4) A (8)	B (17.3) A (7.7)	B (17.5) B (10.1)	C (29.1) B (10.1)	C (28.2) B (10.2)	A (5.6) A (2.3)	A (6.1) A (2.4)	A (8.9) A (3.2)	A (8.9) A (2.2)	A (7.9) A (1.6)	125	120 83	160 86	191 108	156 111	192 110	197 90	198 96	275 118	324 106	326 107	185 81	231 80	272 118	278 121	255 124
	Westbound Thru/Right Northbound Left/Thru Thru/Right	A (0.7) E (62.3)	A (0.8) E (62.9)	A (0.9) E (62.3)	A (0.9) E (62.3)	A (0.9) E (64)	B (10.3) C (28.4)	B (10.1) C (28.4)	A (8) C (30.1)	B (11.8) C (29.5)	B (11.8) C (28.5)	A (3.3) D (53.9)	A (3.5) D (54)	A (4.1) D (48.5)	A (3.2) D (48.6)	A (2.5) D (53.4)		43 102	69 105	84 109	88 153	92 148	128 103	191 114	235 112	137 152	136 151	124 103	151 105	214 103	237 151	236 144
7	Southbound Left/Thru Thru/Right Main Street (E/W) and Chain Bridge Road (N/S)	C (30.4)	D (36.1)	B (18.6)	B (18.6)	C (21)	B (12.3)	B (11.8)	B (15.6)	B (15.6)	B (14.4)	D (36.1)	D (36)	C (32.6)	C (32.1)	C (34.5)		160	178	198	153	174	132	158	218	137	143	197	186	210	194	194
	Overall Intersection (Signalized) Eastbound Left	C (28.5) A (9.4)	C (28.1) A (9.8)	C (29.5) B (11.8)	C (29.5) B (11.8)	C (28.6) B (12)	D (37.4) A (9.7)	D (39.2) B (12.6)	D (39.1) B (11)	C (34.1) B (18.9)	C (32.3) C (20.1)	C (20.5) A (5.1)	C (21.3) A (6.2)	C (21.8) A (5.5)	C (24.1) B (12.7)	C (24.3) B (12.3)	125	53	63	83	105	104	90	119	119	122	125	89	124	100	115	110
	Eastbound Thru/Right Westbound Left	B (13.3) F (74.2)	B (13.1) E (75.2)	B (15.4) E (70.3)	B (15.4) E (70.4)	B (15.9) E (71.8)	B (17.2) E (87.2)	C (23.2)	C (25.2)	C (25)	C (26.5) E (59.5)	B (14.9) E (73)	B (15.2) E (76.1)	B (18.1) F (71.2)	C (27)	C (26.2)	200	259 167	249 167	266 172	283 186	288 190	301 196	307 198	316 198	322 200	325 200	296 165	311 190	316 195	318 199	317 195
	Westbound Thru/Right	B (14.8)	B (16.6)	C (20.3)	C (20.3)	B (19.6)	B (10.2)	B (10.5)	B (13.9)	C (31)	C (34.9)	B (10)	A (9.9)	B (12.1)	B (16.6)	B (15.2)	200	190	242	219	238	228	264	276	301	322	321	140	207	235	269	286
•	Southbound Left/Thru Thru/Right Chain Bridge Road (NIS) and Sager Avenue (EMI)	B (11)	B (11.3)	B (11.7)	B (11.7)	B (12.3)	D (43.6)	D (43.9)	D (44.2)	B (14.1)	B (11.3)	A (8.9)	A (8.9)	A (8.9)	A (8.9)	A (9.2)		150	181	219	208	214	288	306	294	220	187	142	185	208	251	246
	Overall Intersection (Unsignalized) Westhound Right	B (12.4)	B (117)	B (12)	B (12)	B (10.8)	B (11.7)	B (11.4)	B (11.7)	B (11.7)	B (10.6)	B (11.6)	B (10.6)	A (9.5)	A (9.5)	A (9.6)		87	82	106	90	100	128	150	176	111	89	79	84	85	134	134
•	Southbound Left/Thru Thru	A (2.7)	A (2.5)	A (5)	A (5)	A (4.6)	A (2.2)	A (2.1)	A (4.8)	A (4.8)	A (4.5)	A (2.6)	A (2.4)	A (4.7)	A (4.7)	A (4.5)		164	166	254	240	226	275	286	316	277	249	90	109	155	183	188
3	Overall Intersection (Unsignalized)	4 (1 2)	A (1.2)	A (E 1)	A (E 1)	A (E)	A (0.7)	A (0.8)	A (4 7)	A (4 7)	A (4 7)	4 (0.0)	A (0.0)		A (4.4)	A (4.4)		24	22	60	60	60	21	54	110	75	60	15	26	01	01	102
	Westbound Left/ I hru Northbound Left/Right	A (1.2) A (9)	A (1.3) A (9)	A (5.1) A (9.8)	A (5.1) A (9.8)	A (5) A (9.8)	A (0.7) A (9.3)	A (0.8) A (9.2)	A (4.7) B (10.7)	A (4.7) B (10.7)	A (4.7) B (10.7)	A (0.9) A (9.2)	A (0.9) A (9.1)	A (4.4) B (10.6)	A (4.4) B (10.6)	A (4.4) B (10.5)		24 24	33 30	63 82	62 70	60 80	21 52	54 80	116 129	75 123	60 111	15 41	26 58	81 96	91 109	103
10	University Drive (N/S) and Sager Avenue (E/W) Overall Intersection (Signalized)	B (11.2)	B (10.9)	B (10.7)	B (10.7)	B (11)	B (10.2)	B (10.1)	B (10.3)	A (10)	A (9.8)	A (10)	A (9.8)	A (9.5)	A (9.9)	A (9.7)				450	450		400	407								
	Eastbound Left/ I hru/Right Westbound Left	C (33.1) C (29.5)	C (31.4) C (29.5)	C (29.4) C (25.3)	C (29.4) C (25.3)	C (31.8) C (26)	D (36.2) C (27.7)	D (36.2) C (27.7)	C (33.7) C (23.7)	C (28.8) C (23.7)	C (28.6) C (24.2)	C (24.9) C (24.5)	C (24.7) C (24.5)	C (24.4) B (19.8)	C (24.4) B (19.8)	C (23.9) C (20.4)	130	119	117 80	150 100	150 89	148 86	182 102	187 104	184 96	148 77	148 91	104 49	96 51	147 63	157 61	144 56
	Westbound I hru/Right Northbound Left/Thru Thru/Right	C (30.1) A (2.8)	C (30) A (2.8)	C (25.3) A (1.8)	C (25.3) A (1.8)	C (25.9) A (1.7)	C (27.6) A (3.7)	C (27.6) A (3.7)	C (23.5) A (3.5)	C (23.5) A (5.6)	C (24) A (5.3)	C (25.5) A (3.2)	C (25.5) A (3.2)	C (20.3) A (4.3)	C (20.3) A (4.3)	C (21) A (3)		135 84	124 81	128 88	139 80	125 79	131 97	174	211 209	170 194	119 185	117 80	134 86	161 120	192 125	186 125
11	Southbound Left/Thru Thru/Right Sager Avenue (E/W) and South Street/East Street (N/S)	A (0.7)	A (0.8)	A (1)	A (1)	A (0.9)	A (1.1)	A (1.1)	A (3.1)	A (2.7)	A (2.5)	A (1.8)	A (1.7)	A (2.6)	A (3.3)	A (3.1)		81	91	113	74	77	111	117	128	133	137	80	80	120	93	106
	Overall Intersection (Unsignalized) Eastbound Left	A (8.2)	A (8)	A (8.2)	A (8.2)	A (8.2)	A (9.7)	A (9.5)	A (9.9)	A (9.9)	A (9.9)	A (7.9)	A (7.9)	A (8.2)	A (8.2)	A (8.1)	90	55	44	60	57	66	88	89	89	89	89	45	42	48	66	50
	Eastbound Thru/Right Westbound Left	A (7.8) A (7.8)	A (7.7) A (7.7)	A (7.7) A (7.8)	A (7.7) A (7.8)	A (7.7) A (7.8)	B (10.7) A (9.1)	A (10.4) A (9)	B (10.6) A (9.2)	B (10.6) A (9.2)	B (10.6) A (9.1)	A (7.8) A (7.7)	A (7.8) A (7.7)	A (7.8) A (7.8)	A (7.8) A (7.8)	A (7.8) A (7.8)	80	61 63	72 59	66 58	61 67	65 59	251 80	251 80	261 80	314 80	323 80	56 40	65 46	65 36	91 51	66 43
	Westbound Thru/Right Northbound Left/Thru/Right	A (9.7) A (8.9)	A (9.2) A (8.7)	A (9.3) A (8.8)	A (9.3) A (8.8)	A (9.3) A (8.8)	B (11.4) B (11.3)	A (11) A (11)	B (11.3) B (11.4)	B (11.3) B (11.4)	B (11.3) B (11.3)	A (8.8) A (8.4)	A (8.7) A (8.3)	A (8.9) A (8.5)	A (8.9) A (8.5)	A (8.9) A (8.5)		104 74	99 67	98 65	98 71	95 71	397 380	400 398	393 387	396 431	397 423	99 64	116 57	114 58	212 73	163 62
12	Southbound Left/Thru/Right University Drive (N/S) and Entrance Drive/South Street (E/W)	A (9.3)	A (9)	A (9.1)	A (9.1)	A (9.1)	B (13.2)	A (12.8)	B (13.3)	B (13.3)	B (13.3)	A (9.1)	A (9)	A (9.2)	A (9.2)	A (9.2)		147	164	154	154	163	252	251	249	262	272	120	126	127	158	141
	Overall Intersection (Unsignalized - EX 2023, FB 2026) (Signalized - TF 2026) Eastbound Left/Thru/Right (TF 2026)	1 :	-	A (6) A (0)	A (6) A (0)	A (6.1) C (28.8)	-	-	A (9.6) C (26.7)	B (13.6) C (31.1)	B (12.9) C (29.4)	:	-	A (7.1) C (27.9)	A (7) C (27.9)	A (6.2) C (24.8)		-		0	40	48		-	113	120	59	-		97	63	52
	Westbound Left/Right (EX 2023), Westbound Left/Thru/Right (TF 2026) Northbound Left (TE 2026 - MIT)	B (13.9)	B (13.8)	C (29.7)	C (29.7) A (3)	C (30.7) A (2.5)	B (17.3)	C (18.7)	C (33.6)	D (41.7) A (6.3)	D (40.9) A (6.1)	B (11.7)	B (11.5)	C (28.4)	C (28.4) A (2.8)	C (28.3) A (2.7)	50	72	77	118	105 27	116 25	92	93	140	165 70	135	64	75	87	96 48	108 46
	Northbound Left/Thru/Right (TF 2026), Northbound Left/Thru (TF 2026 - MIT) Southbound Left	- A (8)	- A (8)	A (4.2)	A (4.1)	A (3.6)	A (8.3)	A (8 3)	A (6.5)	A (8.4)	A (8.4)	- A (7.8)	A (7 7)	A (3.6)	A (3.4)	A (3.4)		- 25	- 28	129	120	129	- 21	-	143 28	145	147	-	- 12	116	115	118
12	Southbound Thru/Right (TF 2026)	-	-	A (2.4)	A (2.4)	A (2)	-	-	A (4.2)	A (9.4)	A (9.3)	-	-	A (2.3)	A (2.4)	A (2.5)		-	-	104	86	99	-	-	162	161	146	-	-	108	89	100
	Overall Intersection (Unsignalized)	P (11.2)	R (11)	R (11.5)	R (11.5)	R (11.7)	B (12.7)	P (12.2)	P (12.5)	R (12.4)	P (12 7)	R (10.2)	R (10)	R (11)	P (11)	P (11.2)		22	20	54	49	47	46	52	105	124	144	22	20	69	74	91
	Northbound Left/Thru/Right (TF 2031)	-	- - A (7.0)	- - A (7.0)	-	A (0.1)	-	- A (9.2)	-	-	A (0.3)	-	- -	-	-	A (0.4)	RE EQ (MIT)	-	-	-	-	37	-	-	-	-	510	-	-	-	-	150
14	University DriveGeorge Mason Boulevard (N/S) and Armstrong Street (E/W)	R (11.9)	R (11.7)	R (11.7)	R (11.7)	R (11.7)	R (0.3)	R (0.2)	R (0.4)	R (0.4)	R (0.4)	R (11.4)	R (11.4)	R (9.1)	R (9.1)	R (9.1)	65, 50 (MIT)	2	2	4	2	5	5	9	5	4	14	10	5	0	2	
	Eastbound Left/Thru/Right	B (16.7)	B (16.6)	B (17.2)	B (17.2)	B (17.2)	C (20.1)	C (22.3)	C (20.2)	C (20.2)	B (20)	B (19.5)	B (11.4) B (19.6)	B (12) B (16.6)	B (12) B (16.6)	B (16.4)		130	96	114	116	126	128	143	164	201	243	114	122	141	120	133
	Westbound Left/Thru/Right Northbound Left	B (15.6) A (7.8)	B (15.7) A (7.9)	B (16.1) A (7.8)	B (16.1) A (7.8)	B (16.1) A (7.8)	B (19) A (7.4)	C (20.5) A (6.7)	B (18.4) A (8.4)	B (18.4) A (8.4)	B (18.3) A (8.4)	B (16.5) A (6.8)	B (16.5) A (6.8)	B (14.5) A (8.2)	B (14.5) A (8.2)	B (14.4) A (8.3)	160	91 31	96 41	107 39	105	116 55	118 65	124 93	202 75	224	148 135	87 30	87 31	127 40	83 35	147 31
	Northbound I hru/Right Southbound Left	B (10.2) A (7.8)	B (10.1) A (7.8)	A (9.9) A (7.7)	A (9.9) A (7.7)	A (10) A (7.7)	A (9.6) A (7.2)	A (9.2) A (7.5)	B (11.2) A (9.1)	B (11.2) A (9.1)	B (11.4) A (9.1)	A (8.2) A (6.8)	A (8.3) A (6.8)	B (10) A (8.3)	B (10) A (8.3)	B (10.1) A (8.3)	230	31 37	134 37	136 36	130 40	153 38	1// 79	180 54	262	364 34	390 57	120	163 43	170	136 34	167 39
15	Southbound Thru/Right Chain Bridge Road (N/S) and Armstrong Street (E/W)	B (10.5)	B (10.4)	B (10.4)	B (10.4)	B (10.4)	B (11.3)	B (11.7)	B (15.1)	B (15.1)	B (14.9)	A (8.5)	A (8.6)	B (10.6)	B (10.6)	B (10.5)		194	184	187	188	181	232	259	241	235	217	141	162	160	130	166
	Overall Intersection (Signalized) Westbound Left/Right	A (7.3) E (65)	A (8.3) E (65)	A (9.6) E (67.4)	A (9.6) E (67.4)	A (9.3) E (66.9)	B (17) E (69.5)	B (17.1) E (69.7)	B (18.9) E (72.3)	B (18.9) E (72.3)	B (17.6) E (70.2)	A (8.1) D (36.9)	A (8.9) D (35.3)	B (10.6) D (36.4)	B (10.6) D (36.4)	A (9.8) D (35.7)		182	184	204	209	226	283	356	342	343	285	114	161	170	179	169
	Northbound Thru Thru/Right Southbound Left	A (5) A (9.3)	A (6) B (19.9)	A (6.7) B (18.9)	A (6.7) B (18.9)	A (6.7) C (26)	C (22) A (2.5)	C (24) A (2.4)	C (26.1) A (2.7)	C (26.1) A (2.7)	C (25.2) A (2.5)	A (9) A (3.7)	B (10.3) A (4.5)	B (11.9) A (5.6)	B (11.9) A (5.6)	B (11.4) A (5.3)	80	227 79	362 77	355 79	325 79	515 79	376 73	485 78	545 76	577 77	474 79	236 76	294 79	315 80	440 79	413 77
16	Southbound Thru Chain Bridge Road (N/S) and Judicial Drive (E/W)	A (2.1)	A (1.9)	A (2.6)	A (2.6)	A (1.9)	A (2.3)	A (2.2)	A (2.4)	A (2.4)	A (2.2)	A (3.4)	A (4)	A (4.7)	A (4.7)	A (4.4)		215	169	208	213	206	252	259	240	289	286	186	223	193	236	200
	Overall Intersection (Signalized) Eastbound Left	B (15) E (70.6)	B (14.5) E (70.1)	B (14.4) E (70.1)	B (14.4) E (70.1)	B (10.6) E (70.1)	B (15.4) E (66.3)	B (17.4) E (66.3)	B (17) E (66.3)	B (16.8) E (66.3)	B (18.4) E (66.3)	B (10.2) E (59.7)	A (9.6) E (59.7)	A (9.5) E (59.7)	A (9.5) E (59.7)	A (9.4) E (59.7)	410	153	136	146	139	150	170	363	360	277	212	90	98	90	88	134
	Eastbound Right Northbound Left	D (50.4) A (5.4)	D (47.6) A (6)	D (48) A (5.8)	D (48) A (5.8)	D (46.5) A (6.1)	D (45.6) B (15.8)	D (42.8) D (35.2)	D (42.8) C (33.2)	D (42.8) C (33.2)	D (42.8) D (41.5)	D (48) A (1.8)	D (47.8) A (2)	D (47.8) A (2)	D (47.8) A (2)	D (47.8) A (2.1)	160	85 160	76 160	83 160	72 160	75 160	117 161	557 160	737 159	277 160	198 160	82 135	80 149	79 147	89 149	110 151
	Northbound Thru Southbound Thru	A (5.7) B (19.7)	A (6.2) B (19.7)	A (6.1) B (19.3)	A (6.1) B (19.3)	A (6.2) A (5.6)	A (1.4) A (8.3)	A (1.5) B (10.3)	A (1.5) B (10.2)	A (1.5) B (10)	A (1.5) B (12.1)	A (1.8) A (5.2)	A (1.9) A (5.5)	A (1.9) A (5.5)	A (1.9) A (5.5)	A (1.9) A (5.6)		289 252	292 260	295 231	293 261	300 269	282 174	312 231	308 218	293 284	283 314	162 182	196 218	231 195	255 222	240 207
17	Southbound Right Chain Bridge Road (N/S) and Humane Society Driveway (E/W)	D (37.1)	C (34.5)	C (34.1)	C (34.1)	A (2.2)	A (5.9)	A (7.3)	A (6.5)	A (3.6)	A (7.9)	A (4.3)	A (4.4)	A (4.4)	A (4.4)	A (4.4)	240	117	136	96	170	150	47	69	82	62	184	54	78	40	66	100
	Overall Intersection (Unsignalized)	A (D)					A (0)					C (15)											0					40				
40	Southbound Left	A (0)	-			-	A (0)	-		-		A (9.3)	-	-	-		90	ŏ		-		-	ő	-			-	21	-	-		
18	Overall Intersection (Signalized) Eachering Left	-	A (3.4)	A (3.5)	A (3.5)	B (13.6)	-	A (9.3)	A (9)	A (8.4)	B (14.7)		A (7.4)	B (10.7)	B (10.7)	B (15.3)			88	87	59	87		120	140	147	120		41	ac	20	58
	Eastbound Thru/Right	-	E (00.5) E (64.3)	E (00.5) E (64.3)	E (00.5) E (64.3)	E (66.7) E (64.1)		E (61.5)	E (61.5)	E (61.5)	E (65) E (61)		E (60.9)	C (23.9) C (22.8)	C (23.9) C (22.8)	C (30.6) C (29.5)		:	58	62	58 72	48	-	100	92	79	90		41 47	30 48	39 56	60
	VVestoounia Leit/Inru/Hight (IF 2026) Northbound Left	-	A (1.5)	A (U) A (1.4)	A (U) A (1.4)	E (58.3) A (4.6)	-	A (1.8)	A (0) A (1.6)	A (U) A (1.6)	E (66.6) A (4.4)	-	A (2.8)	A (6.2)	A (6.2)	C (29.1) A (8.3)	230	1	0 163	212	68 216	/1 229	-	230	230	130	138	-	56	51 75	1/5	140
	Normbound Thru Thru/Right Southbound Left (TF 2026)	-	A (2)	A (2.1) A (0)	A (2.1) A (0)	A (7.5) B (17.6)	-	A (1.9)	A (1.9) A (0)	A (1.9) A (0)	A (6.3) A (7.2)	1	A (4.9)	A (9.7) A (8.6)	A (9.7) A (8.6)	ы (13.6) В (11.2)	90	1	196 0	320 0	346 83	394 78	-	412 0	407 0	341 30	339 85	1	155 30	227 30	356 81	338 68
19	Southbound Thru Thru/Right West Street (N/S) and South Street (E/W)		A (1.2)	A (1.4)	A (1.4)	C (20.9)	-	A (6.9)	A (6.5)	A (5.3)	A (9.6)	-	A (6)	B (11.2)	B (11.2)	B (15.2)		-	97	104	280	299	-	248	207	215	286	-	288	201	241	239
	Overall Intersection (Unsignalized) Eastbound Left/Thru/Right	-	A (0.8)	A (0.8)	A (0.8)	A (0.7)	-	A (1.2)	A (1.2)	A (1.2)	A (1.2)	-	A (1.9)	A (1.9)	A (1.9)	A (1.5)		-	15	21	14	18	-	34	63	72	38	-	6	6	3	15
L	Southbound Left/Thru/Right	·	B (10.4)	B (10.4)	B (10.4)	B (10.5)		B (10.3)	B (10.4)	B (10.4)	B (10.7)		A (9)	A (9)	A (9)	A (9.2)	I		42	46	50	44	-	38	36	38	40	-	43	45	46	48

VDOT Chapter 536 Compliance

This section provides the results of a Chapter 536 volume-to-capacity ratio (V/C) analysis in order to satisfy VDOT requirements. Per the Chapter 536 Code, VDOT shall specify by name and location any transportation facility within the scope of the review having a functional classification of minor arterial or higher for which an increase in traffic volume is expected to exceed the capacity of the facility as a result of the proposed plan.

Four (4) roadways within the study area have a functional classification of minor arterial or higher:

- Chain Bridge Road (Other Principal Arterial)
- Main Street (Other Principal Arterial)
- North Street (Other Principal Arterial)
- Blenheim Boulevard (Minor Arterial)

The V/C ratios were assessed for seventeen (17) roadway segments using the approach traffic volumes as shown in Table 14. For the purposes of this analysis, the capacity used was based on the industry standard of 1,900 vehicles per hour per lane on an interrupted-flow thoroughfare.

Table 14: V/C Assessment

		AM Peak Hour			PM Peak Hour	SAT Peak Hour				
Segment	Volume per Hour (V)	Hourly Capacity (C)	V/C	Volume per Hour (V)	Hourly Capacity (C)	V/C	Volume per Hour (V)	Hourly Capacity (C)	V/C	
Main Street (West of West Street)										
Existing 2023	2,010	3,800	0.53	1,744	3,800	0.46	1,247	3,800	0.33	
Future without Development 2026	2,080	3,800	0.55	2,507	3,800	0.66	1,947	3,800	0.51	
Future with Development 2026	2.140	3.800	0.56	2.631	3.800	0.69	2.092	3.800	0.55	
Main Street (Between West Street and Chain Bridge Road)		-,			- ,					
Existing 2023	592	3.800	0.16	625	3.800	0.16	613	3.800	0.16	
Future without Development 2026	634	3.800	0.17	712	3.800	0.19	678	3.800	0.18	
Future with Development 2026	694	3,800	0.18	828	3,800	0.22	809	3,800	0.21	
Main Street (Between Chain Bridge Road and University Drive)		.,			-,			-,	-	
Existing 2023	647	3.800	0.17	779	3.800	0.21	801	3.800	0.21	
Future without Development 2026	725	3.800	0.19	869	3.800	0.23	878	3,800	0.23	
Future with Development 2026	748	3.800	0.20	929	3.800	0.24	958	3,800	0.25	
Main Street (Between University Drive and Blenheim Boulevard)		-,			-,			-,		
Existing 2023	632	3.800	0.17	734	3.800	0.19	767	3.800	0.20	
Future without Development 2026	698	3.800	0.18	813	3.800	0.21	834	3.800	0.22	
Future with Development 2026	740	3.800	0.19	904	3.800	0.24	945	3,800	0.25	
Main Street (East of Blenheim Boulevard)					- ,					
Existing 2023	1,935	3,800	0.51	2,479	3,800	0.65	2,001	3,800	0.53	
Future without Development 2026	2,061	3,800	0.54	2,624	3,800	0.69	2,116	3,800	0.56	
Future with Development 2026	2,117	3,800	0.56	2,735	3,800	0.72	2,256	3,800	0.59	
North Street (Between Main Street and Chain Bridge Road)										
Existing 2023	1,461	7,600	0.19	1,730	7,600	0.23	1,245	7,600	0.16	
Future without Development 2026	1,606	7,600	0.21	1,939	7,600	0.26	1,418	7,600	0.19	
Future with Development 2026	1,666	7,600	0.22	1,950	7,600	0.26	1,436	7,600	0.19	
North Street (Between Chain Bridge Road and University Drive)										
Existing 2023	1,599	7,600	0.21	1,996	7,600	0.26	1,486	7,600	0.20	
Future without Development 2026	1,720	7,600	0.23	2,175	7,600	0.29	1,637	7,600	0.22	
Future with Development 2026	1,738	7,600	0.23	2,237	7,600	0.29	1,711	7,600	0.23	
North Street (Between University Drive and Blenheim Boulevard)										
Existing 2023	1,523	5,700	0.27	1,961	5,700	0.34	1,458	5,700	0.26	
Future without Development 2026	1,621	5,700	0.28	2,094	5,700	0.37	1,519	5,700	0.27	
Future with Development 2026	1,626	5,700	0.29	2,110	5,700	0.37	1,593	5,700	0.28	
Chain Bridge Road (North of North Street)										
Existing 2023	1,306	7,600	0.17	1,313	7,600	0.17	1,174	7,600	0.15	
Future without Development 2026	1,408	7,600	0.19	1,444	7,600	0.19	1,271	7,600	0.17	
Future with Development 2026	1,451	7,600	0.19	1,523	7,600	0.20	1,372	7,600	0.18	
Chain Bridge Road (Between North Street and Main Street)										
Existing 2023	1,258	7,600	0.17	1,222	7,600	0.16	1,090	7,600	0.14	
Future without Development 2026	1,348	7,600	0.18	1,340	7,600	0.18	1,172	7,600	0.15	
Future with Development 2026	1,373	7,600	0.18	1,366	7,600	0.18	1,215	7,600	0.16	
Chain Bridge Road (Between Main Street and Sager Avenue)										
Existing 2023	1,393	7,600	0.18	1,468	7,600	0.19	1,245	7,600	0.16	
Future without Development 2026	1,533	7,600	0.20	1,647	7,600	0.22	1,371	7,600	0.18	
Future with Development 2026	1,591	7,600	0.21	1,719	7,600	0.23	1,461	7,600	0.19	

		AM Peak Hour			PM Peak Hour	SAT Peak Hour			
Segment	Volume per Hour (V)	Hourly Capacity (C)	V/C	Volume per Hour (V)	Hourly Capacity (C)	V/C	Volume per Hour (V)	Hourly Capacity (C)	V/C
Chain Bridge Road (Between Sager Avenue and South Street)									
Existing 2023	1,367	7,600	0.18	1,403	7,600	0.18	1,149	7,600	0.15
Future without Development 2026	1,507	7,600	0.20	1,582	7,600	0.21	1,276	7,600	0.17
Future with Development 2026	1,517	7,600	0.20	1,589	7,600	0.21	1,295	7,600	0.17
Chain Bridge Road (Between South Street and Judicial Drive)									
Existing 2023	1,489	7,600	0.20	1,491	7,600	0.20	1,184	7,600	0.16
Future without Development 2026	1,663	7,600	0.22	1,709	7,600	0.22	1,335	7,600	0.18
Future with Development 2026	1,669	7,600	0.22	1,721	7,600	0.23	1,358	7,600	0.18
Chain Bridge Road (Between Judicial Drive and Armstrong Street)									
Existing 2023	1,815	7,600	0.24	1,917	7,600	0.25	1,509	7,600	0.20
Future without Development 2026	2,022	7,600	0.27	2,146	7,600	0.28	1,689	7,600	0.22
Future with Development 2026	2,028	7,600	0.27	2,159	7,600	0.28	1,712	7,600	0.23
Chain Bridge Road (South of Armstrong Street)									
Existing 2023	1,786	3,800	0.47	1,862	3,800	0.49	1,489	3,800	0.39
Future without Development 2026	1,976	3,800	0.52	2,077	3,800	0.55	1,662	3,800	0.44
Future with Development 2026	2,004	3,800	0.53	2,137	3,800	0.56	1,777	3,800	0.47
Blenheim Boulevard (Between Main Street and North Street)									
Existing 2023	1,632	7,600	0.21	2,313	7,600	0.30	1,563	7,600	0.21
Future without Development 2026	1,696	7,600	0.22	2,395	7,600	0.32	1,624	7,600	0.21
Future with Development 2026	1,703	7,600	0.22	2,420	7,600	0.32	1,656	7,600	0.22
Blenheim Boulevard (North of North Street)									
Existing 2023	973	7,600	0.13	1,167	7,600	0.15	897	7,600	0.12
Future without Development 2026	1,035	7,600	0.14	1,256	7,600	0.17	974	7,600	0.13
Future with Development 2026	1,047	7,600	0.14	1,277	7,600	0.17	999	7,600	0.13

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As shown in Table 14, the roadway segments along Main Street, North Street, Chain Bridge Road, and Blenheim Boulevard are anticipated to operate at a V/C ratio of 0.72 or less, indicating sufficient capacity along the roadway network.
Conclusions

This report presented the findings of a Transportation Impact Study (TIS) conducted for the proposed redevelopment of The Ox Fairfax – Block A site in the City of Fairfax, Virginia.

The analysis presented in this report supports the following major finding:

• After the addition of redevelopment traffic, one (1) intersection has movements that begin to operate below acceptable levels of service as compared to the Future Conditions without Development (2026), after mitigations.

Additional assumptions, findings, and conclusions are as follows:

TIA Components

- As determined based on discussions with the City, a growth rate of one (1) percent per year was applied to major movements at the study intersections to account for regional growth in background traffic volumes.
- A Mode Split/TDM reduction of fifteen (15) percent was applied to the theater (attendees and employees), hotel, and office uses, as agreed upon with the City.
- The site is expected to generate approximately 259 trips in the AM peak hour, 511 trips in the PM peak hour, and 580 trips in the Saturday peak hour, after reductions.

Infrastructure

- Existing vehicular access is provided via two (2) driveways on Chain Bridge Road, two (2) driveways on Sager Avenue, and two (2) driveways on University Drive.
- With redevelopment of the property, site access will be provided via Sager Avenue and University Drive.
- The proposed South Street Extension will reroute traffic and relieve congestion on Main Street through Old Town and will ultimately provide access to The Ox Fairfax Block A site. The extension is expected to be complete in 2031 and is to be built by others.

Non-SOV Elements

- Sidewalks and curb ramps generally exist along the corridors adjacent to and within the vicinity of the site.
- A dedicated on-street bike lane currently exists on University Drive adjacent to the site.
- Five (5) bus routes provide service in the vicinity of the site, providing regional access to the area.

Analysis Results

- Nine (9) intersections within the study area have movements that operate below acceptable levels of service under the Existing Conditions (2023).
- Ten (10) intersections within the study area have movements that operate below acceptable levels of service under Future Conditions without Development (2026).
- After the addition of redevelopment traffic, one (1) intersection has movements that begin to operate below acceptable levels of service as compared to the Future Conditions without Development (2026), after mitigations.

Proposed Mitigation

- Future with Development (2026) Mitigation measures have been identified to improve traffic operations in the Future with Development (2026) scenario. These improvements include the following:
 - o Intersection 1: Main Street and West Street/North Street
 - Adjust signal timings to improve operations (SAT Peak only).

- o Intersection 5: Main Street and East Street/Blenheim Boulevard
 - Adjust signal timings to improve operations (PM Peak only).
- Intersection 7: Main Street and Chain Bridge Road
 - Adjust signal timings to improve operations (PM Peak only).
- o Intersection 12: University Drive and South Street
 - Install a traffic signal and add exclusive northbound left lane.